COMPARATIVE STUDY OF MACROSCOPIC TRAFFIC FLOW MODELS AT ROAD JUNCTIONS

PAOLA GOATIN
Inria Sophia Antipolis - Méditerranée, Université Côte d’Azur, Inria, CNRS, LJAD
2004 route des Lucioles - BP 93
06902 Sophia Antipolis Cedex, France

ELENA ROSSI*
Università degli Studi di Milano-Bicocca, Dipartimento di Matematica e Applicazioni
via R. Cozzi 55
20126 Milano, Italy

(Communicated by Kenneth Karlsen)

Abstract. We qualitatively compare the solutions of a multilane model with those produced by the classical Lighthill-Whitham-Richards equation with suitable coupling conditions at simple road junctions. The numerical simulations are based on the Godunov and upwind schemes. Several tests illustrate the models’ behaviour in different realistic situations.

1. Introduction. Starting from the well-known Lighthill-Whitham-Richards (LWR) model [19, 20], a variety of macroscopic traffic flow models based on hyperbolic balance laws have been proposed to capture traffic behaviour in different situations. In this paper, we focus on the description of traffic dynamics on road networks. More precisely, we investigate the role of coupling conditions at road junctions. To this aim, we compare the behaviour of the multilane junction model introduced in [14], and the classical LWR model at junctions [11, Chapter 5].

The multilane junction model [14] allows to handle in detail various realistic cases of road junctions, with the major exception of the diverging ones, which requires some additional information on drivers’ routing preferences upstream. To avoid cumbersome notation, we recap here the main features of the multilane model in the case of a junction consisting of 2 incoming roads and 1 outgoing road, the number of lanes in each road to be specified later.

Let $M_{\ell} = M_{\ell_1} + M_{\ell_2}$ be the number of lanes on the half line $x < 0$, $M_{\ell_j}$ being the number of lanes on the $j$-th incoming road, $j = 1, 2$. Similarly, let $M_r$ be the number of lanes on $x > 0$, corresponding to the outgoing road. Each lane has maximal possible density normalized to 1, but each road may have different maximal speed. The detailed model is described in Section 3 below. Clearly, the case of a
1-to-1 junction is included, but not the 1-to-2 junction. This specific situation will be addressed separately in Section 5.

In order to draw a comparison with the LWR model on networks, we sum the vehicle densities on the various lanes, and we compare the total density profile with the solution given by the LWR model with corresponding maximal density. Notice that, in this way, we are led to consider the LWR model at a 2-to-1 junction with maximal density $M_{j}$ on the $j$-th incoming road and $M_{r}$ on the outgoing road. This choice is driven by the fundamental principle of conservation of the number of vehicles across the junction. The same approach is used on diverging junctions.

The numerical tests described in Sections 4 and 5 point out similarities and differences between the multilane model and its LWR counterpart. Interestingly, in the case of a 1-to-1 junction, the multilane description captures a different behaviour than those commonly described by the classical LWR approach. Merging junctions display pretty the same dynamics with both approaches, while diverging junctions show a more complex behaviour, acting as FIFO (first-in-first-out) when one outgoing road is fully congested, and as non-FIFO otherwise. Further assessment on the validity of the models necessitate the comparison with suitable real data.

2. LWR on $n$-to-$m$ junctions. Macroscopic traffic flow models on networks have been introduced in the mathematical literature since more than two decades [4, 15], see also [9, 11] for a detailed overview. We recall here the LWR model on $n$ to $m$ junctions, that is on junctions with $n$ incoming roads and $m$ outgoing roads.

