Carbstone Pavers: A Sustainable Solution for the Urban Environment

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Abstract: To reduce CO₂ emissions from the building industry, one option is to replace cement in specific applications with alternative binders. The Carbstone technology is based on the reaction of calcium- and magnesium-containing minerals with CO₂ to form carbonate binders. Mixes of carbon steel slag and stainless-steel slag, with tailored particle size distributions, were compacted with a vibro-press and subsequently carbonated in an autoclave to produce carbonated steel slag pavers. The carbonated materials sequester 100–150 g CO₂/kg slag. Compressive and tensile splitting strength of the resulting pavers were determined, and the ratio was found to be comparable to that of concrete. The environmental performance of the Carbstone pavers, with an average tensile splitting strength of 3.6 MPa, was found to be in compliance with Belgian and Dutch leaching limit values for construction materials. In addition, leaching results for a concrete mix made with aggregates of crushed Carbstone pavers (simulating the so-called “second life” of pavers) demonstrate that the pavers can be recycled as aggregates in cement-bound products after their product lifetime.

Keywords: carbonation; sustainable construction materials; alternative binders; vibro-pressing; tensile splitting strength; environmental quality; leaching

1. Introduction

Cement is the second most consumed material by mankind, after water. About 4.1 Gt of cement is produced each year [1]. Traditional cement, or OPC (Ordinary Portland Cement), relies on high temperature (1450 °C) calcination of limestone to generate Portland clinker, emitting about 0.875 tonnes of CO₂ per tonne of Portland clinker produced [2]. Cement typically constitutes 10 to 15% of concrete, and although concrete has one of the lowest embodied energy demands per volume among materials [3], cement ends up being responsible for 5–10% of global CO₂ emissions due to its massive scale of production [4]. Under a Business as Usual scenario, the cement industry may become responsible for up to a third of anthropogenic CO₂ emissions by 2050, as a result of increased demand in developing countries and CO₂ mitigating actions taking place in other sectors [5]. Unlike for other energy-intensive processes, CO₂ emissions in cement production are mainly derived from the decarbonation of raw materials, i.e., limestone, and only 40% of the CO₂ emissions are related to energy [2,6]. The use of green electricity or refuse-derived/biofuels can therefore only partially mitigate the production emissions.

Several pathways exist to reduce CO₂ emissions from the building industry. Concrete mixes can be optimized to avoid overconsumption of cement, or the use of Portland clinker in cement can be reduced by using SCMs (supplementary cementitious materials) to partially replace the OPC [6]. In some applications, the use of cement can be avoided entirely by using alternative binders, if these materials are locally, and to some extent, abundantly available. These alternative materials, such as carbonate-based binder materials [7,8], alkali-activated materials [9], and Calcium Sulfoaluminate (CSA) systems [10,11], can provide local solutions for certain applications. One such alternative is the Carbstone...
technology [12], where calcium- and magnesium-rich minerals from steel slags react with CO₂ to produce carbonate binders. In this process, non-hydraulic steel slags (stainless-steel (electric arc furnace and argon oxygen decarburization slag) and basic oxygen furnace slags) are transformed into high-quality construction materials (up to 130 MPa compressive strength) without using any cement. An additional advantage is that the carbonated materials sequester 100–150 g CO₂/kg slag [13]. Life cycle assessments (LCA) of carbonated steel slag products have shown that the recovery of pure CO₂ from industrial waste gas for carbonation of steel slag products accounts for a large part of the energy consumption, and thus, significantly contributes to the global warming potential (GWP) or carbon footprint of the carbonated products [14–16]. The net CO₂ footprint is, however, still negative, meaning that more CO₂ is captured in the carbonated steel slag products than is emitted during the production process [14–16]. To reduce the CO₂ emissions of the carbonation process further, flue gas can be used instead of pure CO₂ gas, and a direct flowing flue gas can be used instead of a traditional electricity-based carbonation curing chamber or autoclave reactor [16]. This may, however, also influence the CO₂ uptake of the products and needs to be further investigated. Taking into account the avoided CO₂ emissions from cement, up to 18.5 kg CO₂-equ. emission reduction per m² paver can be achieved [17].

