Propagation Network of Tailings Dam Failure Risk: An Empirical Research and The Identification of Key Hazard Node

Zhixin Zhen (✉ zxxzack@126.com)
University of Science and Technology Beijing

Bo Ma
University of Science and Technology Beijing

Huijie Zhao
University of Science and Technology Beijing

Research Article

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Abstract

The tailings dam system is complex, and the dam structure changes continuously over time, which makes it difficult to identify hazards and analyze the causes of failure accidents. This paper uses hazards to represent the nodes, and the relationship between hazards to represent the edges. Based on the complex network theory, the propagation network of tailings dam failure risk is constructed. The traditional identification methods usually focus on one aspect of the information of the network, while it cannot take into account to absorb the advantages of different methods, resulting in the lack of information, which will lead to a certain difference between identified key hazards and real key hazards. In order to solve this problem, by absorbing the advantages of different methods under different hazard remediation ratios, combined with the characteristics of multi-stage propagation of tailings dam failure risk, this paper proposes a multi-stage collaborative hazard remediation method (MCHRM) to determine the importance of hazard nodes. When the important nodes of this network that affect the network efficiency are found, by consulting the monitoring data, daily inspection results and safety evaluation information of each hazard before the dam failure, we can determine the real cause of the accident from the above important nodes according to the grading standards of hazard indicators. In the application example of Feijão Dam I, this article compares the key hazards obtained by the above methods with the conclusions of the accident investigation team. It can be found that the above method has a very good effect on finding the key causes of tailings dam failure.

1. Introduction

The composition of tailings is very complex, which may show strong corrosive, volatile, acidic and other characteristics affected by the types of minerals mined. If the tailings can not be managed effectively, the tailings may leak under the tailings dam failure, which will pose a serious threat to the surrounding environment and communities. On January 25, 2019, the Feijão Dam I in Brazil suddenly broke. More than 200 people died or were missing in the tailings dam accident. The Dam I has a complete management system and monitoring system, using ground-based radar, satellite (InSAR), high-definition video and drones and other advanced monitoring equipment, but before the accident, it was not found that the tailings dam had significant abnormal signals that may cause a failure \[1\] \[2\]. This shows that even in tailings dams with a very high level of safety management, there are still some key accident hazards that have not been discovered or effectively monitored. Therefore, the use of effective methods to timely and accurately to identify the key hazards in the tailings dam system, and to control the various hazards that induce accidents in the bud or latent state, is of great significance for preventing accidents and reducing the risks of tailings dam failure.

The identification of hazards and the determination of their characteristics are an important part of system safety management. It not only defines the scope of research for subsequent accident analysis and prevention, and post-disaster rescue, but also provides decision basis for managers. There are dozens of commonly used methods for identifying hazards, such as failure type and impact analysis, pre-hazard analysis, checklist method, hazard and operability research, fault tree analysis, event tree analysis
In response to the differences in research systems, based on these conventional hazard identification methods, scholars have proposed a series of new hazard identification methods that are more suitable for the research system. Based on the results of accident analysis and interviews, Nascimento F et al. applied grounded theory and template analysis to compile a list of hazards affecting pilots’ night flight capabilities [4]. With the help of safety specialists' experiences, Alizadehsalehi S et al. used BIM software used in the design of the structure to identify potential safety hazards in buildings [5]. Chen RC et al. passed a multivariate Cox regression analysis and a nomogram model to identify potential hazards related to the fatal outcome of COVID-19 [6].

In the research on the identification of hazards in tailings dam, scholars have done a lot of research work. Based on the e-EcoRisk database, Rico M et al. analyzed 147 cases of tailing pond accidents around the world, and found 15 reasons for tailing dam failure [7]. Li Zhaodong et al. established a checklist of factors affecting the tailings dam accidents and assigned points to it, identified dangerous and harmful factors, and evaluated the safety of the tailings dam [3]. Pier-Luc Labonté-Raymond et al. have studied the impact of climate change on the drainage system of tailings ponds [8]. MG Lemos et al. identified the chemical, mineralogical and metallurgical properties of gold tailings located in the Santa Barbara mine [9]. Baker K E et al. applied process safety management tools to the tailings storage and transportation system and visually characterized the possible hazards and control measures to prevent accidents [10]. The safety management of tailings dam is a whole-process management, and the hazards are coupled and influenced by each other during the whole life cycle. Therefore, the above methods are difficult to complete and systematically identify the hazards of tailings dams. In order to overcome these problems, facing the life cycle of tailings ponds and combining the four influencing factors of natural factors, design factors, construction factors and management factors, Zhao Yiqing et al. proposed the process-causing grid method to identify hazards of tailings ponds. Although the process-causing grid method can identify the hazards of tailings dams relatively completely and systematically, it relies more on the subjective judgment of researchers, and the supporting evidence for the identification of hazards is not clear [11].

Complex networks can well characterize the internal relationship between research objects (nodes) [12], and therefore, have been widely used in many fields in recent years [13–18]. Most complex networks are scale-free, and a small number of hub nodes play a leading role in the operation of the network [19]. In order to identify the key nodes in the complex network, Yu E Y et al. generated a feature matrix for each node in the network, and used a convolutional neural network to train and predict the influence of the node [20]. Hou B et al. used the all-around distance method to find influential nodes in complex networks [21]. AXZ et al. used the information transfer probability between any pair of nodes and the k-medoid clustering algorithm to identify influential nodes in complex networks with community structure [22]. Freeman LC etc. defined centrality in terms of the degree to which a point falls on the shortest path between others [23]. In order to rank the spreaders, an average shortest path centrality is proposed [24]. Qin Xuan et al. applied the centrality analysis of the complex network to the study on the risk of tailings pond
accidents. By comparing the decline in network efficiency after deleting the high degree value, high
closeness centrality and high betweenness centrality nodes, it is concluded that the nodes with high
betweenness centrality have a stronger dominance effect on the accident risk network of a tailings pond
[25].

The key spreading hazards (KSN) obtained based on the complex network theory do not consider the
severity of the hazards, and these hazards may be different from the real accident hazards. If these
hazards are evaluated and graded, the actual impact of these hazards on the accident can be determined.
The evaluation and classification methods of hazards are mostly safety evaluation methods. Wu Qi et al.
firstly established a leakage accident risk assessment index system, and then used the analytic hierarchy
process and the fuzzy comprehensive evaluation method to quantify the influencing factors of the
accident risk, and finally calculated the hazards level [26]. Shi Zongbao et al. have redefined safety
hazards and put forward a more reasonable classification standard for safety hazards [27]. In the process
of risk assessment, Zhao Dongfeng et al. used the consequences of accidents to approximate the
consequences of hazards, and solved the problem of risk classification of specific hazards [28]. Tta B et
al. used epigenetic biomarkers as a tool to assess chemical hazards [29].

