Seismic Hazard Microzonation Based on Probability Seismic Hazard Analysis in Bandung Basin

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ABSTRACT The geological and seismic-tectonic setting in the Bandung Basin area proliferates the seismicity risk. Thus, it is necessary to investigate the seismic hazards caused by the foremost seismic source that affects the ground motions in the bedrock. This research employed Probability Seismic Hazard Analysis (PSHA) method to determine the peak ground acceleration value. It considers the source of the earthquakes in the radius of 500 km with a return period of 2500 years. The analysis results showed that the Peak Ground Acceleration (PGA) in this region varies from 0.46 g to 0.70 g. It correlates with the magnitude and hypocentre of the dominant earthquake source of the study locations. The PGA value on the bedrock was used as an input to develop the seismic hazard microzonation map. It was composed using the Geographic Information System (GIS) to visualise the result. This research provides a scientific foundation for constructing residential buildings and infrastructure, particularly as earthquake loads in the building structure design calculations.

Keywords: Seismic hazard, PSHA, PGA in the bedrock, GIS, Bandung Basin

INTRODUCTION

There are two types of earthquake source which geologically affect Bandung Basin; namely the subduction and the shallow crustal zone (Sengara et al., 2001). Lembang Fault is the typical shallow crustal earthquake affecting the basin (Daryono et al., 2019). While the Sunda Megathrust, which is part of the West-Central Java segment, is a typical
subduction earthquake affecting the basin (Syahbana et al., 2018). These earthquake lines have significant physical impacts on several areas in the Bandung basin, including Bandung Regency, Bandung City, and West Bandung Regency. The Metropolitan area in Greater Bandung consists of 56 districts and has a total built-up area of 26,142 Ha of 106,015 Ha (West Java Province Metropolitan Development Management (WJPMDM), 2013). Hence, scientific information about seismic hazards plays a vital role in earthquake-prone area development, especially in infrastructure and building development.

Several large earthquakes swayed the Bandung basin over the last decade. A prior paleo-seismological study has estimated these events based on historical earthquake events. The Lembang Fault triggered an earthquake event with a magnitude around 6.6 Richter Scale (R) 2,000 years ago, and this activity has a strong possibility to recur in the future (Yulianto, 2011). Correspond to this claim, on September 2nd, 2009, the megathrust activity in the south of Java subduction zone triggered a large earthquake event at Tasikmalaya (Daryono et al., 2009). It had a magnitude of 7.3 on the Richter Scale (R), with the epicentre located at -8.24 latitude and 197.32 longitude. The waves rippled through the Bandung basin, which caused considerable losses and severe damages to 53,910 houses and 2,461 public facilities. Moreover, a total of 23 people had died or disappeared, and also, 78,174 people were displaced and suffered.

Two years later, on July 28th and August 28th, there were earthquake events located in Bandung city with a magnitude of 2.9 and 3.3 Mercalli (M), respectively. These events created minor damage to 103 buildings in the Bandung basin (Meilano et al., 2012). The dense population in residential areas in this area is one crucial factor in considering preventive measures to reduce the risk of earthquake events that require an in-depth study of seismic hazards.

Seismic hazard analysis is an approach to estimate the earthquake ground shaking hazard at a particular location. The aim is to advance earthquake-resistant building design and to assess building safety such as bridges, high-rise buildings, dams, etc (Gupta, 2002). There are two conventional approaches to analyse seismic hazard, namely Deterministic Seismic Hazard Analysis (DSHA) and Probability Seismic Hazard Analysis (PSHA) (Krinitzsky, 1995). On the one hand, DSHA is the probability of ground motion analysis using a simple statistic of a single or a set of earthquakes (Krinitzsky, 2002). On the other hand, PSHA considers uncertainties or non-controlled sources, such as future earthquakes (Baker, 2013). Additionally, the DSHA approach calculates site-specific evaluation, while the PSHA approach considers the recurrences of events. The PSHA approach obtains peak ground acceleration value from ground motion on the bedrock to estimate earthquake event occurrences in 2500 years return period.

Recent studies on seismic hazard analysis in the Bandung Basin have been accomplished. Horizontal-to-Vertical Spectral Ratio (HVSR) approach and site classification from CPTu and SPT data were used to reveal velocity amplification distribution in the Bandung Basin (Sari et al., 2019). Another study used a different approach that considers seismic sources from subduction areas and active faults surrounding the Bandung Basin (Sengara et al., 2001). It is necessary to research seismic hazard analysis using the latest earthquake sources as the number of installed seismic instruments has been increasing and lead to successful seismic source identification (Tim Pusat Studi Gempa Nasional, 2017). Hence, it is possible to obtain peak ground acceleration close to the real condition using the latest seismic sources and attenuation equations.