A road network is modelled by a finite collection of edges and vertices, representing respectively unidirectional roads and junctions. On each edge $I$, and thus on each road, traffic flow is described through the Lighthill-Whitham-Richards (LWR) model: the density of vehicles $\rho(t, x)$ satisfies

$$\begin{cases} \partial_t \rho + \partial_x f_I(\rho) = 0, \\ \rho(0, x) = \rho_o(x), \end{cases}$$

(1)

where $f_I$ is assumed to be strictly concave and of class $C^2$, such that $f(0) = f(R_I) = 0$, $R_I$ being the maximal possible density on road $I$. In particular, we may choose

$$f_I(u) = V_I u \left(1 - \frac{u}{R_I}\right),$$

(2)

where $V_I$ is the maximal speed on road $I$. Note that, with this particular choice of flux, $f_I$ attains its unique maximum at $\theta_I = R_I/2$. At network junctions, the conservation of the number of vehicles has to be ensured and, moreover, the usual choice is to maximise the flux through the junction. We introduce therefore the demand and supply functions (see for instance [13]) and express the flux at the junction through an ad hoc minimisation between the two:

$$D_I(u) = \begin{cases} f_I(u) & \text{if } u \leq \theta_I, \\ f_I(\theta_I) & \text{if } u \geq \theta_I, \end{cases}$$

$$S_I(u) = \begin{cases} f_I(u) & \text{if } u \leq \theta_I, \\ f_I(\theta_I) & \text{if } u \geq \theta_I. \end{cases}$$

(3)

The demand and supply functions introduced above are displayed in Figure 1.

Notice that, as in [13], the flux function $f_I$, as well as the corresponding demand and supply functions, may depend also on time, if traffic regulations are applied.

At each junction, the model includes also of the preferences of the drivers, which are prescribed and known a priori. These preferences describe the distribution of traffic from incoming to outgoing roads and are expressed as elements of a matrix
A, called traffic distribution matrix: for a vertex $J$ with $n$ incoming edges $I_1, \ldots, I_n$ and $m$ outgoing edges $I_{n+1}, \ldots, I_{n+m}$, the matrix $A$ is given as follows

$$A = (a_{j,i}) \in \mathbb{R}^{m \times n} \text{ with } 0 \leq a_{j,i} \leq 1 \text{ for all } i \in \{1, \ldots, n\}, j \in \{n+1, \ldots, n+m\}$$

and

$$\sum_{j=n+1}^{n+m} a_{j,i} = 1 \text{ for all } i \in \{1, \ldots, n\}.$$

In other words, each element $a_{j,i}$ of the matrix $A$ gives the percentage of traffic from road $I_i$ going into road $I_j$ at the junction.

In view of the examples considered in this paper, we focus on the cases of one-to-one junctions (modeling speed limit or lane number changes), two-to-one (merging) junctions and one-to-two (diverging) junctions, see Figure 2. In the case of merging junctions, where the number of incoming roads is greater than the number of outgoing roads, it is necessary to introduce a priority parameter for each incoming road to single out a unique solution. In particular, for two-to-one junctions, the two incoming roads are characterised by the priority parameters $P$ and $(1 - P)$, with $P \in [0, 1]$.

2.1. 1-to-1 junction. Consider a one-to-one junction and call $I_1, I_2$ respectively the incoming and outgoing road. Denote by $\rho_i$ the density of vehicles in road $I_i$, $i = 1, 2$. Observe that the LWR model (1) on this particular junction is equivalent to a Cauchy problem for a conservation law with discontinuous flux (see, for instance, [16, Chapter 8] for a brief introduction to the topic):

$$\left\{ \begin{array}{l}
\partial_t \rho + \partial_x f(x, \rho) = 0, \\
\rho(0, x) = \rho_0(x),
\end{array} \right.$$  

(4)
with

\[ v(x, u) = H(x) v_r(u) + (1 - H(x)) v_l(u), \]  
\[ f_l(u) = u v_l(u) \quad f_r(u) = u v_r(u), \]  
\[ f(x, u) = u v(x, u) = H(x) f_r(u) + (1 - H(x)) f_l(u), \]

\[ H \] being the Heaviside function. The functions \( v_l \) and \( v_r \) are strictly decreasing, positive and such that \( v_d(R_d) = 0 \) for \( d = \ell, r \). One admissible choice is

\[ v_d(u) = V_d \left( 1 - \frac{u}{R_d} \right), \]

for a suitable positive constant \( V_d \), leading to the flux function (2).

