Application of UAV Photogrammetry on Ecological Restoration of Abandoned Open-pit mines, Northern Anhui Province, China

Hongbo Dai* and Jiying Xu**(***)†
*School of Environment and Surveying Engineering, Suzhou University, Suzhou, 234000, Anhui, China
**School of Resources and Civil Engineering, Suzhou University, Suzhou, 234000, Anhui, China
***National Engineering Research Center of Coal Mine Water Hazard Controlling (Suzhou University), Suzhou, 234000, Anhui, China
†Corresponding author: Jiying Xu; jiyingxu1986@163.com

ABSTRACT
Mine survey is an important part of the ecological restoration design of an abandoned open-pit mine because it gives precise survey data for mine management. Unmanned aerial vehicle (UAV) photogrammetry technology is applied to mine 3D modeling and earthwork calculation to tackle the problems of high cost, low efficiency, and high labor intensity in traditional manual field mine surveys. Using the abandoned quarries in Baitu, Suzhou City, as the study area, 82 ground control points were set up, and massive elevation data of the ground was obtained using a Dajiang spirit 4 RTK UAV for aerial photography and extracted from a 3D model created with Context Capture software, while a digital orthophoto image of the study area was created. The earthwork volume is calculated using the square grid method. UAV photogrammetry has a considerably higher efficiency than typical manual field measurement results, and it meets the precision criteria of mine geological environment assessment. According to the findings, UAV photogrammetry technology has a promising future in mine geological environment monitoring and ecological environment repair.

INTRODUCTION
Mineral resources are the material basis of human economic and social development, and the “food” and main power source of industry, agriculture, national defense, and other social industries (Zhai & Hu 2021). Large scale and excessive exploitation of mineral resources will bring about environmental problems such as vegetation destruction, soil erosion, ecological environment deterioration, and so on (Hu et al., Mohammad et al., Hu et al., Mohammad, Wahsha et al., 2015). Various mines have not returned to their former ecological state after shutting, and there are some issues that need to be addressed, such as land idleness, tailings buildup, ground accretion, and mineral pollution (Sivarajah et al., 2019). The foundation of abandoned mine restoration, land reclamation, and mine green sustainable development is mine ecological environment repair. The mining survey is the key step that can give an accurate measuring basis for the design of ecological restoration during the restoration project (Isiaka et al., 2019).

The traditional manual field mine survey approach, which employs a total station and GPS technology, has drawbacks such as expensive costs, long lead times, and vulnerability to the field environment, among others (Jie et al., 2018). The 3D laser scanning technology has been used in mine geological monitoring because of its advantages of telemetry, speed, and efficiency, but it also has the disadvantages of high labor intensity and inability to acquire reliable information in the blind area (Thurley et al., 2015). The key difficulty in the field of mine survey is how to lower costs while maintaining high quality and efficient access to geometric information and physical properties of mining areas. In recent years, (unmanned aerial vehicle) UAV technology has become widely used in a variety of fields, particularly in engineering geology research, including basic theory research (Xing et al., 2011, Shan et al., 2013), rock discontinuity investigation (Salvini et al., 2020), underground tunnel mapping (Wirachman et al., 2010), slope stability analysis (Wang et al., 2019), and landslide deformation monitoring. However, there are few studies on ecological restoration of abandoned open-pit mines using UAV photogrammetry.

The study’s goal was to see if UAV photogrammetry might be used to help with open-pit mine ecological restoration. The Dajiang spirit 4 RTK UAV was employed for aerial photography in this paper, with the abandoned quarries in Baitu town of Suzhou City as the study region. Massive elevation data was retrieved from a 3D model created with the Context Capture software. The earthwork...
volume is calculated using the square grid method, and the measurement accuracy is compared to traditional manual field measurement findings. The findings will be used to establish sustainable development plans, as well as to better understand the management and development of abandoned mining resources.

