Congestion cost analysis of Condongcatur signalized intersection Sleman, D.I. Yogyakarta using PTV. Vissim 9

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Abstract. The congestion cost is one of the most important consideration when congestion happens. One of the most frequent congestion is in the intersection. And one of the intersections in Yogyakarta Special Region is Condongcatur signalized intersection, Sleman. This study aims to analyze the congestion cost of intersection by using PTV. Vissim 9 as a traffic microsimulation model. For the congestion costs, the author uses Tzedzakis, 1998 approach. The results show that in the existing condition, the average delay is 103.72 determining Vehicle Operating Cost (BOK), Existing Speed is 70 km/H for primary road and 60 km/H for secondary road, travel time value is KB : Rp. 4.970, KR: Rp. 1.925, and SM : Rp. 315. Therefore, the total congestion cost is Rp. 5.663.790, -/hour.

1 Introduction

The congestion cost is one of the most important consideration when congestion happens. One of the most frequent congestion is in the intersection. And one of the intersections in Yogyakarta Special Region is Condongcatur signalized intersection, Sleman. Condongcatur signalized intersection is a North Ringroad of Yogyakarta which is a connector between Provinces of Central Java and an access road to the city of Yogyakarta.

In this study, the author tries to approach how much the cost incurred in the signalized intersection. According to Stubs in Sugiyanto [1], he mentions that the cost of congestion is a relation between speed and flow on the road and the relation between speed and vehicle cost. If the existing traffic flow limit is exceeded, then the average traffic speed will decrease. When the speed starts to fall, then the vehicle operating costs (BOK) will increase in the range of 0-45 miles / hour and the time to travel will increase. But in case at a signal intersection, not all vehicles have speed due to delay on traffic light effects.

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Therefore, in this study the author uses simulation results from PTV. Vissim 9 student version as the deciding factor of the intersection delay. The intersection delay is used to predict how much it will cost. Vehicle operating costs (BOK) in this study refers to Sugiyan [2], which has made an approach to determine the BOK from the speed of the vehicle.

2 Methodology

2.1 Framework analysis

The data used in this study is based on field survey and previous study which then modeled using PTV. VISSIM. 9. Flow chart that explain the methodology can be seen in Figure 1.

![Flow chart](image)

Figure 1. Framework analysis

2.2 Study location

This study is located at Condongcatur Signalized Intersection Sleman, Special Region of Yogyakarta which is a connecting the North Ringroad, Jl. Angga Jaya, and Jl. Affandi (Gejayan). The Location can be seen in Figure 2.

2.3 Study time

The traffic volume used in this study is secondary data on previous study by Maulidiah [3]. Data collection is done for 12 hours on weekdays at 06.00 – 18.00 WIB, meanwhile, in determining the geometric way, cycle time, and environmental conditions are carried out at the observation on Sundays, December 04, 2016. And to determine the speed of the vehicle
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Figure 2. The study location (Source: Google Earth, 2016)

3 Results and analysis

3.1 Geometric condition

The geometric way in the intersection can be seen at the figure below.

Figure 3. Intersection geometric

a. Wide of arm North (A) : 7,1 meter
b. Wide of arm East (B) : 10,9 meter
c. Wide of arm South (C) : 6,6 meter
d. Wide of arm West (D) : 11,5 meter

(Spot Speed) conducted on Wednesday, March 22, 2016 which represents the weekday through survey every type of vehicle KB (heavy Vehicle), KR (Light Vehicle), and SM (Motorcycle). The study was conducted with survey in the morning at 06.00 – 08.00 WIB, 11.00 – 13.00 WIB, and in the afternoon 16.00 to 18.00 WIB.
3.2 Type of road environment

Type of road environment can be seen at the table below.

| Arm Code             | Regional Condition                  | Type of Road Environment |
|----------------------|------------------------------------|--------------------------|
| Jl. Angga Jaya (N)   | Shopping complex                   | Commercial               |
| Jl. North Ringroad(E)| Shopping complex                   | Commercial               |
| Jl. Gejayan (S)      | Shopping complex, office complex    | Commercial               |
| Jl. North Ringroad(W)| Shopping complex                   | Commercial               |

3.3 Phase condition

Phase condition can be seen at the table below.

| Signal   | Arm | Type of Arm       | Time (second) |
|----------|-----|-------------------|---------------|
|          |     |                   | Red           | Green | Amber | All Red |
| Phase 1  | N   | Protected (P)     | 162           | 34    | 3      | 4       |
| Phase 2  | E   | Protected (P)     | 137           | 59    | 3      | 4       |
| Phase 3  | S   | Protected (P)     | 167           | 29    | 3      | 4       |
| Phase 4  | W   | Protected (P)     | 143           | 53    | 3      | 4       |

| Cycle Time (second) | 203 |

3.4 Traffic volume

Traffic volume survey conducted by data Maulidiah, E [3]. The survey was taken on Monday, 12 January, 2016 which represent on weekdays condition. This is the traffic volume.
3.5 Traffic modeling by PTV. Vissim 9

This is the flowchart how PTV. Vissim used in this research.