An investigation of the earthquake data collected from the year 2009 until 2011 showed that the earthquake disaster was not solely caused by active faults movement located close to the Bandung Basin but also had to consider other earthquake sources. For instance, the PGA value in the bedrock was determined using the seismic hazard analysis approach. Furthermore, this research presents a seismic microzonation map to display the PGA values' distribution from seismic sources with a buffer radius of 500 km measured from the centre of Bandung Basin.

GEOLOGICAL TECTONIC SETTING

Bandung Basin was developed by several geological formations, as shown in Figure 1. The youngest formation, Kosambi Formation, consists of Holocene tuffaceous sand, silt, and clay, which
presents in the southern part of the Bandung Basin. Cibereum Formation that older than the Kosambi Formation, composed of pumice, obsidian, basalt, breccia, tuff, and andesite. This formation presents in the west to the eastern part of the Bandung Basin. The oldest part, Cikapundung Formation, is exposed in the northern part of Bandung Basin, consisting of
breccia, lahar, and lava. The Bandung Basin's bedrocks consist of Tertiary sedimentary rocks, Tertiary volcanic rock, and intrusive rocks. These geological rocks outcrop along the northern part of the Bandung Basin, from west to the east (Silitonga, 2003). Based on these geological formations, 35 different samples were collected using Cone Penetration Tests (CPTu) and borehole measurements. They were distributed in 9 locations in Bandung district, 12 locations in Bandung City, and 14 locations in West Bandung Regency. The distribution of sampling site locations is shown in Figure 1. As for the tectonic setting, there is a subduction mechanism that mainly affects the Bandung Basin.

The subduction zone between the Indo-Australian plate beneath the Eurasian plate causes several identified and unidentified active faults (Soehaimi, 2011). For instance, the identified active faults, such as the lateral strike-slip of Lembang Fault, Cimandiri Fault, and the Baribis Fault, are found in West Java. On the contrary, unidentified right-lateral slips located in the Garut cluster might be related to the local fault (Supendi et al., 2018).

The Lembang Fault has a length of approximately 29 km and a slip-rate of around 1.95 - 3.45 mm/yr (Daryono et al., 2019). While the Cimandiri Fault has 23 km in length and a slip rate around 0.55 mm/yr (Tim Pusat Studi Gempa Nasional, 2017). Subduction mechanism also has a vital role in seismic hazard impacts in the Bandung Basin, such as the Sunda Strait Megathrust and West-Central Java Megathrust (Sari et al., 2020), as shown in Figure 2.

**METHODOLOGY**

The seismic hazard analysis was accomplished using the PSHA method (Baker, 2013), consisting of uniform hazard spectra calculation, deaggregation, and ground motion synthetic modelling. Furthermore, it also incorporates some uncertainties parameters, namely the earthquake magnitude, the earthquake's location, attenuation of earthquake shocks, and earthquake events' recurrence rate (Tim Pusat Studi Gempa Nasional,

![Flow chart of seismic hazard analysis](image-url)

Figure 3. Flow chart of seismic hazard analysis in this study.
The main essence of the PSHA method is seismic source identification in the radius of 500 km from coordinates locations and calculation using selected attenuation functions, such as the Ground Motion Prediction Equation (GMPE). The attenuation function estimates the level of ground shaking due to earthquake waves concerning the magnitude, the distance from the earthquake source to a particular location, and an earthquake source's conditions. The seismic source database was obtained from secondary data. A brief description of the research method is shown in Figure 3.

This study used 29 seismic sources database collected from Peta Sumber dan Bahaya Gempa Indonesia Tahun 2017 (Tim Pusat Studi Gempa Nasional, 2017) and Daryono et al. (2019), as shown in Table 1 and Table 2. PSHA analysis used these seismic sources data as an input database to produce ground motion synthetics on the bedrock.

There are four stages of procedure to obtain PSHA synthetic ground motion. The first stage is to identify the seismic sources mechanism, faults mechanism, subduction mechanism, and select the appropriate attenuation function for each mechanism. The seismic source mechanism opted for GMPE as the attenuation function. The attenuation equation functions used for fault mechanism are the Next Generation Attenuation (NGA) from Boore and Atkinson (2008), Campbell and Bozorgnia (2008), Chiou et al. (2008), and Chiou and Youngs (2008). Meanwhile, the attenuation equation functions for the subduction mechanism applied in this research are Atkinson and Boore (2003), Zhao et al. (2006), and Youngs et al. (1997). The second stage is to determine the seismicity and earthquake distribution on these attenuations function using the logic tree. This logic tree used in PSHA analysis resulted in the Probabilistic Spectra graph. The Probabilistic Spectra graph shows spectral acceleration as a function of the spectral period for a specified return period or equivalently annual probability of exceedance, known as Uniform Hazard Spectra (UHS).