MATERIALS AND METHODS

Study Area

The abandoned quarries are located 55 km north of Suzhou city in Anhui province, China. Its geographical coordinates are 117°2'1"-117°7'22" north and 34°9'26"-34°4'32" east, as shown in Fig. 1. The climate of the study area belongs to the warm temperate zone and semi-humid monsoon section, characterized by a mild climate and four distinct seasons, with an average annual temperature of 14.5°C and an average annual rainfall of around 774 mm. Geologically, the quarries area is located between Su Bei Fault and the Feng Pei fault.

The abandoned quarries are limestone mines for building stone, which consist of four mines and cover a damaged area of 322450.8 m² and an undamaged area of the original mountain 68226.5 m² respectively (Fig. 1). In the study region, there are several dangerous rocks, and the rock at the top of the cliff face is relatively shattered. There is a risk of geological disasters such as collapse and landslides as a result of local slope mining along the rock layers.

Principle of Photogrammetry

UAV photogrammetry uses theories and algorithms such as computer technology, digital picture feature extraction, and image matching to create a three-dimensional model and extract relevant geometric and physical information, all based on the basic principle of photogrammetry (Lin et al. 2018, Wang et al. 2020). The image-object relationship model is constructed based on the collinear relationship of the image point, photogrammetry center, and object point (Qiang & Tian 2019).

\[
x - x_0 = -f \frac{a_1(X - X_S) + b_1(Y - Y_S) + c_1(Z - Z_S)}{a_3(X - X_S) + b_3(Y - Y_S) + c_3(Z - Z_S)}
\]

\[
y - y_0 = -f \frac{a_2(X - X_S) + b_2(Y - Y_S) + c_2(Z - Z_S)}{a_3(X - X_S) + b_3(Y - Y_S) + c_3(Z - Z_S)}
\]

\[\text{...}(1)\]

The photographing scale is determined by the focal length \(f\) and the height \(h\) of the camera (Meng et al. 2020). According to the photographing scale and the pixel size \(a\) of the camera, the ground resolution \(G\) can be determined by the formula (2).

\[
\frac{1}{m} = \frac{F}{H} = \frac{a}{G}
\]

In the formula (2): \(1/m\) represents photographing scale.

Principle of Square Grid Method

The square grid method is common and effective in earthwork calculation (Zhang et al. 2015, Guo et al. 2016). The calculation steps are as follows: (1) the study area is divided into several squares. (2) For each grid, the product of the grid area and difference value (between design elevation and measured elevation) is regarded as the engineering quantity of a single grid. (3) The calculation results of each grid are accumulated to obtain the total quantities of the study area.

Fig. 1: Geographical location map of the study area.
The calculation formula is shown in (3) (4).

\[ H_{ij} = \frac{1}{k} \sum_{j=1}^{k} (H_j - H_D) \quad \ldots (3) \]

\[ V = \sum_{i=1}^{n} H_{ij}ab \quad \ldots (4) \]

In the formula (3): \( H_{ij} \) is the elevation of i row and j column grid; K represents the number of points in the grid; \( H_j \) is the actual elevation of points in the grid; \( H_D \) is the design elevation. In the formula (4): V represents the total amount of work; n is the number of squares in the survey area; a and b are the side length of the grid respectively.

Backfill in earthwork should be taken during ecological restoration if the design elevation is higher than the observed elevation. On the other hand, if the design elevation is lower than the observed elevation, excavation measures should be taken. Furthermore, a significant number of elevation data are generated in a single grid due to the dense matching program of UAV photogrammetry, and the measured height can use the average value of all elevation data in the grid to ensure accuracy.

**Three-Dimensional (3D) Modeling**

Control surveying and UAV route planning were used in the project to gather images of abandoned quarries (He et al. 2020). Using context capture software, a 3D model was created, and the geometric information for the earthwork computation was extracted from the model.

![Fig. 2: Layout of image control points and routes in the study area.](image)

**Control Surveying**

The photo control points are primarily used for photogrammetric picture recognition, which is often based on the principle of “more around, less in the middle,” and is in accordance with the coordinate system developed afterwards (Sun et al. 2018). To make the control network more stable and match the accuracy requirements, 82 image control points (Fig. 2) are set up in the study region due to the substantial topography fluctuation of the open-pit quarry.