![Flowchart](image)

Figure 5. PTV.Vissim 9 process

And this is the output.

Table 2. PTV.Vissim 9 output on existing condition

| NO | MOVEMENT | QLEN | QLENMAX | VEHS (ALL) | PERS (ALL) | LOS (ALL) | LOSVAL (ALL) | VEHDELAY (ALL) | PERSDELAY (ALL) | STOPDELAV (ALL) | STOPS (ALL) |
|----|-----------|------|---------|-----------|------------|-----------|-------------|----------------|----------------|----------------|-------------|
| 1  | Jl. Ringroad Utara (B) - Jl. Angga Jaya (U) | 0.00 | 0.00    | 41.00     | 41.00      | LOS_C     | 3.00        | 24.57          | 24.57          | 13.79          | 3.68        |
| 2  | Jl. Ringroad Utara (B) - Jl. Ringroad Utara (T) | 291.55 | 383.33 | 131.00    | 131.00     | LOS_F     | 6.00        | 139.04         | 139.04         | 115.26         | 4.56        |
| 3  | Jl. Affandi (S) - Jl. Ringroad Utara (B) | 291.55 | 383.33 | 60.00     | 60.00      | LOS_F     | 6.00        | 174.82         | 174.82         | 131.17         | 11.23       |
| 4  | Jl. Angga Jaya (U) - Jl. Ringroad Utara (T) | 0     | 0       | 48.00     | 48.00      | LOS_A     | 1            | 6.47           | 6.47           | 3.01           | 0.17        |
| 5  | Jl. Ringroad Utara (B) - Jl. Affandi (S) | 140.02 | 167.94 | 59.00     | 59.00      | LOS_F     | 6            | 188.52         | 188.52         | 157.37         | 3.75        |
| 6  | Jl. Angga Jaya (U) - Jl. Ringroad Utara (B) | 140.02 | 167.94 | 13.00     | 13.00      | LOS_F     | 6            | 115.25         | 115.25         | 94.15          | 2.46        |
| 7  | Jl. Affandi (S) - Jl. Ringroad Utara (B) | 24.66  | 148.29  | 19.00     | 19.00      | LOS_D     | 4            | 36.57          | 36.57          | 21.43          | 1.68        |
| 8  | Jl. Affandi (S) - Jl. Angga Jaya (U) | 152.48 | 207.82 | 59.00     | 59.00      | LOS_F     | 6            | 208.15         | 208.15         | 185.12         | 3.15        |
| 9  | Jl. Affandi (S) - Jl. Ringroad Utara (T) | 152.48 | 207.82 | 53.00     | 53.00      | LOS_F     | 6            | 197.8          | 197.8          | 177.62         | 2.73        |
| 10 | Jl. Ringroad Utara (T) - Jl. Affandi (S) | 0.01   | 12.32   | 105.00    | 105.00     | LOS_B     | 2            | 15.9           | 15.9           | 4.51           | 0.67        |
| 11 | Jl. Ringroad Utara (T) - Jl. Ringroad Utara (B) | 227.85 | 365.66 | 153.00    | 153.00     | LOS_F     | 6            | 102.98         | 102.98         | 83.6           | 2.27        |
| 12 | Jl. Ringroad Utara (T) - Jl. Angga Jaya (U) | 227.85 | 365.66 | 47.00     | 47.00      | LOS_F     | 6            | 122.59         | 122.59         | 99.33          | 3.55        |
|    | Average  | 49.38 | 383.33  | 597.00    | 597.00     | LOS_F     | 6            | 103.72         | 103.72         | 83.87          | 3.69        |

3.6 Model validation

To test the validation of traffic modeling with Vissim, the author makes correlation with the number of real vehicle and from the model. Let’s see the model validation from regression linier analysis with Microsoft Excel 2016.
Table 3. Comparison between the number vehicle from actual survey and from the model

| Arm | Direction | Real (veh/h) | Model (veh/15 m) |
|-----|-----------|-------------|------------------|
|     | LT        | 671         | 105              |
|     | E ST      | 2513        | 153              |
|     | RT        | 600         | 47               |
|     | LT        | 866         | 19               |
|     | S ST      | 1059        | 59               |
|     | RT        | 979         | 55               |
|     | LT        | 112         | 41               |
|     | W ST      | 2935        | 131              |
|     | RT        | 1419        | 60               |
|     | LT        | 262         | 48               |
|     | N ST      | 1916        | 59               |
|     | RT        | 407         | 13               |

Figure 6. The regression analysis to validate the model

From the figure above, we can see that R square is 0.5742. The author argues that the model is not too correlate with the real number of vehicle in Gejayan intersection. Because it is not close with 1. The other reason is about the software version. The author uses Student Version from this model. Student Version just only can run in 10 minute simulation.