Initial development of the Carbstone technology focused on the production of interior wall building blocks, having compressive strength in the range of 30 to 50 MPa [12,18]. The production process was adapted from traditional sand-lime brick production, and used hydraulic press compaction and autoclave curing. In 2018, within the nationally funded Stapsteen project (Vlaanderen Circulair project 153), pavers were identified to be high potential applications for the urban environment. Pavers are typically made by vibration pressing, and the strength requirement (according to EN 1338 [19]) is formulated in terms of tensile splitting strength (min. 3.6 MPa). As such, this change in application required a twofold adaptation in the Carbstone process: from hydraulic to vibration press, and from optimization of compressive to tensile splitting strength. In concrete-based building products, mechanical properties are commonly linked to compressive strength, since access to a compressive strength test machine is simple and more frequent, and it can be evaluated experimentally in a straightforward manner without difficulty in its interpretation. The relation between compressive and split tensile strength in concrete has been studied extensively over the years, and many data are available in the literature [20,21]. Formulas have been given by various standards to estimate the tensile strength of concrete from its compressive strength [22–24]. However, in the case of Carbstone products, the main mechanical property evaluated so far has been the compressive strength, and the relationship between the compressive strength and split tensile strength has not been investigated or established. Steel slags are by-products, with potentially significant concentrations of residual metals such as Cr, Mo, and V. Stainless-steel slags typically contain 1 to 7 wt% Cr [25]. The concentration of these residual metals may depend on the source, chemical composition, cooling history and demetallization process of the steel slags [26,27]. The use of steel slags or other by-products (e.g., as unbound or bound aggregate in concrete) can lead to significant leaching of the residual metals. Carbonation can limit or increase the leaching by reducing the pH, since the leaching of most pollutants is pH-dependent [28], by decreasing the porosity [29], or by mineralogical changes [30]. The extent of leaching of Carbstone products needs to be evaluated and should also include an evaluation of their use as recycled secondary aggregates in concrete, as their use in concrete is generally associated with a significant pH increase that may result in a remobilization of pollutants. To the best of our knowledge, the environmental impacts of carbonated steel-slag-based pavers (or carbonated steel-slag-based construction blocks) as recycled (secondary) aggregates in concrete have not been evaluated. This is essential as this may impact the ‘closing the loop’ of concrete, and, in particular, of recycled concrete aggregates.

To increase the confidence in carbonated steel slag products, and their acceptance in the market in applications that can replace concrete as construction material, it is of paramount importance to show that these products comply with all local environmental
standards. In addition, it should be ascertained that at the end of its service life, these products can be recycled in a sustainable way (e.g., as unbound aggregate or as cement bound aggregate) such as construction and demolition waste (C&DW), without affecting the concrete loop of recycled aggregates. The objective of this research is twofold: (1) To carry out a preliminary investigation into the relationship between compressive and tensile splitting strength for Carbstone pavers, and to compare it to that of concrete. (2) To evaluate the environmental performance of a Carbstone paver, which conforms to the tensile split cylinder requirements of EN 1338:2003. The environmental performance of the Carbstone pavers in different lifecycles will be compared to the leaching limits set by VLAREMA [31] in the Flemish region, and the Dutch soil quality decree in the Netherlands [32].

2. Materials and Methods

2.1. Materials and Characterization Methods

Steel slag (LD-Slag) from Arcelor Mittal Gent, and three stainless-steel slag fractions from Orbix—Stinox® 2/6 fraction, Stinox® 0/2 fraction, and Carbinox®—were used in this study. These steel slag fractions are produced by Orbix during demetallization of electric arc furnace and argon oxygen decarburization slags from stainless-steel production.

Figure 1 shows the particle size distribution (PSD) of the received raw materials measured by sieve analysis. For the sieve analysis, a sieve tower was made with 350 g of material. Shaking time was set to 10 min. In case of laser diffraction, Horiba’s Partica mini LA-350 Laser Scattering Particle Size Distribution Analyzer was used to measure and estimate the PSD of the Carbinox®. Before the measurement, 20 mL of isopropanol and 0.07 g of homogenized Carbinox were placed in a 50 mL plastic tube. An external ultrasonication probe was used for 2 min at 250 Watts to ensure a good dispersion of the powder.

![Figure 1. Particle size distribution of Carbinox®, LD Slag, Stinox® 2/6, and Stinox® 0/2 obtained using Sieve analysis.](image)

Chemical composition was determined using a high-performance energy dispersive XRF spectrometer with polarized X-ray excitation geometry (HE XEPOS, Spectro Analytical Systems, Kleve, Germany). The instrument was equipped with a 50 W tungsten end window tube (max. 60 kV, 2 mA) and a Silicon Drift Detector. For signal optimization, different targets were applied. All analyses were performed under Helium atmosphere. For analysis of loose powder, an XRF sampling cup, provided with a 4 μm Prolene foil (Chemplex), was filled with the sample and subsequently placed in the autosampler of the EDXRF system. The quantification was performed using a pre-calibrated software package for (semi)-quantitative analysis of geological materials. Table 1 summarizes the chemical
compositions of the LD-Slag, Carbinox® and Stinox® fractions. Note for the elements Na and Mg, there is a measurement uncertainty of 30%, and the values for S are indicative. For all other elements, the measurement uncertainty is approximately 10 to 20%.