**UAV Image Acquisition**

There are several high and steep slopes in the research region, with a maximum slope of 80° and a difference between the platform surface and the top of the hill in the treatment area ranging from 90 to 140 m, due to the influence of mining technologies and natural conditions. As a result, the study area’s layers and blocks follow the design path (Xu et al. 2016). The center of each block area is chosen as the flight site to assure the quality of the images. The relative elevation of the study area is higher during the field survey, the wind speed is strong and changes often, and the field aerial survey difficulty coefficient is higher than that of residential aerial survey.

The UAV of Dajiang spirit 4 RTK was used to obtain the image. To meet the accuracy requirements of earthwork

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Flow chart of 3D modeling.
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```mermaid
graph TD
New project --> Import photos
Import photos --> Setting image parameters
Setting image parameters --> coordinate system\[Camera model\][Image accuracy]
Setting image parameters --> Setting processing template (3D)
Setting processing template (3D) --> Setting output coordinate system (CGCS2000)
Setting output coordinate system (CGCS2000) --> Automatic aerial triangulation
Automatic aerial triangulation --> Accuracy check
Accuracy check --> Manual adjustment
Manual adjustment --> Qualified
Qualified --> Unqualified
Unqualified --> Mark control points
Mark control points --> Generate 3D model
```

![Fig. 3: Flow chart of 3D modeling.](image)
calculation, the mapping scale of 1:1000 and the photography scale of 1:5000 are selected. The designed altitude is set at 90 m, and the overlap of heading and side direction is 80%. A total of 12 routes (Fig. 2) are arranged, flying in three voyages, and the horizontal flight speed is 6 m/s\(^{-1}\). A total of 805 images were obtained from the front, back, left and right orthophoto 5 cameras, the image size is 5472 pixels × 3678 pixels, and the average ground resolution of tilt image is 2.08 cm.

3D Modeling

According to the results of the control survey and the images obtained by UAV, the 3D model of the abandoned outcrop mine is established by using context capture software and the process shown in Fig. 3 (Jing et al. 2015). The model post-processing is carried out by using the software of EPT and EPS (encapsulated postscript). The 3D modelings of abandoned mines are shown in Fig. 4.

Model Accuracy Evaluation

Mean square error is often used to evaluate the accuracy of mapping, including \(M_S\) (plane) and \(M_H\) (elevation) (Puliti et al. 2018). The calculation formula is as follows (5), (6), and (7).

\[
M_S = \sqrt{\frac{\sum (\Delta S)^2}{n}} \quad \ldots(5)
\]

\[
M_H = \sqrt{\frac{\sum (\Delta H)^2}{n}} \quad \ldots(6)
\]

\[
\Delta S = \sqrt{(\Delta X)^2 + (\Delta Y)^2} \quad \ldots(7)
\]

In the above formulas, the \(\Delta s\) represents the plane distance error, \(\Delta x\), \(\Delta y\), and \(\Delta h\) represent the absolute error of three-dimensional coordinates respectively, and \(n\) is the number of measuring points. Based on the manual measurement data (measured by total station) and 3D model measurement data (extracted from UAV), \(M_S\) and \(M_H\) are calculated.

RESULTS AND DISCUSSION

First, 50 checking points were evenly selected in the survey area, and the total station was used to obtain the three-dimensional coordinates \((X, Y, H)\); second, the total station measurement results are subtracted from the corresponding 3D model measurement data, and the absolute errors of the \(\Delta x\), \(\Delta y\), \(\Delta h\), and \(\Delta s\) are calculated; finally, \(M_S\) and \(M_H\) are used to calculate by the formulas (5), (6) and (7). The accuracy evaluation results of the 3D model in the study area are shown in Table 1.