Concerning the initial datum \( \rho_o \), it satisfies the following constraints:

\[ \rho_o(x) \in [0, R_d] \text{ for } x \in ]-\infty, 0[, \quad \rho_o(x) \in [0, R_r] \text{ for } x \in ]0, +\infty[. \]

Hyperbolic conservation laws with discontinuous flux such as (4) arise in the modelling of several phenomena, such as, for instance, two phase flow in heterogeneous media and traffic flow with rough road conditions. Among the rich literature on the subject, we refer the interested reader to [2, 3, 7, 18, 23] and the references therein.

The main issue when dealing with conservation laws with discontinuous flux function is the lack of a unique solution, since the classical theory by Kružkov does not apply. Clearly, Kružkov entropy conditions are valid away from the point(s) of discontinuity of the flux, but they are not enough to provide the uniqueness of solution. With the aim of proving the well-posedness of the problem, various notions of solutions have been introduced in the literature, based of different admissibility conditions. We recall for example the minimal variation criterion introduced by Gimse and Risebro [12] and the \( \Gamma \)-condition described by Diehl [6]. We refer to [3] for a more complete overview.

In the present study, we will focus on the solutions given by the supply-demand flux maximizing criterion classically used in traffic flow modeling (satisfying the entropy criterion of [1]) and by the vanishing viscosity limit [7].

2.2. 2-to-1 junction. In the case of merging junctions, a priority parameter has to be assigned to each incoming road, see [11, Section 5.2.2]. Therefore, in the specific situation of a two-to-one junction, the two incoming roads have priority \( P \) and \( 1 - P \) respectively, with \( P \in ]0, 1[ \). The idea is that, in general, not all vehicles can pass at the junction: denoting by \( \gamma \) the amount of vehicles that can enter the outgoing road in the supply-limited case, \( P \gamma \) vehicles comes from the first incoming road, and \( (1 - P) \gamma \) cars from the second incoming road.

Clearly, since there is only one outgoing road, the traffic distribution matrix \( A \) reduces to the two-dimensional vector \((1, 1)\).

The initial datum satisfies the following constraints:

\[ \rho_{o,\ell_1}(x) \in [0, R_{\ell_1}] \text{ for } x \in ]-\infty, 0[, \quad \rho_{o,r}(x) \in [0, R_r] \text{ for } x \in ]0, +\infty[. \]

Following [13], denoting by \( \rho_{\ell_i} \) and \( \hat{\gamma}_{\ell_i} \) the density and the flux on the incoming roads \((i = 1, 2)\) and by \( \rho_r, \hat{\gamma}_r \) the density and the flux on the outgoing one, for
In terms of the demand and supply functions (3), the FIFO rule amounts to the following: denoting by \( \rho_r \) the density on the incoming road, by \( \rho_{r_1} \) and \( \rho_{r_2} \) the densities on the outgoing roads, by \( \gamma_\ell, \gamma_{r_1} \) and \( \gamma_{r_2} \) the fluxes on the incoming and outgoing roads respectively, we have

\[
\begin{align*}
\gamma_\ell(\rho_\ell, \rho_{r_1}, \rho_{r_2}) &:= \min \left\{ D_\ell(\rho_\ell), \frac{S_{r_1}(\rho_{r_1})}{\alpha}, \frac{S_{r_2}(\rho_{r_2})}{1 - \alpha} \right\}, \\
\gamma_{r_1}(\rho_\ell, \rho_{r_1}, \rho_{r_2}) &:= \alpha \gamma_\ell(\rho_\ell, \rho_{r_1}, \rho_{r_2}), \\
\gamma_{r_2}(\rho_\ell, \rho_{r_1}, \rho_{r_2}) &:= (1 - \alpha) \gamma_\ell(\rho_\ell, \rho_{r_1}, \rho_{r_2}).
\end{align*}
\]