In addition, ICP-AES analysis was performed to determine the content of Cr, Ni, Mo, V, and Ba. The metal content in the sample was determined with inductively coupled plasma–atomic emission spectrometry (ICP-AES) after acid digestion using a microwave system according to CMA/2/II/A.3. About 0.5 g of sample was weighed in the digestion vessel. Then, the following acids were separately added: 6 mL HCl, 2 mL HNO₃ and 4 mL of HBF₄. The digestion vessel was placed into the microwave unit (6 positions, Milestone) and the following digestion process was applied: 2 min at 250 W, 2 min at 0 W, 5 min at 250 W, 5 min at 400 W and 5 min at 500 W. At the end of the program, the vessels were cooled down to room temperature, transferred to a volumetric flask, and filled with up to 100 mL with ultrapure water. The digested solution was measured with an ICP-AES (Perkin Elmer, Optima 3000 DV). The fluoride (F) content was measured by ion-selective electrode analysis after hydro-pyrolysis of the solid sample.

The mineralogy of the materials (milled to a particle size < 63 µm) was determined by Rietveld analysis of X-ray powder diffraction (XRD) scans using an X’Pert PRO XRD Philips diffractometer equipped with a Co tube (λ = 1.7890100 nm). Measurements were carried out between 5° and 120° 2θ with a step size of approximately 0.013° and 48 s net integration time per step. The resulting diffractograms were analyzed using HighScore Plus version 4.0 software. The quantification of the mineral phases was carried out adapting the method reported in [33], which consists of a quantitative mineral analysis using the Rietveld method combined with an external standard for determining the content of amorphous phases. The main mineralogy of the LD-slags is characterized by portlandite, β-Ca₂SiO₄, Srebrodolskite, (all between 8 and 18 wt.%) and minor contents of lime, calcite, katoite, cuspidine, quartz, and magnesioferrite (all < 5 wt.%); the amorphous content is about 25 wt.%. For the Carbinox and Stinox samples, merwinite, bredigite, cuspidine, γ-Ca₂SiO₄ and gehlenite-akermanite are the major phases (between 7 and 20 wt.%) and the minor phases are β-Ca₂SiO₄, wollastonite, periclase, magnetite, spinel and calcite (<5 wt.%). The amorphous content is about 25 wt.% [18,34].

Table 1. Chemical composition of the LD slags and Carbinox® and Stinox® from stainless-steel slags determined by ED-XRF analysis, except for F, which was measured by ion-selective electrode analysis after hydro-pyrolysis of the solid sample.

|          | LD Slag | Carbinox® 0/2 mm | Stinox® 2/6 mm | Stinox® 2/6 mm |
|----------|---------|------------------|---------------|---------------|
| CaO (%)  | 51      | 40               | 38            | 39            |
| SiO₂ (%) | 14      | 37               | 38            | 38            |
| MnO (%)  | 2.5     | 0.77             | 0.98          | 0.93          |
| MgO (%)  | 3.8     | 13               | 14            | 14            |
| Na₂O (%) | 3.6     | 0.81             | 0.92          | 0.70          |
| P₂O₅ (%) | 1.7     | 0.28             | 0.25          | 0.28          |
| Al₂O₃ (%)| 2.8     | 5.5              | 6.3           | 5.9           |
| FeO (%)  | 20      | 0.54             | 0.68          | 0.35          |
| TiO₂ (%) | 0.65    | 0.55             | 0.68          | 0.68          |
| Cr₂O₃ (%)| 0.15    | 2.1              | 2.2           | 1.9           |
| MoO₃ (%) | 0.00083 | 0.0049           | 0.011         | 0.0048        |
| V₂O₅ (%)| 0.580   | 0.051            | 0.046         | 0.045         |
| S (%)    | 0.15    | 0.22             | 0.20          | 0.16          |
| F (%)    | 0.33    | 1.8              | 0.8           | na*           |

* na = not analyzed.