In order to solve the above problems, this paper proposes an evidence-based three-dimensional hazard
identification framework method to identify the hazards and paths of tailings dam failure. Then, the
complex network theory is used to establish a propagation network of tailings dam failure risk (PNTDFR),
and some structural characteristics and characteristics of the network are analyzed. Taking the network
efficiency as the measurement index of risk communication capability, the MCHRM proposed in this
paper is used to find the key hazard nodes (KSN) that may cause the failure of the target tailings dam.
After the KSN is confirmed, by confirming the trigger state of the KSN, the key hazards and paths of the
tailings dam failure can be obtained. Finally, the above method was applied to the Feijão Dam I, and
compared with the conclusion of the Dam I failure investigation team to verify the accuracy of the
method in this paper.

2. Research Method

2.1 Hazard identification and network establishment

The 'hazard' is the potential occurrence of an event within a prescribed time and space, and its definition
has been expanded as a process, phenomenon or human activity [36]. In order to avoid the subjectivity of
hazard identification, this paper proposes a new hazard identification method from the perspective of
safe production: a three-dimensional hazard identification framework (THIF). This method selects
accident cases, laws and regulations, standard specifications, documents and other materials as
evidence for hazard identification, and systematically identify the hazards of the personnel, material,
environment, and management in tailings dams based on the life cycle of the construction, operation,
closure, and reclamation of tailings dams [38].
This paper uses the identifying hazards of tailings dams and the evolutionary relationship between hazards to construct an adjacency matrix, and then import the adjacency matrix into Pajek software, and construct a propagation network of tailings dam failure risk (PNTDFR). The nodes in the PNTDFR represent hazards, and the edges represent the relationship between hazards. According to the status change of the hazards, the PNTDFR is divided into three layers of nodes (dormant hazard, armed hazard, activity hazard or accident) and two stages and two stages (from dormant hazard to armed hazard, from armed hazard to activity hazard)\(^\text{[30,38]}\). The initial dormant hazards can only cause other hazards and cannot be caused by other hazards, that is, the in-degree value is 0, including all initial nodes of the four influencing factors of tailings dam break, such as floods, excessive rainfall, and excessive standard earthquakes. Armed hazards are formed by the evolution of the dormant hazards, and these armed hazards will may cause damage accidents under certain working environments or conditions, such as the rapid rise of pond water level, the dam deformation, and the tailings liquefaction. These hazards mean the imminent accidents and disasters. Active hazards are accidents that are or have occurred. If these active hazards cannot be effectively suppressed, they will lead to serious consequences and disasters, including overtopping and dam break and so on\(^\text{[24]}\).

When the network model is established, we can use complex network theory to analyze the statistical features of the PNTDFR, such as degree, betweenness centrality, network density, characteristic path length and clustering coefficient. From these characteristics, the propagation law of tailings dam failure risk. can be analyzed and discovered.

### 2.2 Global network efficiency and priority remediation order of hazards nodes

When the PNTDFR is a scale-free network, the PNTDFR will appear vulnerable to deliberate attacks\(^\text{[31]}\). In other words, if we can prioritize to remedy the hazard nodes that have a greater impact on network connectivity, the spreading efficiency of the network can be reduced, thereby slowing down or even blocking the spread of risks. Therefore, this paper chooses network efficiency as an index to measure the spreading ability of dam-break risk.

Global network efficiency, also known as network connectivity, refers to the difficulty of average network connectivity, which is the average of the sum of the reciprocal lengths of the shortest path between all pairs of hazard nodes in the entire network\(^\text{[31]}\). Degree centrality, betweenness centrality and closeness centrality are commonly used methods to characterize the importance of nodes in complex networks. In this paper, the importance of nodes determined according to the three methods is used as the priority of hazard remediation (deletion), and then the differences of the three methods in reducing network efficiency are compared. By absorbing the advantages of different methods under different hazard remediation (deleted) ratios, combined with the characteristics of multi-stage propagation of tailings dam failure risk, this paper proposes a multi-stage collaborative hazard remediation method ((MCHRM)) to
determine the importance of hazard nodes. The specific implementation process of this method is as follows:

(1) Since the first-layer nodes (dormant hazards) only have out-degree values, and the betweenness centrality is 0, only the degree value needs to be considered in determining the remediation order of the first-layer hazards, and priority is given to the hazard nodes with greater degree value.

(2) The second-layer nodes (armed hazards) have degree values, betweenness centrality and closeness centrality, which are the intermediate stage of dormant hazards and activity hazards. Therefore, it is necessary to consider the influence of three indicators on risk propagation at the same time. When there are differences among three hazard remediation methods under different remediation proportions, priority is given to the remediation method that can reduce the speed of risk evolution faster.

(3) The third-layer nodes (activity hazards) are the possible accident modes of a tailings dam, and the remediation method is the same as that of the second-layer node. The hazard of dam break is the object of the accident studied in this paper, so it is not remedy.

(4) After the remediation priority of hazards at the same layer according to the corresponding methods is determined, among hazard nodes at different layers, those nodes with a smaller remediation proportion will be prioritized.

When all the hazard nodes of the PNTDFR have been treated, by observing the change trend of network efficiency, the key hazards of the PNTDFR can be determined (those hazards that can significantly reduce network efficiency after deleting). In this paper, these important hazard nodes are called key propagation hazards (KPH). In addition, if the MCHRM can reduce network efficiency more effectively than the commonly used methods in the past, the remediation order of hazard nodes determined by this method can better characterize the importance of different nodes in the PNTDFR.

### 2.3 Evaluation and classification of key hazards

The KPH refers to some important hazards that may occur based on the environment, structure, and management level of the target tailings dam. If you want to determine which of the hazards caused the accident, you need to determine whether these hazards are in a triggered state and how serious. Because the China Tailings Pond Safety Grade Classification Standard divides the tailings ponds into four levels: normal, mild, moderate, and dangerous, the paper divides the grading standards of the KPH indicators of tailings dam into four levels combining the Technical Regulations for Safety of Tailings Pond and the Code for Design of Tailings Facilities. Level 1 is a normal state, level 2 is a mild danger, level 3 is a moderate danger, and level 4 is a serious danger. In the classification of grading standards, the indicators that can obtain specific values are classified using quantitative analysis methods. For example, the evaluation indicator of hazard 5 (heavy rainfall) is rainfall, which is calculated in the depth of the water layer per unit area within 24 hours. Hazards that are difficult to quantitatively classify are qualitatively used. For example, hazards 355 (Insufficient experience in personnel or organization qualification
problems) are divided into four levels based on the personnel’s education, working hours, and qualification levels of the institution.

When the classification standard of the KPH is completed, by comparing the monitoring data, daily inspection results, and safety evaluation information before the accident, the level of the KPH indicators of the studied tailings dam can be obtained, so as to determine the states of these hazards [37]. The KPH of level 1 are in a normal state and will not further evolve or cause other hazards. This part of the hazards is not the KPH that causes the tailings dam to break. The remaining hazards with a level greater than 1 are the KPH that led to the dam failure. By excluding the hazard nodes of level 1, we can determine the key hazards and spreading paths between hazards. In the accident investigation report, these key hazards are also referred to as the main cause of the accident.