As a consequence, if one of the outgoing roads is fully congested (i.e. \( \min \{S_{r_1}(\rho_{r_1}), S_{r_2}(\rho_{r_2})\} = 0 \)), the vehicles are stuck at the junction \( \gamma_{r_1} = \gamma_{r_2} = 0 \), even if some of them could proceed in the other outgoing road. In order to overcome this problem, non-FIFO models have been developed, see for instance [13]. Non-FIFO model allows some flow through the junction even if one of the outgoing road is fully congested. Using the same notation as before, the non-FIFO rule reads as follows, in terms of the demand and supply functions (3):

\[
\begin{align*}
\gamma_{r_1}(\rho_\ell, \rho_{r_1}, \rho_{r_2}) &:= \min \{ \alpha D_\ell(\rho_\ell), S_{r_1}(\rho_{r_1}) \}, \\
\gamma_{r_2}(\rho_\ell, \rho_{r_1}, \rho_{r_2}) &:= \min \{ (1 - \alpha) D_\ell(\rho_\ell), S_{r_2}(\rho_{r_2}) \}, \\
\gamma_\ell(\rho_\ell, \rho_{r_1}, \rho_{r_2}) &:= \gamma_{r_1}(\rho_\ell, \rho_{r_1}, \rho_{r_2}) + \gamma_{r_2}(\rho_\ell, \rho_{r_1}, \rho_{r_2}).
\end{align*}
\]

In the case of a diverging junction, the initial datum satisfies the following constraints:

\[
\begin{align*}
\rho_{o,\ell}(x) &\in [0, R_\ell] \text{ for } x \in ]-\infty, 0[, \\
\rho_{o, r_1}(x) &\in [0, R_{r_1}] \text{ for } x \in ]0, +\infty[, \\
\rho_{o, r_2}(x) &\in [0, R_{r_2}] \text{ for } x \in ]0, +\infty[.
\end{align*}
\]

3. A multilane model for \( n \)-to-1 junctions \( (n = 1, 2) \). We recall the main features of the multilane junction model introduced in [14], based on the multilane traffic flow model proposed in [17]. The model provides a description of traffic on road networks with several lanes, allowing for lane changes and overtaking, as well as change in the speed laws and in the number of lanes along the road. In particular, \( \mathcal{M}_\ell \subset \mathbb{N}^+ \) represents the set of indexes of the \textit{active} lanes on \( ]-\infty, 0[ \), with cardinality \( |\mathcal{M}_\ell| \geq 1 \); on the other hand, \( \mathcal{M}_r \subset \mathbb{N}^+ \) is the set of indexes of the \textit{active} lanes on \( ]0, +\infty[ \), with cardinality \( |\mathcal{M}_r| \geq 1 \). We let \( M \geq \max \{ |\mathcal{M}_\ell|, |\mathcal{M}_r| \} \), its choice depending on the specific situation considered, and we assume, for technical needs, that there are \( M - |\mathcal{M}_\ell| \) and \( M - |\mathcal{M}_r| \) additional
phantom} lanes on $]-\infty, 0]$ and $]0, +\infty[$ respectively. We add moreover a condition (see (19) below), which prevents vehicles from passing from the active to the fictive lanes.