2.2. Mixtures

To better understand the relationship between the compressive strength and tensile splitting strength of the carbonated materials, 8 mixes were designed consisting of various combinations of LD Slag, and the three stainless-steel slag fractions (Table 2). Note that the
as-received LD Slag (0–3 mm) and Carbinox® (<0.5 mm) were milled to below 250 microns, to allow for better compaction, and optimum reactivity during the carbonation process. In addition, by decreasing the particle size distribution of the LD Slag, the free lime can easily convert to calcium hydroxide during the hydration process, which mitigates the possibility of cracking (due to the hydration of lime) during carbonation of the pavers. Furthermore, the last column in Table 2 shows the moisture content (water/solid) ratio to which the mixtures were brought 24 h before compaction and carbonation to let hydration occur. In addition, using the proper range of moisture content will allow better compaction in the vibro-press. Figure 2 shows the overall cumulative particle size distribution of mixtures 1 to 8. Note that once the relationship between the compressive strength and tensile splitting strength was established based on Mix 1 to Mix 8, a mix was designed to validate and verify the observed relationship for a mixture which was not part of the experiment, shown in the last row in Table 2 as Mix Final.

Table 2. Mixtures used to investigate the relation between compressive strength and split tensile strength. Last column in the table shows the moisture content (water/solid) ratio, to which the mixtures were brought 24 h before compaction and carbonation.

| Mix   | LD-Slag (<150 µm)% | Carbinox (<150 µm)% | Stinox (0/2 mm)% | Stinox (2/6 mm)% | Stinox (2/10 mm)% | Moisture Content |
|-------|---------------------|----------------------|------------------|------------------|-------------------|-----------------|
| Mix 1 | 70                  |                      | 30               |                  |                   | 14.0            |
| Mix 2 | 50                  |                      | 50               |                  |                   | 11.4            |
| Mix 3 | 25                  | 25                   | 50               |                  |                   | 10.8            |
| Mix 4 | 30                  |                      | 70               |                  |                   | 10.8            |
| Mix 5 | 15                  | 15                   | 70               |                  |                   | 10.8            |
| Mix 6 | 20                  |                      | 40               | 40               |                   | 5.0             |
| Mix 7 | 30                  |                      | 20               | 50               |                   | 4.5             |
| Mix 8 | 25                  |                      | 15               | 60               |                   | 4.0             |
| Mix Final | 25 | 35                   | 20               | 20               |                   | 13.0            |

Figure 2. Overall cumulative particle size distribution of mixtures 1 to 8, along with cumulative particle size distribution of the components used in designing of the mixtures.
2.3. Carbonation Methodology

As schematically shown in Figure 3, the steps needed to make the pavers are as follows:

1. Pretreatment stage of each mixture: All components used to make the mixtures were first dried at 105 °C. Then, components were combined according to Table 2, to make 3 pavers with dimensions 220 × 220 × 80 mm for each mixture. The mixtures were then placed in a bucket and sealed firmly to allow the mixture to hydrate for 24 h and avoid drying before the shaping stage.

   \[
   \begin{array}{c|c|c}
   \text{PRETREATMENT} & \text{SHAPING} & \text{CURING} \\
   \hline
   \text{water} & \text{slag 1} & \text{slag 2} \\
   \text{mixing} & \text{compaction} & \text{carbonation} \\
   \end{array}
   \]

   \textbf{Figure 3.} Schematic drawing of the carbonation process at lab-scale [12].

2. Shaping stage: A Metalika Vibro-press VPS 2000/1 GT with a frequency of 100 kHz and a capacity of 8–12 pavers per 30 s cycle at Orbix was used to make the pavers. For each mixture, the hydrated mixes were placed in three of the 220 × 220 × 80 mm paver’s molds. Several filling steps followed by compaction through vibration/stamping steps are needed to obtain an optimum compaction, and thus, compressive strength. The compaction stage gave the newly shaped pavers sufficient green strength to be handled and placed in the autoclave for carbonation (Figure 4).

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\item \text{Carbonation stage: The fresh pavers were placed in a Scholz autoclave at Orbix in Farciennes for carbonation. The carbonation was carried out at 6 bars (with 100% CO}_2\text{) at 35 °C for 72 h (Figure 4).}
\end{itemize}

After carbonation, some of the samples were cut in half, and a solution of phenolphthalein indicator was used to evaluate the extent of carbonation based on pH. After contact
with the phenolphthalein indicator solution, a pink color is indicative of a pH > 9, indicating incomplete carbonation. Note that for Mix 1 and Mix 2, the pavers were cut in two, resulting in a smooth surface, while for the rest of the mixes, the (rough) halves of the split tensile strength were used (Figure 5).

**Figure 5.** Carbonation fronts identified by phenolphthalein indicator of the mixes. Note that in case of Mix 1 and 2, the samples were cut, while in case of Mix 3 to 8, and the mix final the phenolphthalein indicator test was carried out on the blocks after splitting tensile.

### 2.4. Mechanical Strength Measurement Methods

Once the pavers were carbonated, the methods described below were used to evaluate various mechanical properties of the mixtures.