According to “Specification for aerial photogrammetry of 1 : 500 1 : 1000 1 : 2000 topographic map” (China National Standardization Committee 2008), \(M_S\) and \(M_H\) of 1 : 1000 topographic map in mountainous and hilly areas should be less than 0.4 m and 0.35 m respectively. The calculated results of \(M_S\) and \(M_H\) for the study area are 0.083 m and 0.069 m respectively, indicating 3D modeling meets the accuracy requirements, which can be used for earthwork calculation.

Earthwork Calculation of Abandoned Mines

Based on the manual measurement data (total station) and 3D model measurement data (extracted from UAV), the volume of earthwork was calculated by the square grid method (Liu et al. 2020). To create the original topographic map and the construction topographic map, the original elevation points and construction elevation points obtained by two types of survey methods are first loaded into Cass 9.1 software. Second, the boundary between filling and excavation is delineated by superimposing and comparing the elevation data of the two maps, and the region is divided into blocks and serially numbered, generating a block map. Finally, the original map and construction map for a single block is superimposed on the block map, and the block’s volume \(V_1\) and \(V_2\) are calculated using 0 elevations as the datum plane and 5 meters as the grid length. The value of \(V_1\) minus \(V_2\) is the block’s excavation or filling volume. Finally, the regional blocks are added together to determine the mine’s earthwork volume. Table 2, Fig. 5, and Fig. 6 present the results of four abandoned quarries’ calculations.

Hence, the excavation difference between the two methods is 4577.36 m\(^3\), and the difference rate is between 0.019-0.035, with an average of 0.023. Meanwhile, the filling difference between the two methods is 1669.41 m\(^3\), and the difference rate is between 0.018-0.014, with an average of 0.020. The engineering earthwork calculation is a complex problem involving multiple influencing factors, which cannot be accurately calculated in advance, and the current specifications do not specify the error of earthwork measurement. Generally, the error of earthwork measurement from relatively flat to uneven is controlled within 5% (Li
Table 1: The calculation results of accuracy evaluation.

| point | ΔX(m) | ΔY(m) | ΔH (m) | ΔS (m) | point | ΔX (m) | ΔY(m) | ΔH(m) | ΔS(m) |
|-------|-------|-------|--------|--------|-------|-------|-------|-------|-------|
| 1     | -0.018| 0.043 | 0.056  | 0.047  | 26    | 0.061 | 0.029 | 0.042 | 0.068 |
| 2     | 0.053 | 0.021 | 0.034  | 0.057  | 27    | 0.025 | -0.082| -0.069| 0.086 |
| 3     | 0.017 | -0.090| -0.077 | 0.092  | 28    | -0.013| 0.048 | 0.061 | 0.050 |
| 4     | -0.021| 0.040 | 0.053  | 0.045  | 29    | 0.057 | 0.077 | 0.090 | 0.096 |
| 5     | 0.049 | 0.089 | 0.102  | 0.102  | 30    | 0.026 | 0.058 | 0.071 | 0.064 |
| 6     | 0.018 | 0.070 | 0.083  | 0.072  | 31    | -0.085| 0.022 | 0.135 | 0.088 |
| 7     | -0.093| 0.134 | 0.087  | 0.163  | 32    | 0.045 | -0.016| -0.003| 0.048 |
| 8     | 0.037 | -0.004| 0.099  | 0.037  | 33    | 0.074 | 0.054 | 0.067 | 0.092 |
| 9     | 0.086 | 0.066 | 0.079  | 0.108  | 34    | 0.055 | 0.023 | 0.036 | 0.060 |
| 10    | 0.067 | 0.035 | 0.048  | 0.076  | 35    | 0.019 | -0.088| -0.075| 0.090 |
| 11    | 0.031 | -0.076| -0.063 | 0.082  | 36    | -0.019| 0.042 | 0.055 | 0.046 |
| 12    | -0.007| 0.054 | 0.067  | 0.054  | 37    | 0.051 | 0.091 | 0.104 | 0.104 |