3. Case Analysis

This paper takes Feijão Dam I in Brazil as a case. The dam crest elevation of Dam I is 942 m, the maximum height is 86 m, and the dam crest length is 720 m. The height of each sub-dam varies from 5 m to 18 m. The slope of the upstream and downstream slopes is between 1:2.5 and 1:1.5, and the other slopes of the dam body generally adopt a slope of 1:2. After 2013, Dam I stopped the construction of its tailings dam. Later, in July 2016, the stockpiling of tailings was stopped and the tailings pond was closed. More information about Dam I can be found in Report of the Expert Panel on the Technical Causes of the Failure of Dam I [33].

3.1 Classification of hazards and the relationship between Hazards

A total of 117 hazards and 535 relationships are obtained by the THIF method, as shown in Appendix A [38]. In Appendix A, the first column indicates the categories of hazards, including four categories: environment factor, personnel factor, material factor, and management factor. The second column indicates the number (ID) of the hazards in the third column. The fourth column indicates the number of the hazards caused by the hazard in the third column. For example, the hazard named ‘heavy rainfall’ in the second row of the third column is numbered 5, which belongs to the environment factor. Through the THIF method, we can get the hazards that may be caused by the ‘heavy rainfall’. These hazards are numbered 19, 67, 69, 150, 193 and 19.

3.2 Propagation network of Dam I failure risk

This section uses hazards of Dam I and the relationship between the hazards in Appendix A to construct the adjacency matrix, and then import it into Pajek software to construct the propagation network of Dam I failure risk (I-FRPN), as shown in Fig. 1.

3.2.1 Degree and degree distribution
The degree value of each node in I-FRPN can be obtained through Pajek complex network software as shown in Fig. 2. The average degree of the I-FRPN is 9.15, and the network density is 0.04, indicating that a hazard node is directly related to 9.15 hazard nodes on average, but the overall density of the I-FRPN is not large.

It can be seen from Fig. 2 that among the top 10 hazards, 355 (Insufficient experience in personnel or organization qualification problems) is the hazard node with the largest degree value in the I-FRPN, which directly affects 61 hazards. It shows that if the personnel and organization do not have sufficient experience or do not meet the corresponding qualification requirements, the tailings dam will always be threatened throughout its life cycle. 191 (Fracture of drainage structure) is directly related to 36 hazards, which is the second largest hazard in the degree value. It is classified as a material factor among the four influencing factors. The degree values of 62 (partial landslide and collapse of the dam), 64 (Dam instability), 65 (Dam deformation), 157 (Filter failure), 195 (Rapid rise of pond water level) and 327 (Safety monitoring facilities cannot fully reflect the operating status of the tailings pond) are respectively 22, 31, 26, 27, 24 and 24. These hazards belong to the material factor together with the hazard 191, and account for 70% of the top 10 hazards, highlighting the fact that the material factor plays a leading role in tailings dam safety management.

Hazard 308 (Closure design not in accordance with regulations) has a degree value of 25, which belongs to the same personnel factor as hazard 355, and these hazards are indirect factors that lead to dam break. 351 (Improper maintenance) is directly related to 24 hazards, which is the management factor, indicating that management plays an important role in the safety management of tailings dams.

From the point of view of out degree, the values of hazards 355 (Insufficient experience in personnel or organization qualification problems), 308 (Closure design not in accordance with regulations), 351 (Improper maintenance), 2 (Flood) and 312 (Dam body remediation does not meet the requirements) are respectively 61, 24, 23, 19, and 16, which are the five nodes with the largest out-degree value, indicating that personnel factors, management factors, and environmental factors are more likely to cause other hazards. 191 (fracture of drainage structure), 62 (partial landslide and collapse of dam), 64 (dam instability), 65 (dam deformation), and 157 (failure of water filter body) are the 5 hazards with the highest in-degree value, and in-degree values are respectively 31, 29, 21, 21, and 21. These hazards all are material factors, indicating that material factors are prone to form armed hazards under the influence of dormant hazards.

Cumulative degree distribution of the I-FRPN is shown in Fig. 3. The cumulative degree distribution presents a power-law distribution that has the approximate fit $P(k) = 3.7179x^{-1.285}$ ($R^2 = 0.8101$). The above result deviates from the power-law nature for larger k, which indicates that the I-FRPN has scale-free property. It means that a few hub nodes play a dominant role in the I-FRPN. If we can find these key nodes, the spread of risk can be slowed down or even blocked, thus preventing the occurrence of dam break. The degree studied in this section is an important indicator for judging the importance of network nodes. In addition, there are also indicators such as betweenness centrality and
closeness centrality that are also commonly used to measure the importance of nodes. In the next section, we will conduct more analysis on this aspect.

### 3.2.2 Network diameter and average path length

The network diameter, also known as the maximum path length of the network, represents the largest step length between two nodes in the network \(^{31}\). After calculation, the network diameter of the I-FRPN is 8, which means that a hazard node can affect any node in the network only after a maximum of 8 steps. The most distant node pairs of the network are v32 and v150 or v7 and v45. Compared with some accident networks studied in the past \(^{15, 34, 35}\), the diameter of I-FRPN is larger, and the evolution path of the risk is complicated.

The characteristic path length is also called the average path length. After calculation, the average path length of the I-FRPN is 2.81, indicating that it takes less than 3 steps on average to transfer the risk of dam break from one hazard to another hazard. The above results show that the characteristic path length of the I-FRPN is small, and the risk of dam break can be spread quickly on the network. If no corresponding measures are taken, the emergence of a serious hazard may cause a tailings dam break in a relatively short time.

### 3.2.3 Clustering coefficient and small-world property

The clustering coefficient of the I-FRPN refers to the degree of interconnection between adjacent nodes of a hazard node in the network \(^{34}\). That is to say, there is no clustering coefficient for nodes with a degree value of 1. In this paper, the average clustering coefficient of the I-FRPN is calculated by Pajek software as 0.15. After excluding the nodes with a degree of 1, the clustering coefficients of the hazard nodes in the network are obtained, as shown in Fig. 4. It can be seen from the figure that the clustering coefficient of the hazard node in the I-FRPN is between 0-0.5. The clustering coefficients of hazard 32 (Insufficient tank length) and 220 (The maximum flow rate of flood control structure design is greater than the allowable flow rate of building materials) are both 0.5, which are the nodes with the largest clustering coefficient, indicating that the adjacent hazards of the hazard 32 and 220 have a strong correlation and show strong clustering.

Small-world networks usually have large clustering coefficients and small characteristic path lengths \(^{31}\). In order to judge whether the clustering coefficient of the I-FRPN meets the requirements of the small world, this paper constructs a random network with the same number of nodes and the same degree value as the I-FRPN, and calculates the clustering coefficient to be 0.08, which is smaller than the clustering coefficient of the I-FRPN (0.15). The equal-sized dam failure risk random network is shown in Fig. 5. Combined with the characteristic path length of the I-FRPN is only 2.81, it can be concluded that the I-FRPN has small-world property. In other words, the break accident for Dam I has the characteristics of multi-factor coupling and short disaster path.

### 3.3 Priority remediation order of hazard nodes in the I-FRPN
This paper first treats the node with the largest index value and calculates the network efficiency, and then calculates the network efficiency after every 5 hazard nodes are treated. Figure 6 shows the changes of the network efficiency under the hazard remediation methods.