Non-destructive ultrasonic pulse velocity (UPV) has been used extensively to assess the quality and homogeneity of concrete [35]. UPV has also been used to follow the changes in concrete properties over time, whether those changes were due to hydration, enhancing the stiffness of the concrete or deterioration due to durability-related issues [36]. Finally, although UPV is proportional to the elastic modulus/stiffness of the concrete, the pulse velocity method has been used to estimate the strength of concrete, once the relation between compressive strength and UPV has been established [37,38]. Effects such as improper compaction, changes in water to cement ratio, type and quality of coarse aggregates can influence the elastic stiffness and thus, the pulse velocity of concrete [39]. Many of the examples mentioned above can also be used in the case of carbonation. Here, UPV was used to assess the quality of the carbonated pavers, as a proxy for the carbonation degree of the pavers. The UPV measurements were also correlated with the compressive strength of the Carbstone pavers. The UPV test was carried out on the 220 × 220 × 80 mm pavers following EN 12504-4:2004 with some adaptations. PUNDIT® PL-200 was used under direct through-transmission mode, with two 54 kHz Ø50 mm × 46 mm transducers, one as a transmitter and one as a receiver. Knowing the travel distance is 220 mm, the pulse velocity of the material was calculated to assess the quality of the pavers from each mixture after carbonation.

The split tensile test was carried out on the 220 × 220 × 80 mm pavers following EN 1338:2003 with some adaptations. The split tensile strength of the pavers was calculated based on Annex F.4 of the standard. According to the standard, the split tensile strength should not be lower than 3.6 MPa.

Eight samples of 40 × 40 × 40 mm were wet cut with a continuous rim diamond blade saw from pieces of the pavers from the split tensile strength test (approximately 220 × 110 × 80). The compressive strength test was carried out on these samples. A loading rate of 2.4 kN/s was used. Note that according to EN 1338:2003, the compressive strength is not considered a requirement.

### 2.5. Environmental Quality

Based on the results obtained from the relationship between compressive strength and split tensile strength, the validated final mixture passing the 3.6 MPa split tensile strength
The requirement set by the EN 1338:2003 was selected to determine the environmental quality at the end of life phase.

The environmental quality of the pavers was tested according to the Flemish Materials Decree of the 23rd of December 2011, and the accompanying implementing decisions in VLAREMA of 17 February 2012. VLAREMA is an acronym for ‘Decision of the Flemish Government establishing the Flemish regulations concerning the sustainable management of materials cycles and waste’ [31]. It provides a clear framework for defining the end-of-waste phase of a waste material.

Three pavers were crushed in different steps to obtain a 0–4 mm fraction that was used to perform an upstream column percolation test (according to CMA/2/II/A.9.1 and NEN 7373:2004 standards) (L/S = 10, 7 eluate fractions). In addition, two upstream column percolation leaching tests according to CMA/2/II/A.9.5 (L/S 10, 1 eluate fraction) were performed to establish the cumulative leaching at L/S = 10 more firmly. These percolation tests are carried out to study the leaching behavior (percolation test in 7 fractions) and to establish if the material complies with the Flemish and Dutch environmental regulations. The performed percolation test is similar to the European prEN 16637-3:2020 standard. However, 95 wt% of the material was crushed to a grain size < 4 mm, while in the European standard, a larger grain size is maintained. In the European standard, crushing is avoided as much as possible, i.e., crushing is only carried out in the case of an oversized fraction (size depending on the column diameter) or to fill a lack of fine grains less than 4 mm. A subsample was milled for analysis of the total composition after microwave destruction in a mix of HCl, HNO$_3$ and HBF$_4$ in a microwave oven (conform CMA/2/II/A.3).

To further support the environmental quality in a second life, a concrete mix was made with aggregates from crushed Carbstone pavers. This simulates what would happen in the so-called “second and third life”, when Carbstone pavers are demolished, and the resulting fragments used as secondary aggregates in concrete. For the concrete mix, a worst-case situation was evaluated by replacing both the fine (0–4 mm) and coarse (4–20 mm) natural aggregates with (100%) recycled Carbstone paver aggregates. This situation will never occur in practice, because Carbstone pavers will always be mixed with other construction and demolition wastes. Additionally, only part of the natural aggregates in concrete are generally replaced by recycled aggregates because of their technical properties (higher water absorption, etc.). CEM I was used, as this generally leads to the highest pH, and therefore, worst-case scenario for leaching of most metals. After 28 days curing, the concrete was gently crushed to <4 mm (95% < 4 mm) aggregates and subjected to upstream percolation tests (according to VLAREMA CMA/2/II/A.9.1 (7 fractions) and NEN 7373:2004).