The next step is obtaining the dominant earthquake magnitude and distance, known as deaggregation. Deaggregation determined the most significant earthquake source among many earthquake sources within a 500 km buffer and based on recurrent earthquake events per year (Sunardi, 2015). These results were used to obtain the response spectra. The modification of response spectra and UHS value were performed to obtain Modified Response Spectra (MRS). The deaggregation also produced the hazard curve. The hazard curve showed a full picture of dominant earthquake sources and mechanisms that influence the particular location.

Finally, ground motion synthetics are developed to determine the seismic hazard. It is based on seismic conditions at a particular area using spectral matching between the MRS against the time histories. In this study, the targeted spectra between PSHA's UHS and deaggregation response spectra were used for the MRS acquisition. Moreover, recorded data of earthquake acceleration merged as the time histories into the MRS. Thus, this spectral matching resulted in ground motion synthetics based on the earthquake source's real conditions affecting the particular location.

### RESULT AND DISCUSSION

The PSHA results are shown as the UHS curves, that resulted in the hazard spectra involving 29 seismic sources to the site location for a return period of 2475 years. These graphics showed a relationship between spectral acceleration and

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Table 1. Megathrust source database used in PSHA.

| Structure          | Segment                  | Length (km) | Width (km) | Magnitude | b*  | a*  |
|--------------------|--------------------------|-------------|------------|-----------|-----|-----|
| Sumatran Megathrust| Selat Sunda              | 290         | 100        | 8.7       | 1.15| 5.99|
| Sunda Megathrust   | West-Central Java'       | 700         | 150        | 8.7       | 1.08| 5.55|
| Sunda Megathrust   | East Java                | 280         | 150        | 8.7       | 1.08| 5.63|

*Gutenberg–Richter constant value

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period at several locations. Here UHS curves of sites CPTu-04, BH -2, and CPTu-13 are presented in Figure 4, showing the maximum until the minimum values.

The CPTu-04 UHS curve shows the maximum value of spectral acceleration on 0 s period (T), of 0.60g. The UHS curve's minimum value showed on CPTu-13, about 0.48g, while the UHS curve on BH-02 showed the dominant UHS curve value in the Bandung Basin, which is 0.52g. These UHS curves were then further analysed using response spectra from deaggregation analysis to produce the Modified Response Spectra (MRS).

In the deaggregation stage, an analysis is carried out at the return period of 2475 years at the 0-second point (PGA). The results show that the dominant magnitude at the review locations: CPTu-04, BH -2, and CPTu-13 (Figure 5) is around 7.55-7.86 M, with a distance of about 66 - 130 km. The earthquake mechanism affecting the earthquake source and distance are shown in Figure 6.

Table 2. Faults source database used in the PSHA.

| Segment                     | Fault Type | Dip   | Length (km) | Width (km) | Magnitude (Maximum) |
|-----------------------------|------------|-------|-------------|------------|---------------------|
| Kumering-North              | SS*        | ~90   | 111         | 20         | 7.5                 |
| Kumering-South              | SS         | ~90   | 60          | 20         | 7.1                 |
| Semangko Barat-A           | SS         | ~90   | 90          | 20         | 7.4                 |
| Semangko Barat-B            | SS         | ~90   | 80          | 20         | 7.3                 |
| Semangko Timur-A           | SS         | ~90   | 12          | 20         | 6.5                 |
| Semangko Timur-B            | SS         | ~90   | 35          | 20         | 6.9                 |
| Semangko Graben             | N**        | 60    | 50          | 20         | 6.5                 |
| Ujung Kulon A               | SS         | ~90   | 80          | 20         | 7.3                 |
| Ujung Kulon B               | SS         | ~90   | 150         | 20         | 7.6                 |
| Cimandiri                   | R***       | E-W   | 23          | 23         | 6.7                 |
| Nyalindung-Cibeber          | R          | 30    | 15          | 15         | 6.5                 |
| Rajamandala                 | SS         | 45    | 22.5        | 6.6        |
| Lembang                     | SS         | E-W   | 29.5        | 29.5       | 6.8                 |
| Subang                      | R          | NW    | 33          | 16.5       | 6.6                 |
| Cirebon-1                   | R          | NW    | 15          | 7.5        | 6.5                 |
| Cirebon—2                   | R          | NW    | 18          | 9          | 6.5                 |
| Karang Malang               | R          | EW    | 22          | 22         | 6.7                 |
| Brebes                      | R          | 22    | 11          | 6.5        |
| Tegal                       | R          | ENE   | 15          | 15         | 6.5                 |
| Pekalongan                  | R          | NE    | 16          | 16         | 6.6                 |
| Weleri                      | R          |       | 17          | 17         | 6.6                 |
| Semarang                    | R          | EW    | 34          | 17         | 6.6                 |
| Rawakening                   | R          | NW    | 18          | 9          | 6.5                 |
| Demak                       | R          | EW    | 31          | 15.5       | 6.6                 |
| Purwodadi                   | R          | EW    | 38          | 19         | 6.7                 |
| Cepu                        | R          | ESE   | 100         | 50         | 7.1                 |

* Strike – Slip fault  
** Normal fault  
*** Ring fault
Figure 4. UHS curves in: (a) CPTu-04, (b) BH-02 and (c) CPTu-13.