The model reads then as follows: for $x \in \mathbb{R}$ and $t > 0$, the vehicle density on lane $j$, $\rho_j = \rho_j(t, x)$, solves the Cauchy problem

$$\begin{aligned}
\partial_t \rho_j + \partial_x f_j(x, \rho_j) &= G_{j-1}(x, \rho_{j-1}, \rho_j) - G_j(x, \rho_j, \rho_{j+1}), \quad j = \ldots, M, \\
\rho_j(0, x) &= \rho_{o,j}(x), \quad j = \ldots, M,
\end{aligned}$$

(12)

with, for $j = 1, \ldots, M$,

$$\begin{aligned}
v_j(x, u) &= H(x) v_{v,j}(u) + (1 - H(x)) v_{t,j}(u), \\
f_{\ell,j}(u) &= u v_{v,j}(u), \quad f_{r,j}(u) = u v_{t,j}(u), \\
f_j(x, u) &= u v_j(x, u) = H(x) f_{r,j}(u) + (1 - H(x)) f_{\ell,j}(u),
\end{aligned}$$

(13)-(15)

$H$ being the Heaviside function. The velocities $v_{d,j}$, for $d = \ell, r$ and $j = 1, \ldots, M$, are strictly decreasing positive functions such that $v_{d,j}(1) = 0$. We assume that each map $f_{d,j}(u) = u v_{d,j}(u)$ admits a unique global maximum attained at $u = \theta_d$.

We set $\rho_{o,j} : \mathbb{R} \to [0,1]$ for $j = 1, \ldots, M$, and

$$\begin{aligned}
\rho_{o,j}(x) &= 0 \quad \text{for } x \in ]-\infty, 0] \text{ and } j \notin \mathcal{M}_\ell, \\
\rho_{o,j}(x) &= 1 \quad \text{for } x \in ]0, +\infty[ \text{ and } j \notin \mathcal{M}_r.
\end{aligned}$$

(16)-(17)

The source terms, which account for the flow rates across lanes, are defined as in [17]:

$$G_{d,j}(\rho_j, \rho_{j+1}) = K \left[ (v_{d,j+1}(\rho_{j+1}) - v_{d,j}(\rho_j))^+ \rho_j - (v_{d,j+1}(\rho_{j+1}) - v_{d,j}(\rho_j))^- \rho_{j+1} \right]$$

(18)

for $d = \ell, r$ and $j = 1, \ldots, M-1$, where $(a)^+ = \max\{a, 0\}$ and $a^- = -\min\{a, 0\}$ and $K$ is a dimensional constant $(1/m)$, which can be assumed equal to 1 after rescaling space and time. To account for separate lanes, such as different roads or fictive lanes, we set

$$G_{d,j}(u, w) = 0 \quad \text{for some } j_d \in \{1, \ldots, M-1\}, \ d = \ell, r.$$  

(19)

The functions appearing in the source term of (12) are then defined as follows

$$\begin{aligned}
G_j(x, u, w) &= H(x) G_{r,j}(u, w) + (1 - H(x)) G_{\ell,j}(u, w) \quad \text{for } j = 1, \ldots, M-1, \\
G_0(x, u, w) &= G_M(x, u, w) = 0.
\end{aligned}$$

(20)-(21)

For the sake of brevity, we introduce the notation $\rho = (\rho_1, \ldots, \rho_M)$, so that the initial data associated to problem (12)–(16)–(17) read $\rho(0, x) = \rho_{o}(x)$.

For simplicity, and with slight abuse of notation, we consider $\rho = \rho(t, x)$ for $t > 0$, $x \in \mathbb{R}$. However, as shown in [14], by (16), (17) and (19), there holds $\rho_j(t, x) = 0$ for all $t > 0$, $x \in ]-\infty, 0[$ and $j \notin \mathcal{M}_\ell$, respectively $\rho_j(t, x) = 1$ for all $t > 0$, $x \in ]0, +\infty[ \text{ and } j \notin \mathcal{M}_r.$

In order to compare the multilane model to the LWR model at junctions, we make the following choices. In all the numerical experiments, for the multilane model we choose

$$v_{I,j}(u) = V_I (1 - u) \quad \text{for } j = 1, \ldots, M,$$
with $V_I > 0$. Therefore, the maximal speed is the same for all lanes before, respectively after, $x = 0$. This corresponds to the following velocity for the LWR model:

$$v_I(u) = V_I \left(1 - \frac{u}{M_I}\right),$$

where $M_I$ is the cardinality of the active lanes.