In Fig. 6, it can be found that the preferential treatment of nodes with large betweenness centrality can achieve better results in the early stage (low proportion). In other words, when the remediation proportion of hazard nodes is small, the risk propagation speed can be reduced more quickly by the betweenness centrality. However, when the proportion of hazard remediation reaches 13.68%, the hazard node with a higher degree value will have a better effect of reducing risk spread.

It can be seen from Fig. 6 that the MCHRM performs better than the other three commonly used methods, whether in the early stage of the hazard node remediation or in other stages. In addition, we can also find that all four methods show that when the proportion of node remediation reaches about 30%, the decline in network efficiency tends to slow down significantly. Further increasing the proportion of node remediation will not significantly reduce the spread efficiency of the network. In other words, when we are in the process of hazard remediation of tailings dams, if we give priority to the top 30% of hazard nodes determined by the MCHRM, we can use the vulnerability of the network to reduce network efficiency more quickly. In this paper, these hazard nodes that can quickly reduce the propagation efficiency of the I-FRPN are called key propagation nodes, and the relationship between these key nodes is called the critical propagation path.

3.4 Evaluation and classification of key hazards of Feijão Dam I

In order to improve the accuracy of the KPH identification, when determining the range of KPH, this paper will increase the priority remediation range from the top 30% of the index value to the top 45%. The I-FRPN has a total of 117 hazard nodes, and 45% of the priority g remediation proportion includes 53 nodes. According to the KPH determined by MCHRM, the order of priority remediation is shown in Table 1. The first column of Table 1 is the serial number of the KPH, indicating the order of the remediation of the hazards. The third column is the name of the hazard to be studied, the second column is the number (ID) of the hazard, the sixth column is the level of the corresponding hazard node, and the fourth and fifth columns are the degree value and the betweenness centrality of the hazard node.

| Table 1 |
| Key propagation hazards |
| Sequence number | Node number | Node name                                                                 | degree centrality | betweenness centrality | Hazard level |
|-----------------|------------|---------------------------------------------------------------------------|-------------------|------------------------|--------------|
| 1               | 195        | Rapid rise of pond water level                                            | 24                | 0.05123                | 2            |
| 2               | 64         | Dam instability                                                           | 31                | 0.0035                 | 3            |
| 3               | 65         | Dam deformation                                                           | 26                | 0.0356                 | 3            |
| 4               | 157        | Filter failure                                                            | 27                | 0.0370                 | 3            |
| 5               | 191        | Fracture of drainage structure                                            | 36                | 0.0457                 | 2            |
| 6               | 192        | Leaking drainage structure                                                | 21                | 0.0299                 | 2            |
| 7               | 158        | Leakage channel                                                           | 16                | 0.0284                 | 3            |
| 8               | 267        | Pipes and grooves deformation                                             | 22                | 0.0243                 | 1            |
| 9               | 308        | Closure design not in accordance with regulations                         | 25                | 0.0004                 | 1            |
| 10              | 327        | Safety monitoring facilities cannot fully reflect the operating status of the tailings pond | 24                | 0.0223                 | 3            |
| 11              | 355        | Insufficient experience in personnel or organization qualification problems | 61                | 0.0000                 | 2            |
| 12              | 62         | Local landslide and collapse of the dam                                   | 22                | 0.0014                 | 3            |
| 13              | 68         | Uneven settlement of the dam                                              | 20                | 0.0199                 | 3            |
| 14              | 66         | Dam crack                                                                 | 22                | 0.0145                 | 2            |
| 15              | 69         | Scour the dam                                                             | 16                | 0.0185                 | 1            |
| 16              | 351        | Improper maintenance                                                      | 24                | 0.0003                 | 1            |
| 17              | 67         | Dam surface water saturation                                              | 18                | 0.0139                 | 3            |
| 18              | 70         | Tailings liquefaction                                                     | 16                | 0.0123                 | 4            |
| 19              | 193        | Scour or cavitation drainage structures                                    | 21                | 0.0094                 | 1            |
| 20              | 200        | Insufficient flood discharge capacity                                      | 16                | 0.0101                 | 3            |
| 21              | 234        | Blockage or siltation                                                     | 16                | 0.0166                 | 2            |
| 22              | 2          | Flood                                                                     | 19                | 0.0000                 | 1            |
| 23              | 73         | Poor stability of tailings dam slope                                       | 18                | 0.0074                 | 3            |
| 24              | 136        | Dam foundation instability                                                | 13                | 0.0080                 | 3            |
| Sequence number | Node number | Node name                                                                 | degree centrality | betweenness centrality | Hazard level |
|-----------------|-------------|---------------------------------------------------------------------------|-------------------|------------------------|--------------|
| 25              | 238         | Serious corrosion of equipment                                             | 8                 | 0.0090                 | 1            |
| 26              | 312         | Dam body renovation does not meet the requirements                        | 18                | 0.0017                 | 1            |
| 27              | 5           | Heavy rainfall                                                            | 6                 | 0.0000                 | 3            |
| 28              | 39          | Insufficient storage capacity of tailings pond                            | 9                 | 0.0074                 | 1            |
| 29              | 135         | Uneven foundation subsidence                                               | 15                | 0.0062                 | 1            |
| 30              | 167         | Seepage line is higher than control seepage line                          | 12                | 0.0076                 | 2            |
| 31              | 325         | Monitoring instrument failure, work interruption                          | 7                 | 0.0073                 | 1            |
| 32              | 19          | Landslides in the tailings pond                                           | 14                | 0.0061                 | 1            |
| 33              | 61          | Poor control of tailings deposits                                         | 10                | 0.0041                 | 1            |
| 34              | 343         | Inadequate safety evaluation                                               | 12                | 0.0033                 | 1            |
| 35              | 346         | Improper data management                                                  | 15                | 0.0000                 | 1            |
| 36              | 347         | Insufficient or wrong hydrological and geological data                     | 15                | 0.0000                 | 1            |
| 37              | 45          | Tailings particle size/gradation does not meet the requirements           | 7                 | 0.0023                 | 2            |
| 38              | 75          | Improper calculation method of tailings dam stability                      | 10                | 0.0003                 | 2            |
| 39              | 183         | Filter failure                                                            | 7                 | 0.0031                 | 2            |
| 40              | 273         | Subsidence or deformation of supporting facilities such as pipes, trenches and tunnels | 9 | 0.0029 | 1 |
| 41              | 11          | Liquefied soil, soft clay and collapsible loess foundation                | 5                 | 0.0000                 | 2            |
| 42              | 156         | Leakage damage                                                            | 13                | 0.0016                 | 4            |
| 43              | 190         | Overtopping                                                               | 12                | 0.0128                 | 1            |
| 44              | 126         | Unreasonable design of cast-in-place protective surface                   | 10                | 0.0001                 | 1            |
| 45              | 307         | Pump failure                                                              | 9                 | 0.0017                 | 1            |
| 46              | 352         | Design defects of emergency plan                                          | 10                | 0.0006                 | 3            |
Using the ‘grading standards for KPH indicators for tailings dams’ proposed in the 2.3 section, the KPH indicators for Dam I are classified, as shown in Appendix B.