3. Results and Discussion

3.1. Relation between Compressive Strength and Splitting Tensile Strength

Figure 5 shows the results of the phenolphthalein indicator solution test. In all mixtures, especially the ones containing larger particles, there are some non-carbonated pockets rather than carbonation fronts. Obviously, this observation is more vivid in the case of mixes with coarser Stinox aggregates. Overall, the mixtures were carbonated uniformly throughout.

UPV was used to assess the quality of the pavers after carbonation (Figure 6). It can be seen that in most cases, except Mix 1 and Mix 2, the UPV values have reached 3500 m/s; note that in the case of concrete classification, a UPV value between 3500 m/s and 4500 m/s is considered to be a good concrete quality [40]. Mixture 3 had the highest UPV, reaching approximately 4000 m/s.

Figure 7 shows that higher compressive strength results in higher split tensile strength. For a better visualization of the data, the ratio of splitting tensile to compressive strength versus paver’s compressive strength was plotted (Figure 8). A clear trend emerged. In addition, the fitted line for many concrete data has also been shown for comparative reasons [20]. The trends are very similar to the ones from concrete, where samples with lower compressive strength have higher $f_{\text{top}}/f_c$. As the compressive strength increases, the increase in the
split tensile strength is not as pronounced and thus, the ratio of split tensile to compressive strength as a function of compressive strength decreases significantly. Based on Figure 8, the pavers should have a minimum compressive strength in the range of 30–35 MPa, in order for their splitting tensile strength to be 3.6 MPa. However, more data are needed to confirm the observed trend, especially at high compressive strengths.

Figure 6. Ultrasonic pulse velocity of pavers up to 72 h of carbonation.

Figure 7. Split tensile and compressive strength of the pavers from various mixes.

Figure 8. Ratio of splitting tensile to compressive strength versus paver’s compressive strength.
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Figure 8. Ratio of splitting tensile to compressive strength versus paver’s compressive strength.

3.2. Environmental Quality

For the final mix, which is the mix containing the highest amount of Carbinox and thus, the highest amount of potential pollutants, the environmental quality was established with regard to the leaching limits imposed by the Flemish government in the Flemish region and by the Dutch government in the Netherlands. The leaching values are compared with the current and (proposed) future VLAREMA legislation of the Flemish region, and with the Dutch soil quality decree (‘regeling bodemkwaliteit’) of the Netherlands (Table 3).

The results show that the environmental quality complies with the current and future VLAREMA legislation. For molybdenum, the Dutch soil decree limits are exceeded in two of the three leaching tests.

Fluoride leaching exceeds (2x) the Flemish target value and the Dutch limit value. However, although anion concentrations must be monitored in VLAREMA, they are not grounds for exclusion. In the Netherlands, application could be targeted to locations in direct contact with seawater or brackish water, for which the fluoride leaching limit is increased to 220 mg/kg dry matter.

The release mechanism for Cr, Mo, Ba, V and the anions fluoride, chloride and sulphate was assessed following the guidelines of the European prEN 16637-3:2020 standard. For all other elements, the leaching concentrations were too low to properly determine the release mechanism. The leaching behavior as a function of cumulative L/S is shown in Figure 9. For all the elements, the release was pH-dependent. Pearson’s correlation coefficient was calculated for leachate concentrations of fractions 2 to 7 (Table 4). The concentrations in the leachate of fraction 1 were not included in the analysis, as the leaching in this fraction is often affected by wash-off. Note that for most elements (V being the only exception), the leaching increases with increasing pH. This indicates that a further decrease in the pH, for example, by a better carbonation of the pavers, may lead to a lowering of the leaching concentrations. However, it also indicates that when recycling these materials in a concrete matrix, the leaching could increase because of the higher pH environment.
Table 3. Total element content and leaching concentrations (mg/kg dry matter) as determined by upstream column percolation testing.

