Study of Early Strength of Concrete for Bridge Construction

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Abstract: Quickened connect development has turned into a prevalent option in contrast to utilizing conventional development procedures in new extension development and existing scaffold deck substitution due to the decrease of time spent in field exercises.

A key element of scaffolds manufactured utilizing ABC strategies is the broad utilization of pre-assembled segments. Pre-assembled segments are participated in the field utilizing little volume conclusion pours including elite materials (steel and cement) to guarantee satisfactory exchange of powers between segments.

The objective of this examination was to make a technique to create solid blends that are planned utilizing nonexclusive constituents and that fulfill execution prerequisites of quickened connect development conclusion pours in New England, essentially high early quality and long haul toughness.

Keyword: Quickened, Scaffold, Satisfactory, Execution, Toughness.

I. INTRODUCTION

Quickened connect development is a development system that has turned out to be prominent with existing extension deck supplanting and even with some new scaffold development ventures due to the decrease in on location exercises. By decreasing the on location exercises, ABC procedures diminish the general development time, which results in monetary funds. ABC strategies likewise make more secure roadway conditions and decrease traffic postpones when contrasted with conventional development techniques.

One regular innovation utilized with ABC is pre-assembled connect components and frameworks.

II. OBJECTIVE OF THIS STUDY

The primary target of this exploration venture was to create and approve solid blends that grow high-early quality without adversely influencing their long haul execution.

The solid blends intended for this examination venture were created for use in ABC in New England; along these lines, consideration was paid to conditions explicit to the earth in the area when building up the blends, for example, solidify defrost cycles, the utilization of deicing materials and solid position under a shifted scope of temperatures.

III. LITERATURE REVIEW

Byard and Ries 2011 Due to this inadequacy in the interfacial change zone of cement, a powerless point frames in the solid and gives an ideal pathway to the entrance of conceivably unsafe chloride particles.

Fowler 2006; Young et al. 1998 There are a few sorts of total degrees. All in all, persistently evaluated total and degrees with high pressing densities are great to grow high quality cement.

Koehler 2014 Gap degrees regularly result in a solid with an increasingly liquid consistency, which implies less high-go water lessening (HRWR) admixture is required for an ideal usefulness. Hole degree can likewise be useful to solid quality since it can help achieve high pressing thickness.

Mindess and Young 1981 There has likewise been some analysis of this strategy; expressing that the allegorical degrees essentially don't work and there should be a sure extent of fine total for functionality purposes.

Xie et al. 2012 most extreme total size strongly affects the quality of cement. The primary explanation behind this is ascribed to the adjustment in bond quality between coarse totals and concrete glue with various total sizes.
A. Compressive Strength
The compressive strength was measured for every batch of every concrete mixture prepared during this project.

### Table 4.1: Compressive Strength Values for Selected Concrete Mixtures

| Concrete Mixture | Batch | 12-HOUR | 24-HOUR | 7-DAY  | 28-DAY |
|------------------|-------|---------|---------|--------|--------|
| MIX 6-HD         | a     | 4280    | 5940    | -      | -      |
|                  | b     | 3500    | 5870    | -      | -      |
|                  | c     | 3870    | 5710    | 7680   | 10560  |
|                  | d     | 3570    | 5660    | -      | -      |
|                  | e     | 3970    | 5340    | -      | -      |
|                  | f     | 3840    | 5470    | -      | -      |
| Average          |       | 3840    | 5660    | 7680   | 10560  |
| MIX 6-HD-H       | a     | 5140    | 6160    | -      | -      |
| Average          |       | 5140    | 6160    | -      | -      |
| MIX 15-HD        | a     | 3970    | 5620    | -      | -      |
|                  | b     | 3700    | 5360    | -      | -      |
|                  | c     | 3930    | 5350    | 7500   | 10300  |
|                  | d     | 3670    | 5140    | -      | -      |
| Average          |       | 3820    | 5370    | 7500   | 1030   |
| MIX 15-HD-H      | a     | 5200    | 6160    | -      | -      |
| Average          |       | 5200    | 6160    | -      | -      |