By consulting the monitoring data, daily inspection results and safety evaluation information of each hazard before the failure of Dam I, according to the grading standards of hazard indicators in Appendix B, the level of each hazard is obtained, as shown in Table 1.

By excluding the hazard nodes of level 1 in the normal state, we can determine that there are 31 key hazards in the failure accident of Feijão Dam I. Combining the evolution relationship among the hazards based on evidence in Appendix A, we can obtain the 240 propagation paths between key hazards. The key hazards and propagation paths of Dam I failure are shown in Fig. 7.

### 4. Comparison And Analysis

In order to verify whether the key hazards (causes) of the Dam I failure accident identified above are reasonable, this paper compares the above results with the conclusions made by the accident investigation expert group chaired by Dr. Peter K. Robertson. The expert group concluded that the direct cause of the failure of the Dam I was the liquefaction of the tailings of the dam. The expert group conducted research on the composition of the dam body material and the dam-break trigger mechanism, and found that 6 technical problems were the main factors leading to the dam break. Compare the key hazards with a level greater than 1 in Table 2 with the main factors found by the expert group, as shown in Table 2 [33].

| Sequence number | Node number | Node name                                             | degree centrality | betweenness centrality | Hazard level |
|-----------------|-------------|-------------------------------------------------------|-------------------|------------------------|--------------|
| 47              | 47          | Excessive tailings unit weight                        | 4                 | 0.0012                 | 4            |
| 48              | 49          | Strongly corrosive tailings                           | 2                 | 0.0010                 | 1            |
| 49              | 77          | The tailings dam slope ratio is unreasonable           | 9                 | 0.0009                 | 3            |
| 50              | 309         | Close the tailings pond without understanding the hazards and risks | 7                 | 0.0010                 | 1            |
| 51              | 313         | The improvement of flood discharge system does not meet the requirements | 9                 | 0.0013                 | 2            |
| 52              | 82          | The dam layout is unreasonable (the location sub dam and primary dam) | 7                 | 0.0008                 | 3            |
| 53              | 25          | There are mining activities near the site             | 4                 | 0.0000                 | 1            |

Table 2
Key hazard comparison table

| Technical problem                                                                 | Node number | Node name                                                                 |
|-----------------------------------------------------------------------------------|-------------|---------------------------------------------------------------------------|
| (1) A design that resulted in a steep upstream constructed slope                    | 77          | The tailings dam slope ratio is unreasonable                                |
| (2) Water management within the tailings impoundment that at times allowed ponded  | 195         | Rapid rise of pond water level                                             |
|   water to get close to the crest of the dam, resulting in the deposition of weak   |             |                                                                           |
|   tailings near the crest                                                          |             |                                                                           |
| (3) A setback in the design that pushed the upper portions of the slope over       | 82          | The dam layout is unreasonable                                             |
|   weaker fine tailings                                                            |             |                                                                           |
| (4) A lack of significant internal drainage that resulted in a persistently high    | 200/167     | Insufficient flood discharge capacity / Seepage line is higher than control seepage line |
|   water level in the dam, particularly in the toe region                           |             |                                                                           |
| (5) High iron content, resulting in heavy tailings with bonding between particles. | 47          | Excessive tailings unit weight                                              |
|   This bonding created stiff tailings that were potentially very brittle if triggered to become undrained |
| (6) High and intense regional wet season rainfall that can result in significant   | 5/70        | Heavy rainfall / Tailings liquefaction                                      |
|   loss of suction, producing a small loss of strength in the unsaturated materials |             |                                                                           |
|   above the water level                                                           |             |                                                                           |

Through comparison, it can be found that the main reasons for the failure of the Dam I proposed by the expert group are 6 aspects, involving 8 hazard nodes. When the key nodes identified by the MCHRM are used as the priority remediation criteria (top 30%), the hazard nodes 5, 70, 167, 195, and 200 are consistent with the causes of the dam failure mentioned in the expert group’s conclusion, accounting for 62.5% of the 8 hazards; when the priority remediation range is increased to the top 45% of hazard nodes, hazard nodes 47, 77 and 82 are also included.

The above comparison results show that the method proposed in this paper can better find the key causes of the dam failure. When the priority range of remediation is increased by 15%, it will be possible to cover all the expert groups to propose the main causes. Although the conclusions of the expert group cannot be completely equated with the true cause and risk propagation path of Dam I failure, the expert group members have rich experience and outstanding academic attainments on the issue of tailing dam failure. Therefore, expert group’s conclusion is highly reliable. In addition, the failure causes and risk propagation paths of the Dam I identified in this paper also involve some hazard nodes and propagation paths that the expert group did not mention, which may include some problems that the expert group did not notice, which will help improve the safety management of tailings dams.

5. Discussion
The causes of tailings dam failure are complex, and the dam structure changes continuously over time, which makes it difficult to identify hazards and analyze the causes of accidents. In order to solve this problem, this paper proposes a new method for identifying the key hazards and the risk propagation path of tailings dam failure. This method is divided into four steps: preliminary identification of hazards of tailings dam failure, network model construction and analysis, identifying key propagation hazards and their importance, and evaluation and classification of key hazards.

The biggest difference between the THIF method proposed in the preliminary identification stage and the previous hazard identification and accident cause analysis methods is that the THIF method uses accident cases, laws and regulations, documents and media as evidence of the existence of hazards and the relationship between hazards. To a certain extent, the method avoids the subjectivity of researchers in the process of identifying hazards and the relationship between hazards.

Compared with commonly used methods such as accident trees and accident chains, complex networks can more completely and systematically link these evidence-based hazards and the relationship between hazards, and characterize the evolution process of dam-break risk in the form of a network. The centrality analysis of nodes is an important direction of complex network research, and its purpose is to find the hub nodes that play a dominant role in the operation of the network system. For the PNTDFR, the hub nodes are the key hazards for the spread of dam failure risk. If key hazard nodes can be found, and timely remediation measures can be taken, the spread of the risk of dam break can be blocked, so as to avoid the occurrence of tailings dam failure.

By utilizing the advantages of different methods under different hazard remediation (deleted) proportions in finding hub nodes, this paper proposes the MCHRM. The MCHRM can significantly reduce the network efficiency, but there are also the problems that the severity or level of the hazards is not considered, and the weights between nodes in the network are assumed to be equal, which will lead to a certain difference between identified key hazards and real key hazards from tailings dam failure. At the same time, due to the complex causes of dam failure accidents and the difficulty of quantification, it is difficult to accurately give the weight of the relationship between hazards. In order to solve the above problems, this paper sets a certain reserve range when determining the range of key nodes in the PNTDFR, that is, increases the range of priority remediation. The specific reserve range can be adjusted to a certain extent according to the difference of the research objects.