3.7. Vehicle operating cost

The vehicle operating cost estimate by using Sugiyanto [2, 4] by using references prices at the end of September 2009.
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\[ BOK = 0.4937V^2 - 60.218V + 2991.9 \]

(1)

With, \( V \) = Vehicle speed (Km/Hour)

Based on the vehicle type and arm, this is the result of BOK.

Table 3. Vehicle operational cost

| No | Arm   | Vehicle Type | BOK (Rp/Veh.km) |
|----|-------|--------------|-----------------|
| 1  | NORTH | KB           | 1754.62         |
|    |       | KR           | 1621.97         |
|    |       | SM           | 375.07          |
| 2  | EAST  | KB           | 1207.84         |
|    |       | KR           | 1207.13         |
|    |       | SM           | 351.96          |
| 3  | SOUTH | KB           | 1219.62         |
|    |       | KR           | 1177.69         |
|    |       | SM           | 365.70          |
| 4  | WEST  | KB           | 1440.24         |
|    |       | KR           | 1239.61         |
|    |       | SM           | 351.43          |

Note:
- KB : Heavy Vehicle
- KR : Light Vehicle
- SM : Motorcycle
3.8 Existing speed (A) and ideal speed (B)

In this study, on the east and West arms which is the primary arterial road based on Government regulation No. 34 of 2006 on the Road is determined to be the lowest at 60 km/h, but based on the survey speed that is done at peak hours speed of 60 km/h is still exceeded, thus determined the ideal speed of 70 km/h to calculate the congestion charge. as for the North and South arms is determined at 60 km/h which is the primary collection road.

3.9 Travel time value (V)

The time value is calculated using the study of Indonesian Highway Capacity Manual [5] with approach of Gross Regional Domestic Product (GRDP). Based on the vehicle type, the time value for heavy vehicle is taken at Rp. M4.970, Light vehicle Rp. 1.925, and Motorcycle Rp. 315.

3.10 Queue and delay Time

The amount of queue or delay time is obtained from the output of VISSIM modeling.

| No | Arm | Delay (second) | Queue Time (hour) |
|----|-----|----------------|------------------|
| 1  | North | 103.413       | 0.0287           |
| 2  | East  | 80.49          | 0.0224           |
| 3  | South | 118.468        | 0.0329           |
| 4  | West  | 112.81         | 0.0313           |

3.11 Congestion cost analysis

Congestion cost is a travel expense due to traffic delay or additional vehicle volume approaching or exceeding road service capacity [6]. Using the equation 1-2, then the congestion cost in the existing condition can be seen in Table 5.

\[ C = N \ast \left[ GA + \left(1 - \frac{A}{B}\right) V' \right] T \]  

where:
\[ C = \text{Congestion cost (Rupiah)}, \]
\[ N = \text{Vehicle volume (Vehicle)}, \]
\[ G = \text{Vehicle operating cost (Rp/Veh.Km)}, \]
\[ A = \text{Existing speed (Km/Hour)}, \]
\[ B = \text{Ideal speed (Km/Hour)}, \]
\[ V' = \text{Vehicle time value fast (Rp/Veh.Hour)}, \]
\[ T = \text{Delay time (Hour)}. \]
Table 5. Total congestion cost analysis

| No | Lengan | Congestion Cost (Rp/Hour) |
|----|--------|--------------------------|
| 1  | North  | IDR 774,589              |
| 2  | East   | IDR 1,573,246            |
| 3  | South  | IDR 1,351,843            |
| 4  | West   | IDR 1,964,113            |
|    | Total of Congestion Cost | IDR 5,663,790          |

4 Conclusion

Based on the results of modeling and analysis using software VISSIM 9 at the signalized intersection Condongcatur Sleman Yogyakarta, it can be concluded as follows.

1. Existing condition of signalized intersection Condongcatur Sleman Yogyakarta shows the highest traffic volume (peakhour) occurred in the morning at 07.30 – 08.30 WIB with average delay rate of 103.72 seconds, average queue length of 49.38 meters, and Level of service level is F (very bad).

2. The costs incurred due to congestion that occurred at the signalized intersection Condongcatur Sleman Yogyakarta is Rp. 5,663,790,-/hour at peak hour condition 07.30 – 08.30 WIB.

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