| 13    | 0.063 | 0.069 | 0.082  | 0.093  | 38    | 0.020 | 0.072 | 0.085 | 0.075 |
| 14    | 0.032 | 0.050 | 0.063  | 0.059  | 39    | -0.091| 0.136 | 0.149 | 0.164 |
| 15    | -0.079| 0.014 | 0.027  | 0.080  | 40    | 0.039 | -0.002| 0.011 | 0.039 |
| 16    | 0.051 | -0.024| -0.011 | 0.056  | 41    | 0.088 | 0.068 | 0.081 | 0.111 |
| 17    | 0.066 | 0.046 | 0.059  | 0.080  | 42    | 0.069 | 0.037 | 0.050 | 0.078 |
| 18    | 0.047 | 0.015 | 0.028  | 0.049  | 43    | 0.033 | -0.074| -0.061| 0.081 |
| 19    | -0.089| -0.096| -0.083 | 0.131  | 44    | -0.005| 0.056 | 0.069 | 0.056 |
| 20    | -0.027| 0.034 | 0.047  | 0.043  | 45    | 0.065 | 0.071 | 0.084 | 0.096 |
| 21    | 0.043 | 0.083 | 0.096  | 0.093  | 46    | 0.034 | 0.052 | 0.065 | 0.062 |
| 22    | 0.012 | 0.064 | 0.077  | 0.065  | 47    | -0.077| 0.054 | 0.067 | 0.094 |
| 23    | -0.099| 0.039 | 0.031  | 0.106  | 48    | 0.053 | 0.023 | 0.036 | 0.058 |
| 24    | 0.031 | -0.010| 0.003  | 0.033  | 49    | 0.068 | -0.058| -0.075| 0.089 |
| 25    | 0.080 | 0.060 | 0.073  | 0.100  | 50    | 0.049 | 0.042 | 0.055 | 0.065 |

& Yang 2020, Contreras & Chung 2012). As a result, the process of determining the earthwork volume of an open pit mine using a UAV aerial survey may fully meet engineering precision criteria.

The workload of manual measurement and UAV is shown in Table 3. Table 3 shows that UAV photogrammetry requires two surveyors and two days for outdoor work, and two surveyors and 2.5 days for indoor work. Manual measurement, on the other hand, necessitates 4 surveyors and 4.75 days for fieldwork, and 1 surveyor and 2.5 days for indoor work. The conclusion can be made that UAV photogrammetry has a substantially higher work efficiency than traditional manual measuring.

**CONCLUSION**

(1) Using context capture software, a 3D model of an abandoned mine was created based on the ground control survey and images obtained by the Dajiang spirit 4 RTK UAV. The horizontal and elevation mean square errors are 0.083 m and 0.069 m, respectively, which meet the accuracy standards.

Table 2: Comparison of earthwork calculation results between manual measurement and UAV.

| No. | Area [m²] | UAV [m³] | Manual measurement | Difference rate | Area [m³] | Manual measurement | Difference rate |
|-----|-----------|----------|--------------------|-----------------|-----------|--------------------|-----------------|
| 1#  | 166821.1  | 195680.31| 192840.65         | 0.015           | 36182.96  | 36845.52           | 0.018           |
| 2#  | 137486.9  | 58959.95 | 56974.14          | 0.035           | 29147.04  | 29642.73           | 0.017           |
| 3#  | 59779.6   | 59080.51 | 60222.98          | 0.019           | 12342.92  | 12342.92           | 0.014           |
| 4#  | 26589.7   | 42243.72 | 41349.36          | 0.022           | 22915.35  | 22915.35           | 0.030           |
(2) The earthwork volume was calculated using the square grid method, which was based on data from UAV photogrammetry and a classical total station survey. The average difference rate is 0.022, which satisfies the earthwork measuring standards. In comparison to traditional field surveys, UAV photogrammetry provides the advantages of a shorter field time and automatic data processing, which considerably enhances labor productivity and is unaffected by external topographical impediments, particularly in large-area surveys.

The 3D mine model has a good overall situation, and it is also an image model that can measure and extract information, which provides significant basic data for mine ecological restoration design, and it has a promising future use in mine ecological restoration.

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