Although this paper has done a lot of work in order to find the key hazards and the risk propagation paths of tailings dam failure, there are still three shortcomings: a. Although the reserve range of priority remediation can cover all key hazards of dam failure, it is difficult to give an accurate reserve ratio, and the actual application needs to combine the experience of some technical personnel. b. In the formulation of hazard classification standards, due to the numerous influencing factors of hazards and the difficulty of quantifying some of the influencing factors, the classification standards of some nodes adopt a subjective qualitative classification method, which affects the accuracy of some classification indicators. c. The hazard nodes and the relationships between hazards in this paper are all based on evidence
(accident cases, laws and regulations, documents and media, etc.), but the reliability of different evidences is different, which will affect the accuracy of the research. In order to better solve the above problems, the author of this paper plans to study more accident cases in the next step, so as to determine a more specific reserve range of priority remediation. At the same time, this paper will consider the evidence according to the reliability of the evidence, and select more quantitative indicators to classify the hazard indicators to improve the practicability of the above methods.

6. Conclusion

Based on accident cases, laws and regulations, and documents, this paper systematically has identified the hazards of each stage of whole life cycle in accordance with personnel, material, environment, and management factors. The hazard identification method is called the three-dimensional hazard identification framework, and by the method, 117 hazards and 535 relationships between hazards were obtained in the Dam I. This paper uses the complex network theory to propose the PNTDFR with three-layer nodes and two stages, and it is applied to the Dam I. By analyzing the characteristics of the I-FRPN, it can be obtained that the I-FRPN is a small-world and scale-free network.

By absorbing the advantages of betweenness centrality and degree centrality under different remediation proportion of hazard nodes in finding key hazard nodes, combined with the three-layer and two-stage characteristics of the PNTDFR, this paper proposes the MCHRM to identify the KPH and the priority remediation order among the KPH. By analyzing the I-FRPN, it can be found that the top 30% of the index value is the KPH. When the priority remediation range is increased from 30–45%, the KPH will cover all the causes of accidents proposed by the Dam I failure investigation expert group.

In this paper, by formulating the classification standards of KPH, comparing the monitoring data, daily inspection results and safety evaluation information of each hazard before the accident, it is determined that a total of 31 KPH are in the trigger state before the Dam I failure. These triggered hazards are the key hazards of the Dam I failure risk, and the relationships between the key hazards are the propagation paths of the failure risk.

Appendix

Appendix A. List of hazards of tailings dam failure
| Impact factors | Number | Name of hazards or factors | Number of hazards caused |
|----------------|--------|-----------------------------|--------------------------|
| Environment factor | 2      | Flood                       | 19, 60, 62, 64, 65, 66, 67, 69, 150, 156, 158, 167, 190, 191, 192, 193, 195, 273, 325 |
|                 | 5      | Heavy rainfall              | 19, 67, 69, 150, 193, 195 |
|                 | 10     | Gravel foundation           | 157                      |
|                 | 11     | Liquefied soil, soft clay and collapsible loess foundation | 68, 70, 135-136, 157 |
|                 | 19     | Landslides in the tailings pond | 39, 195               |
|                 | 25     | There are mining activities near the site | 19, 62, 64, 66 |
|                 | 32     | Insufficient impoundment length (upstream wet tailings impoundment) | 39 |
|                 | 34     | Large catchment area        | 195                      |
| Material factor | 39     | Insufficient storage capacity of tailings pond | 190 |
|                 | 45     | Tailings particle size/gradation does not meet the requirements | 47, 66, 68, 61, 234 |
|                 | 47     | Excessive tailings unit weight | 68, 61 |
|                 | 49     | Strongly corrosive tailings | 238                      |
|                 | 60     | Dam break                   |                           |
|                 | 62     | Local landslide and collapse of the dam | 60 |
|                 | 64     | Dam instability             | 60, 62                    |
|                 | 65     | Dam deformation             | 62, 64, 157, 267, 273    |
|                 | 66     | Dam crack                   | 62, 64, 73, 158          |
|                 | 67     | Dam surface water saturation | 62, 64-66, 73, 157       |
| Page | Description | References |
|------|-------------|------------|
| 68   | Uneven settlement of the dam | 62, 64-66, 191-192, 267, 273 |
| 69   | Scour the dam | 62, 64-66 |
| 70   | Tailings liquefaction | 62, 64, 68, 136, 156-158 |
| 73   | Poor stability of tailings dam slope | 62, 64 |
| 77   | The tailings dam slope ratio is unreasonable | 62, 64-65, 73, 157 |
| 78   | Unreasonable width of dam crest | 62, 64-65, 157 |
| 79   | Improper dam type selection for the initial dam | 39, 64, 157 |
| 80   | The height of initial dam is unreasonable | 39, 64-65, 73, 81, 194 |
| 81   | The ratio of the initial dam height to the total dam height of the upstream tailings dam is unreasonable | 64-65, 73 |
| 82   | The dam layout is unreasonable (the location sub dam and primary dam) | 32, 39, 69, 73, 135 |
| 61   | Poor control of tailings deposits | 64-65, 68, 77, 152, 157 |
| 92   | The tailings dam is not equipped with anti-scouring measures | 69, 82 |
| 122  | There is a horizontal weld on the slope | 64, 66, 73 |
| 132  | No effective filter layer is set on the dam foundation | 157 |
| 135  | Uneven foundation subsidence | 66, 68, 73, 136, 191, 267, 273 |
| 136  | Dam foundation instability | 64-66, 68, 73 |
| 149  | The length or thickness of the horizontal paving | 157 |
front of the dam is insufficient