Through our numerical integrations, we show that the outcome may be different when not considering the number of lanes involved.

4. Comparison study. We compare now numerically the solution given by the multilane $n$-to-1 model with the corresponding LWR solutions. We first describe the numerical schemes used to compute approximate solutions of the considered models. Then we report on some numerical tests, which illustrates the behaviour of solutions in different cases.

4.1. Numerical schemes. We provide in this section the details on the numerical schemes used throughout the paper. We introduce a uniform mesh in space, of width $\Delta x$, and a time step $\Delta t$, subject to a suitable CFL condition. For $k \in \mathbb{Z}$ set

$$x_k = \left(k + \frac{1}{2}\right) \Delta x, \quad x_{k-1/2} = k \Delta x,$$

where $x_k$ is the centre of the cell and $x_{k \pm 1/2}$ its interfaces. Notice that nonnegative integers denote the cells on the positive part of the $x$-axis. Set $\lambda = \Delta t / \Delta x$. For the multilane model, we approximate the initial data as follows: for $j = 1, \ldots, M$

$$\rho_{j,k}^0 = \frac{1}{\Delta x} \int_{x_{k-1/2}}^{x_{k+1/2}} \rho_{o,j}(x) \, dx,$$

recalling that (16) and (17) hold. The approximated initial data for the LWR model is given consequently as the sum of the initial data on various lanes, depending on the configuration under consideration:

- **1-to-1 junction (Section 4.2):** the initial data for the LWR model is merely given by the sum, thus

$$\rho_k^0 = \sum_{j=1}^{M_l} \rho_{j,k}^0 \text{ for } k \leq -1; \quad \rho_k^0 = \sum_{j=1}^{M_r} \rho_{j,k}^0 \text{ for } k \geq 0.$$

- **2-to-1 junction (Section 4.3):** for the merging junction, there are two incoming and one outgoing roads:

$$\begin{align*}
\text{for } k \leq -1: & \quad \rho_{i,k}^0 = \sum_{j=1}^{M_{l_i}} \rho_{j,k}^0 \text{ for } i = 1, 2; \\
\text{for } k \geq 0: & \quad \rho_{r,k}^0 = \sum_{j=1}^{M_r} \rho_{j,k}^0.
\end{align*}$$
268 PAOLA GOATIN AND ELENA ROSSI

- **1-to-2 junction (Section 5):** for the diverging junction, there are one incoming and two outgoing roads:

  for \( k \leq -1 \):
  \[
  \rho_{t,k}^0 = \sum_{j=1}^{M_k} \rho_{j,k}^0;
  \]

  for \( k \geq 0 \):
  \[
  \rho_{r_i,k}^0 = \sum_{j=1}^{M_{r_i}} \rho_{j,k}^0 \quad \text{for } i = 1, 2.
  \]

  The solution to the multilane model \((12)-(16)-(17)-(19)\) is obtained through a Godunov type scheme, with fractional step to take into account the source terms, see \[14, \text{Algorithm 2.1}\]:

  \[
  \rho_{j,k}^{n+1/2} = \rho_{j,k}^n - \lambda \left[ F_j(x_{k+1/2}, \rho_{j,k}^n, \rho_{j,k+1}^n) - F_j(x_{k-1/2}, \rho_{j,k}^n, \rho_{j,k}^n) \right],
  \]

  \[
  \rho_{j,k}^{n+1} = \rho_{j,k}^{n+1/2} + \Delta t G_j(x_k, \rho_{j-1,k}^{n+1/2}, \rho_{j,k}^{n+1/2}) - \Delta t G_j(x_k, \rho_{j,k}^{n+1/2}, \rho_{j+1,k}^{n+1/2}),
  \]

  where

  \[
  F_j(x, u, w) = \begin{cases} \min \{ D_j(x, u), S_j(x, w) \} & \text{if } x \neq 0, \\ \min \{ D_{\ell,j}(u), S_{r,j}(u) \} & \text{if } x = 0, \end{cases}
  \]

  with

  \[
  D_j(x, u) = H(x)D_{j,\ell}(u) + (1 - H(x))D_{j,\ell}(u),
  \]

  \[
  S_j(x, u) = H(x)S_{j,r}(u) + (1 - H(x))S_{j,r}(u),
  \]