| Metal  | Total Concentration | Column 7 Fractions | Column 1 Fraction A | Column 1 Fraction B | VLAREMA Annex 2.3.2 | VLAREMA Future | Dutch Soil Decree |
|--------|---------------------|--------------------|---------------------|---------------------|----------------------|----------------|------------------|
|        | limit values        | proposed limit values | limit values        | limit values        | limit values | limit values | limit values |
| As (mg/kg) | <2                  | 0.053              | <0.050              | <0.050              | 0.8               | 0.8           | 0.9             |
| Cd (mg/kg) | <0.5                | 0.011              | <0.010              | <0.010              | 0.03              | 0.03          | 0.04            |
| Cr (mg/kg) | 3160               | 0.26               | 0.21                | 0.22                | 0.5                | 2.6           | 0.63            |
| Cu (mg/kg) | 67                 | 0.057              | <0.050              | <0.050              | 0.5                | 0.8           | 0.9             |
| Hg (mg/kg) | <0.2               | <0.0020            | <0.0020             | <0.0020             | 0.02              | 0.02         | 0.02            |
| Pb (mg/kg) | 7.1                | 0.11               | <0.10               | <0.10               | 1.3                | 1.3           | 2.3             |
| Ni (mg/kg) | 636                | 0.053              | <0.050              | <0.050              | 0.75               | 0.75         | 0.44            |
| Zn (mg/kg) | 31                 | 0.11               | <0.10               | <0.10               | 2.8                | 2.8          | 4.5             |
| Ba (mg/kg) | 180                | 0.70               | 0.28                | 0.43                | 20                 | 22           |                 |
| Co (mg/kg) | 31                 | 0.053              | <0.050              | <0.050              | 0.5                | 0.54         |                 |
| Mo (mg/kg) | 128                | 2.3                | 0.50                | 1.4                 | 55                 | 55           | 1               |
| Sb (mg/kg) | <4                 | 0.11               | <0.10               | <0.10               | 1                  | 0.32         |                 |
| Se (mg/kg) | <4                 | 0.21               | <0.20               | <0.20               | 2                  | 0.15         |                 |
| Sn (mg/kg) | <4                 | 0.11               | <0.10               | <0.10               | 1                  | 0.4          |                 |
| V (mg/kg) | 925                | 0.20               | 0.71                | 0.51                | 2.5                | 1.8          |                 |

| Anion | target value (†) |
|-------|------------------|
| Bromide (mg/kg) | - 4.0 <10 <10 20 20 (2) |
| Chloride (mg/kg) | - 350 330 370 1000 616 (2) |
| Fluoride (mg/kg) | - 98 100 98 55 55 (2) |
| Sulphate (mg/kg) | - 230 240 250 2200 2430 (2) |

(†) For anions, the future VLAREMA legislation only imposes target values. These must be monitored, but are not grounds for exclusion. (2) When using building materials in places where direct contact with sea water or brackish water with a natural chloride content of more than 5000 mg/L is possible, no maximum emission values for chloride and bromide are imposed. Maximum emission values for fluoride and sulphate are multiplied by a factor of 4.

Figure 9. Leaching behavior of paver aggregates as a function of cumulative L/S. (a) Leaching of Ba, Cr, F and Mo (b) Leaching of chloride, sulphate and V.
Table 4. Pearson’s correlation coefficient based on the leachate concentrations in fractions 2 to 7. Green is a strong correlation, orange is a medium correlation.

| Pearson’s | pH  | Ba  | Cr  | Mo  | V   | Chloride | Fluoride | Sulphate |
|-----------|-----|-----|-----|-----|-----|----------|----------|----------|
| pH        |     | 0.89| 0.50| 0.89| −0.97| 0.67     | 0.99     | 0.94     |
| Ba        | 0.89|     | 0.79| 0.99| −0.85| 0.91     | 0.89     | 0.99     |
| Cr        | 0.50| 0.79|     | 0.79| −0.40| 0.95     | 0.45     | 0.72     |
| Mo        | 0.89| 0.99| 0.79|     | −0.83| 0.93     | 0.89     | 0.99     |
| V         | −0.97| −0.85| −0.40| −0.83|     | −0.59    | −0.95    | −0.89    |
| chloride  | 0.67| 0.91| 0.95| 0.93| −0.59| 0.67     | 0.88     | 0.94     |
| fluoride  | 0.99| 0.89| 0.45| 0.89| −0.59| 0.67     | 0.88     | 0.94     |
| sulphate  | 0.94| 0.99| 0.72| 0.99| −0.89| 0.88     | 0.94     |          |