A table providing the compressive strengths measured for each batch can be found. The compressive strength goal for the concrete mixtures developed in this project was to reach 4000 psi in 12 hours. The compressive strength that the two selected concrete mixtures, MIX 6-HD and MIX 15-HD reached in 12 hours is only about 5% under the target strength. If it is necessary to reach the exact target compressive strength of 4000 psi in 12 hours, the curing temperature can be increased. By increasing the curing temperature, a 12-hour compressive strength 30% greater than the target was attained during this project.

B. Slump/Spread
A slump or spread measurement was taken for every concrete mixture that was prepared for this project. In APPENDIX C, a full tabulated list of slump and spread measurements taken for each concrete mixture is provided. Great variability existed in the consistency of trial batch and selected concrete mixtures, not only between different concrete mixture designs, but also within one concrete mixture design. This can be clearly seen with the two selected concrete mixtures, MIX 6-HD and MIX 15-HD.

### Table 4.2: Slump and Spread Values for Selected Concrete Mixtures

| Concrete Mixture | Test Executed | Slump or Spread Reading (inches) |
|------------------|---------------|---------------------------------|
| 6-HD             | spread        | 21                              |
|                  |               | 19                              |
|                  |               | 18                              |
|                  |               | 17                              |
|                  | slump         | 8                               |
|                  |               | 5                               |
|                  |               | 3                               |
| 15-HD            | spread        | 30                              |
|                  |               | 21                              |
|                  |               | 18                              |
|                  |               | 16                              |
|                  | slump         | 16                              |
IV. CONCLUSIONS

To develop the concrete mixtures, various proportioning methods were studied. There were three methods considered: (1) building upon past experience by using state-of-practice concrete mixtures; (2) following ACI 211.4R Guidelines; and (3) targeting a mixture with maximum aggregate compaction.

The first method used was the state-of-practice concrete mixtures, which were collected from DOTs and pre-casters throughout New England. For this method, the state-of-practice concrete mixtures were used as baseline designs. However, when state-of-practice concrete mixtures were replicated during this project, the strength and/or constructability goals of this project were not met. This was expected since the reported strength and/or constructability were also less than the goals of this project. Chemical admixture dosages were altered in an attempt to achieve strength and constructability goals with the state-of-practice concrete mixtures, but it was not successful.

The second method considered for concrete proportioning was to follow the guidelines presented by ACI Committee 211, ACI 211.4R: Guide for Selecting Proportions for High-Strength Concrete Using Portland Cement and Other Cementitious Materials. The trial batch concrete mixtures developed using this method produced baseline results that were relatively close to acceptable strength and constructability limits. Through the use chemical admixtures and slight alterations to the proportions, it appeared that this method would provide satisfactory results. However, this method was not chosen to be used as the primary proportioning method in this research project because proportioning is based on tables and equations without the exact constituent proportions necessarily being provided. The final method considered for concrete proportioning, maximum compaction of aggregates, also produced baseline results that had strength and constructability results that were within acceptable limits, and this method allowed for easier manipulation of the concrete mixture designs, given the knowledge of exact proportions.

The final method, maximum compaction of aggregate was developed using five parameters: aggregate gradation, coarse aggregate size, w/cm ratio, percent fly ash replacement and volume of paste to volume of voids (V_{paste}/V_{voids}) ratio. Aggregate gradation was designed to create maximum compaction of aggregates, which was achieved using Fuller-Thompson curves. The coarse aggregate size, w/cm ratio, percent fly ash replacement and V_{paste}/V_{voids} ratio were modified throughout the development of the concrete mixtures. The initial values used were as follows: coarse aggregate size equal to 1/2 inch, w/cm equal to 0.26, 0% fly ash replacement and V_{paste}/V_{voids} equal to 1.75.

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