| Page | Description                                                                 | Reference(s) |
|------|-----------------------------------------------------------------------------|---------------|
| 150  | Natural paving (covering) is destroyed                                       | 158           |
| 152  | Poor deposition control for dry beach face                                   | 157           |
| 156  | Leakage damage                                                               | 60, 62, 64    |
| 157  | Filter failure                                                               | 64, 67, 136, 156, 167, 195 |
| 158  | Leakage channel                                                              | 64, 68, 135-136, 156 |
| 167  | Seepage line is higher than control seepage line                             | 65-67, 156    |
| 176  | Poor drainage of composite geotechnical drainage network                     | 157           |
| 182  | Unqualified filter material                                                  | 183           |
| 183  | Filter failure                                                               | 65, 157       |
| 190  | Overtopping                                                                 | 60, 62, 64, 69 |
| 191  | Fracture of drainage structure                                              | 66, 69, 158, 192, 200 |
| 192  | Leaking drainage structure                                                  | 66-67, 69, 150, 158, 195, 200 |
| 193  | Scour or cavitation drainage structures                                     | 191-192       |
| 194  | Insufficient regulating water storage                                       | 39            |
| 195  | Rapid rise of pond water level                                              | 39, 65, 67, 152, 167, 190, 194 |
| 197  | The foundation pit at the higher groundwater level has no drainage facilities | 195, 200      |
| 198  | The flood drainage system does not match the dam construction method         | 191, 200      |
| Code | Issue                                                                 | References   |
|------|----------------------------------------------------------------------|--------------|
| 200  | Insufficient flood discharge capacity                                 | 193, 195     |
| 206  | Insufficient elevation of drainage holes in front of the dam         | 200          |
| 207  | Flood drainage structures are directly located on the tailings sediment beach | 191          |
| 209  | Insufficient foundation bearing capacity of underground flood drainage structures | 191          |
| 210  | Improper installation of flood interception and drainage facilities  | 191, 200     |
| 218  | Improper installation of energy dissipation facilities               | 191, 193     |
| 219  | No energy dissipation measures have been taken in the tailings facility | 191, 193     |
| 220  | The maximum flow rate of flood is greater than the allowable flow rate of the building materials | 191, 193     |
| 221  | The clarified water of the tailings pond is not used for backwater utilization | 195          |
| 234  | Blockage or siltation                                                | 176, 191, 195, 200 |
| 238  | Serious corrosion of equipment                                       | 191, 325     |
| 240  | No anti-corrosion treatment in tailings facilities                   | 238          |
| 241  | Unqualified anti-corrosion materials                                  | 193, 238     |
| 260  | Improper handling of local hydraulic                                 | 234, 238, 267 |
| Page | Phenomena                                                                 | References        |
|------|---------------------------------------------------------------------------|-------------------|
| 267  | Pipes and grooves deformation                                             | 191, 193, 234     |
| 268  | Defects of the interception ring in pipe body                            | 69, 192-193       |
| 269  | The pipe body is in direct contact with the big rocks                     | 191, 267          |
| 270  | The outer wall of the pipe is not protected                               | 191, 267          |
| 271  | The dimensions of pipes, grooves, tunnels, etc. do not meet the requirements | 191, 193, 234, 267 |
| 272  | Pipes and grooves material unqualified                                   | 191, 193, 267     |
| 273  | Subsidence or deformation of supporting facilities such as pipes, trenches and tunnels | 191, 267          |
| 275  | Excessive slope deviation for laying pipes, trenches, tunnels, etc.       | 191, 193, 234, 239, 267 |
| 276  | Improper design of corners of pipes, grooves, tunnels, etc.               | 191, 193, 234, 267 |
| 277  | Improper subgrade design of Pipes and grooves                             | 193, 234          |
| 278  | Improper design of slope ratio of pipe trench and embankment              | 193               |
| 281  | Poor quality of fill around the pipeline                                 | 191, 267          |
| 282  | The axial filling height of the pipe in the dam body is different         | 191, 267          |
| 286  | The joint length of the drain pipe is                                     | 191-192, 267      |
| Page | Content                                                                 | References                                      |
|------|-------------------------------------------------------------------------|-------------------------------------------------|
| 296  | Poor pump quality                                                      | 192-193, 234, 307                               |
| 299  | Improper installation of pump                                          | 192, 195, 200, 234, 307                         |
| 307  | Pump failure                                                           | 61, 100, 127, 192, 195, 200, 231                |
| 310  | The surrounding environment improvement does not meet the requirements | 19                                              |
| 312  | Dam body renovation does not meet the requirements                     | 62, 64-70, 73, 135-136, 148, 157-158, 167, 183 |
| 313  | The improvement of flood discharge system does not meet the requirements; | 191-192, 195, 234, 267, 273, 307                 |
| 325  | Monitoring instrument failure, work interruption                      | 327, 343                                       |
| 327  | Safety monitoring facilities cannot fully reflect the operating status of the tailings pond | 19, 45, 47, 49, 65-69, 135-136, 191-192, 200, 267, 343 |
| 334  | The number of water quality monitoring wells around the tailings pond is insufficient | 327                                              |
| Management factor | Improper measures to reduce the seepage line                                                                 | 167                                              |
| 168  |                                                                                                  | 158, 183                                       |
| 336  | The setting of monitoring facilities is not included in the construction plan | 325, 327                                       |
| 343  | Inadequate safety evaluation                                                                 | 19, 60, 156, 190, 200, 327                     |
| Page | Issue Description | References |
|------|------------------|------------|
| 346  | Improper data management | 38, 79, 162, 197, 199, 205, 207, 274, 299, 309, 324, 327, 343, 352 |
| 347  | Insufficient or wrong hydrological and geological data | 38, 79, 162, 197, 199, 205, 207, 274, 299, 309, 324, 327, 343, 352 |
| 351  | Improper maintenance | 60, 62, 64-70, 156-158, 167, 183, 190-193, 234, 238, 267, 307, 325 |
| 352  | Design defects of emergency plan | 19, 60, 62, 156, 190-191, 195 |
| 354  | Insufficient emergency plan drills | 19, 60, 62, 156, 190-191, 195 |

**Personnel factor**

| Page | Issue Description | References |
|------|------------------|------------|
| 38   | Inaccurate storage capacity calculation | 39, 194 |
| 75   | Improper calculation method of tailings dam stability | 64, 73, 77-81, 92 |
| 123  | Improper selection and care of slope protection turf | 73 |
| 124  | Slope cutting did not follow the design requirements | 19, 64-65 |
| 125  | Slope protection was not carried out in time | 19, 62, 64-65, 73, 122 |
| 126  | Unreasonable design of cast-in-place protective surface | 19, 62, 64-66, 73, 77, 122, 157 |
| 145  | No coverage measures in the pond area | 157 |
| 148  | Weakness of paving has not been reinforced | 158 |
| 130  | Poor construction quality of horizontal paving | 157 |
| 162  | Unreasonable anti-seepage design | 19, 156-157 |
| 199  | The determination of the flood control standard of the tailing pond is not accurate | 190, 194, 200 |
| 201 | Blocking defects of flood drainage facilities | 192-193, 195, 200 |
|-----|-----------------------------------------------|-----------------|
| 205 | The installation location and elevation of drainage facilities do not meet the design requirements | 193, 195, 200 |
| 274 | Improper installation of supporting facilities | 191, 267, 273 |
| 308 | Closure design not in accordance with regulations | 19, 62, 64-70, 73, 135-136, 148, 157-158, 167, 183, 191-192, 234, 238, 267, 273, 307 |
| 309 | Close the tailings pond without understanding the hidden dangers and risks | 66, 310, 312-313 |
| 324 | Improper selection of monitoring instruments and equipment | 327, 343 |
| 332 | No monitoring of groundwater and surrounding water bodies | 327 |
| 355 | Insufficient experience in personnel or organization qualification problems | 31, 38, 75, 79, 82, 61, 123-134, 145, 148-149, 162, 168, 170, 176, 197-199, 201, 205-207, 209-210, 218-221, 240, 260, 268-272, 274-278, 281-282, 286, 299, 308-310, 312-313, 324, 332, 334, 336, 343, 346-347, 351-352, 354 |