Figure 5. Deaggregation analysis results in: (a) CPTu-04, (b) BH-02 and (c) CPTu-13.
Figure 6. Seismic sources distribution graph in: (a) CPTu-04, (b) BH-02 and (c) CPTu-13.
Figure 7. Ground motion synthetics in: (a) CPTu-04, (b) BH-02, and (c) CPTu-13.
As shown in Figure 6, the seismic source distribution charts were observed to analyse the dominant seismic source's earthquake mechanism. The PGA values were plotted on the seismic source distribution graph against the hazard values. Then, we can observe the earthquake mechanism through the Annual Frequency of Exceedance. Experimental results toward 35 location points of CPTu-04, BH-02, and CPTu-13 show that the subduction earthquake source mechanism has dramatically impacted the Lembang Fault at CPTu-04 location points. Meanwhile, the earthquake source mechanism of the West-Central Java Megathrust has affected BH-02 and CPTu-13 location points.

The response spectra values were obtained from the deaggregation result using the dominant earthquake's magnitude and distance. It also considers the attenuation functions based on the earthquake source mechanism taken from the seismic source distribution graph.

To obtain the targeted response spectra, or better known as the MRS, the response spectra were modified using the UHS value. We performed spectral matching using the MRS and selected time histories to acquire the ground motion synthetic values on 35 sample points. The results of the ground motion synthetics analysis, as shown in Figure 7, show that soil acceleration in the bedrock varies from 0.46g to 0.70g. The detailed...

| SPT and CPTu Locations | a<sub>max</sub> (bedrock), g | SPT and CPTu Locations | a<sub>max</sub> (bedrock), g |
|------------------------|-----------------|------------------------|-----------------|
| BH01                   | 0.502           | CPTu 01                | 0.578           |
| BH02                   | 0.505           | CPTu 02                | 0.596           |
| BH03                   | 0.488           | CPTu 03                | 0.518           |
| BH04                   | 0.538           | CPTu 04                | 0.700           |
| BH05                   | 0.486           | CPTu 05                | 0.517           |
| BH06                   | 0.510           | CPTu 06                | 0.526           |
| BH07                   | 0.538           | CPTu 07                | 0.509           |
| BH08                   | 0.516           | CPTu 08                | 0.525           |
| BH10                   | 0.497           | CPTu 09                | 0.506           |
| BH A                   | 0.488           | CPTu 10                | 0.543           |
| BH B                   | 0.492           | CPTu 11                | 0.677           |
| BH C                   | 0.488           | CPTu 12                | 0.668           |
|                        |                 | CPTu 13                | 0.457           |
|                        |                 | CPTu 14                | 0.505           |
|                        |                 | CPTu A                 | 0.621           |
|                        |                 | CPTu C                 | 0.506           |
|                        |                 | CPTu D                 | 0.481           |
|                        |                 | CPTu E                 | 0.483           |
|                        |                 | CPTu F                 | 0.486           |
|                        |                 | CPTu G                 | 0.500           |
|                        |                 | CPTu H                 | 0.532           |
|                        |                 | CPTu I                 | 0.497           |
|                        |                 | CPTu J                 | 0.512           |
PGA values on the bedrock are presented in Table 3.

The dominant earthquake magnitude and distance influence the variations of the PGA value. As we could see in Figure 8, the maximum value of $a_{\text{max}}$ showed in CPTu 04 is around 0.70g. This value is influenced by the Lembang Fault based on the deaggregation result. In contrast to CPTu-04, the minimum PGA value showed in CPTu-13 is 0.46g, influenced by the West-Central Java Megathrust. Although these survey locations earthquake nearby, specific point locations can be more affected by different hypocentres and seismic sources. These results also showed that the needed seismic sources complete database are highly recommended to calculate more accurate seismic hazard analysis.

CONCLUSION

Our research results show that bedrock acceleration values in the Bandung Basin area range from 0.46g - 0.70 g, while Indonesia Earthquake Map 2017 showed PGA values from 0.50g-0.60g. This research shows more detailed results than previous research that would be advantageous for building construction references, such as designing bridges, tunnels, and tall buildings design construction. In the construction design calculation, it is crucial to give attention to an earthquake's acceleration load in the computation process. The discrepancy in the annual return period will affect the acceleration value in the bedrock. Apart from the differences, the number of seismic sources database will significantly affect the seismic hazard analysis results' accuracy.

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