To further support the environmental quality, a concrete mix was made with crushed Carbstone pavers replacing all natural aggregates. This simulates what would happen in the so-called “second and third life”, when pavements made with Carbstone pavers are demolished, and the resulting construction and demolition waste is used to make recycled aggregates for concrete. For the concrete mix, a worst-case scenario was selected in which both the fine (0–4 mm) and coarse (4–20 mm) natural aggregates are fully (100%) replaced by recycled Carbstone paver aggregates. This situation will normally not occur in practice, because Carbstone pavers will always be mixed with other construction and demolition waste. Furthermore, most standards (e.g., [41]) recommend or, in some countries, only allow part of the natural aggregates to be replaced by recycled aggregates in certain applications because of their technical properties (higher water absorption, Los Angeles coefficient or Micro Deval value, etc.). Ordinary Portland cement (CEM I) was used, because this leads to the highest pH [42], and therefore, generally, the worst-case scenario for leaching. Cement with mineral admixtures such as blast furnace slag, fly ash, etc., reduce the calcium hydroxide content and refine the pore structure of the concrete through pozzolanic reactions. This enhances the compressive strength and durability of concrete made with these cements [43], and generally reduces the leaching of pollutants. After 28 days curing, the concrete was broken into <4 mm aggregates and subjected to upstream column tests (according to CMA/2/II/A.9.1 (7 fractions) and NEN 7373:2004 standards). The leaching results show that using recycled Carbstone pavers as secondary aggregates in concrete does not constitute a risk for the environment (Table 5). In Figure 10, the leaching behavior is shown; it can be seen that, except for Ba, the leaching of most pollutants is very low.

**Figure 10.** Leaching behavior of concrete aggregates as a function of cumulative L/S. (a) Leaching of Ba, Cr and Mo (b) Leaching of chloride, sulphate and fluoride.
Table 5. Results leaching concrete mix (350 kg/m³ CEM I), with replacement of fine (0–4 mm) and coarse aggregates (4–20 mm) with recycled aggregate from crushed Carbstone pavers. na = not analyzed.

| Metals  | Column 7 Fractions -A | Column 7 Fractions -B | VLAREMA Annex 2.3.2B | VLAREMA Future | Dutch Soil Decree |
|---------|-----------------------|-----------------------|-----------------------|----------------|------------------|
| As (mg/kg) | 0.007–0.020          | na                    | 0.8                   | 0.8            | 0.9              |
| Cd (mg/kg) | <0.005               | na                    | 0.03                  | 0.03           | 0.04             |
| Cr (mg/kg) | 0.31                  | 0.43                  | 0.5                   | 2.6            | 0.63             |
| Cu (mg/kg) | <0.03                | 0.01                  | 0.5                   | 0.8            | 0.9              |
| Hg (mg/kg) | na                   | na                    | 0.02                  | 0.02           | 0.02             |
| Pb (mg/kg) | <0.02                | na                    | 1.3                   | 1.3            | 2.3              |
| Ni (mg/kg) | <0.03               | na                    | 0.75                  | 0.75           | 0.44             |
| Zn (mg/kg) | <0.15                | na                    | 2.8                   | 2.8            | 4.5              |
| Ba (mg/kg) | 7.4                  | 8.0                   | 20                    | 22             |                  |
| Co (mg/kg) | 0.035                | 0.02                  | 0.5                   | 0.54           |                  |
| Mo (mg/kg) | 0.33                 | 0.37                  | 55                    | 1              |                  |
| Sb (mg/kg) | <0.020              | na                    | 1                     | 0.32           |                  |
| Se (mg/kg) | 0.010                | na                    | 2                     | 0.15           |                  |
| Sn (mg/kg) | 0.042               | <0.010                | 1                     | 0.4            |                  |
| V (mg/kg) | <0.010              | <0.005                | 2.5                   | 1.8            |                  |

Anions target value (1)

| Anions  | target value (1) |
|---------|------------------|
| Bromide (mg/kg) | 0.56            |
| Chloride (mg/kg) | 250             |
| Fluoride (mg/kg) | 24              |
| Sulphate (mg/kg) | 97              |

(1) For anions, the future VLAREMA legislation only imposes target values. These must be monitored, but are not grounds for exclusion. (2) When using building materials in places where direct contact with sea water or brackish water with a natural chloride content of more than 5000 mg/L is possible, no maximum emission values for chloride and bromide are imposed. Maximum emission values for fluoride and sulphate are multiplied by a factor of 4.

4. Conclusions

In the range of 10–40 MPa, a clear trend is observed between compressive and tensile split strength for the carbonated pavers. The trend is similar to that observed for concrete, with a significant decline in $f_{sp}/f_c$ ratio with increasing compressive strength. The trend shows that a minimum compressive strength in the range of 30–35 MPa must be reached to meet the requirement of 3.6 MPa split tensile strength. Further investigation is needed to verify the trend observed for higher compressive strengths, and other materials with carbonation potential.

The environmental properties of the Carbstone pavers comply with the current Flemish regulations, and the Dutch soil quality decree, when applied in contact with seawater or brackish water. After their service life, the Carbstone pavers can be recycled as aggregate in concrete without any environmental concerns. Concrete in which all natural aggregates have been replaced with recycled Carbstone pavers complied with all leaching limit values of VLAREMA and the Dutch soil decree.

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