Appendix B. Grading standards of hazard indicators
| Grading Indicator | Classification and Value of Grading Indicator |
|-------------------|---------------------------------------------|
|                   | 1   | 2   | 3   | 4     |
| Personnel experience or organization qualification | >0.75 | 0.50-0.75 | 0.25-0.50 | <0.25 |
| Flood (once in N years) | <5   | 5-20 | 20-50 | >50  |
| Rainfall (mm/24h) | <50  | 50-100 | 100-200 | >200 |
| Liquefaction degree of tailings | <0.25 | 0.25-0.50 | 0.50-0.75 | >0.75 |
| The degree of impact of mining activities near the pond area | <0.25 | 0.25-0.50 | 0.50-0.75 | >0.75 |
| The height from the warning water level (m) | >8.00 | 8.00-4.00 | 4.00-0.00 | <0.00 |
| Fracture degree of drainage structure | <0.25 | 0.25-0.50 | 0.50-0.75 | >0.75 |
| Water leakage degree of drainage structure | <0.25 | 0.25-0.50 | 0.50-0.75 | >0.75 |
| Deformation degree of dam | <0.25 | 0.25-0.50 | 0.50-0.75 | >0.75 |
| Deformation degree of Pipe (groove) | <0.25 | 0.25-0.50 | 0.50-0.75 | >0.75 |
| Leakage channel | <0.25 | 0.25-0.50 | 0.50-0.75 | >0.75 |
| Dam settlement | <0.25 | 0.25-0.50 | 0.50-0.75 | >0.75 |
| Filter body | >0.75 | 0.50-0.75 | 0.25-0.50 | <0.25 |
| Monitoring blind spots of safety monitoring facilities | <0.25 | 0.25-0.50 | 0.50-0.75 | >0.75 |
| Blockage or siltation | <0.25 | 0.25-0.50 | 0.50-0.75 | >0.75 |
| Scoured dam | <0.25 | 0.25-0.50 | 0.50-0.75 | >0.75 |
| Dam crack | <0.25 | 0.25-0.50 | 0.50-0.75 | >0.75 |
| Water content of the dam surface | <0.25 | 0.25-0.50 | 0.50-0.75 | >0.75 |
| Dam foundation stability | <0.25 | 0.25-0.50 | 0.50-0.75 | >0.75 |
| Parameter                                                                 | 0.50  | 0.75   |
|---------------------------------------------------------------------------|-------|--------|
| Flood discharge capacity                                                  | >0.75 | 0.50-0.75 |
|                                                                          |       | 0.25-0.50 |
|                                                                          |       | <0.25   |
| Degree of erosion or cavitation of drainage structures                    | <0.25 | 0.25-0.50 |
|                                                                          |       | 0.50-0.75 |
|                                                                          |       | >0.75   |
| Equipment corrosion degree                                               | <0.25 | 0.25-0.50 |
|                                                                          |       | 0.50-0.75 |
|                                                                          |       | >0.75   |
| Height of seepage line (m)                                                | >8.00 | 6.00-8.00 |
|                                                                          |       | 1.40-1.70 |
|                                                                          |       | <1.40   |
| Remaining storage capacity of tailings pond                              | >60%  | 20%-60% |
|                                                                          |       | 10%-20%  |
|                                                                          |       | <10%    |
| Monitoring instrument stability                                          | >0.75 | 0.50-0.75 |
|                                                                          |       | 0.25-0.50 |
|                                                                          |       | <0.25   |
| Slope stability of tailings dam                                          | >0.75 | 0.50-0.75 |
|                                                                          |       | 0.25-0.50 |
|                                                                          |       | <0.25   |
| Degree of foundation subsidence                                          | <0.25 | 0.25-0.50 |
|                                                                          |       | 0.50-0.75 |
|                                                                          |       | >0.75   |
| Possibility of landslides in the pond area                                | <0.25 | 0.25-0.50 |
|                                                                          |       | 0.50-0.75 |
|                                                                          |       | >0.75   |
| Sedimentation level of tailings                                          | >0.75 | 0.50-0.75 |
|                                                                          |       | 0.25-0.50 |
|                                                                          |       | <0.25   |
| Dam stability                                                             | >0.75 | 0.50-0.75 |
|                                                                          |       | 0.25-0.50 |
|                                                                          |       | <0.25   |
| Calculation method of dam stability                                      | >0.75 | 0.50-0.75 |
|                                                                          |       | 0.25-0.50 |
|                                                                          |       | <0.25   |
| Design of dam surface protection                                         | >0.75 | 0.50-0.75 |
|                                                                          |       | 0.25-0.50 |
|                                                                          |       | <0.25   |
| Completeness and accuracy of information                                 | >0.75 | 0.50-0.75 |
|                                                                          |       | 0.25-0.50 |
|                                                                          |       | <0.25   |
| Maintenance log                                                           | >0.75 | 0.50-0.75 |
|                                                                          |       | 0.25-0.50 |
|                                                                          |       | <0.25   |
| emergency plan                                                            | >0.75 | 0.50-0.75 |
|                                                                          |       | 0.25-0.50 |
|                                                                          |       | <0.25   |
| Safety assessment                                                         | >0.75 | -       |
|                                                                          |       | -       |
|                                                                          |       | 0       |
| Status of supporting facilities such as pipes, trenches, tunnels, etc.    | >0.75 | 0.50-0.75 |
|                                                                          |       | 0.25-0.50 |
|                                                                          |       | <0.25   |
| Filter                                                                   | >0.75 | 0.50-0.75 |
|                                                                          |       | 0.25-0.50 |
|                                                                          |       | <0.25   |
| Tailing particle size                                                     | >0.50 | 0.20-0.50 |
|                                                                          |       | 0.05-0.20 |
|                                                                          |       | <0.05   |
|                         | 0.75 | 0.50-0.75 | 0.25-0.50 | <0.25 |
|-------------------------|------|-----------|-----------|-------|
| Pump                    |      |           |           |       |
| Renovation of the dam body | >0.75 | 0.50-0.75 | 0.25-0.50 | <0.25 |
| The degree of local landslide and collapse of the dam | <0.25 | 0.25-0.50 | 0.50-0.75 | >0.75 |
| Flood drainage system renovation | >0.75 | 0.50-0.75 | 0.25-0.50 | <0.25 |
| Tailings unit weight    | >2.00 | 1.70-2.00 | 1.40-1.70 | <1.40 |
| Corrosiveness of tailings | <0.25 | 0.25-0.50 | 0.50-0.75 | >0.75 |
| Closure design          | >0.75 | 0.50-0.75 | 0.25-0.50 | <0.25 |
| Knowing the hazards and risks of the tailings dam before closing | >0.75 | 0.50-0.75 | 0.25-0.50 | <0.25 |
| Slope ratio of tailings dam (1:n) | <1.00 | 1.00-3.00 | 3.00-5.00 | >5.00 |
| Dam layout              | >0.75 | 0.50-0.75 | 0.25-0.50 | <0.25 |
| Dam break               | 0     | -         | -         | 1     |
| Leakage damage          | 0     | -         | -         | 1     |
| Overtopping             | 0     | -         | -         | 1     |

**Declarations**

**Declaration of competing interest**

The authors declare that they have no known competing financial interests or personal relationships that could have appeared to influence the work reported in this paper.

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Figures

Figure 1

Mode of the I-FRPN
**Figure 2**

Node degree in the I-FRPN

**Figure 3**

Cumulative degree distribution of the I-FRPN
Figure 4

Clustering coefficient of nodes in the I-FRPN

Figure 5

Equal-sized dam failure risk random network
Figure 6

The impact of the four remediation sequences of hazard nodes on network efficiency

Figure 7

Key hazards and propagation paths of Dam I failure