  \( D_{j,d} \) and \( S_{j,d} \), \( d = \ell, r \), defined as in \((3)\).

  The LWR model is numerically integrated through a Godunov type scheme, which provides the solution to the Cauchy problem \((1)\) that maximises the flux through the junction:

- **1-to-1 junction (Section 4.2):**

  \[
  \rho_{k}^{n+1} = \rho_k^n - \lambda \left[ F(x_{k+1/2}, \rho_k^n, \rho_{k+1}^n) - F(x_{k-1/2}, \rho_k^n, \rho_{k-1}^n) \right],
  \]

  with \( F \) defined as in \((23)\), omitting index \( j \).

  In Section 4.2, we apply also a different strategy for the numerical integration of the LWR model: we make use of an upwind scheme, which provides the solution to the Cauchy problem \((1)\) coming from the vanishing viscosity approach. The numerical scheme reads as in \((24)\), where the numerical flux is now chosen as \( F(x, u, w) = u v(x, w) \).

- **2-to-1 junction (Section 4.3):** we follow \[13\] so that, exploiting the notation introduced in \((9)\), setting \( \hat{\gamma}_i^n = \hat{\gamma}_i(\rho_{\ell,i-1}^n, \rho_{r_{i-1},i}^n, \rho_{r_i}^n) \) for \( i = \ell_1, \ell_2, r \), we have

  for \( k < -1, \ell = \ell_1, \ell_2 \):
  \[
  \rho_{\ell,k}^{n+1} = \rho_{\ell,k}^n - \lambda \left[ F_\ell(\rho_{\ell,k}^n, \rho_{\ell,k+1}^n, \rho_{\ell,\ell+1}^n) - F_\ell(\rho_{\ell,k}^n, \rho_{\ell,k}^n, \rho_{\ell,k-1}^n) \right],
  \]

  for \( k > 1 \):
  \[
  \rho_{r,k}^{n+1} = \rho_{r,k}^n - \lambda \left[ F_r(\rho_{r,k}^n, \rho_{r,k+1}^n, \rho_{r,\ell+1}^n) - F_r(\rho_{r,k}^n, \rho_{r,k}^n, \rho_{r,k-1}^n) \right],
  \]

  for \( k = -1, \ell = \ell_1, \ell_2 \):
  \[
  \rho_{\ell,k-1}^{n+1} = \rho_{\ell,k-1}^n - \lambda \left[ \hat{\gamma}_\ell^n - F_\ell(\rho_{\ell,k}^n, \rho_{\ell,k-1}^n) \right],
  \]

  for \( k = 0 \):
  \[
  \rho_{r,\ell}^{n+1} = \rho_{r,\ell}^n - \lambda \left[ F_r(\rho_{r,\ell}^n, \rho_{r,\ell}^n, \rho_{\ell,\ell+1}^n) - \hat{\gamma}_r^n \right].
  \]

- **1-to-2 junction (Section 5):** as already recalled in Section 2.3, we use two different schemes, satisfying different rules at the junction.

  **FIFO rule:** we follow \[11\], and exploit the notation already introduced in \((10)\), with \( \gamma_i^n = \gamma_i(\rho_{\ell,i-1}^n, \rho_{r_1,i}^n, \rho_{r_2,i}^n) \) for \( i = \ell, r_1, r_2 \).
Non-FIFO rule: we follow [13] and exploit the notation already introduced in (11), with \( \gamma_i^n = \gamma_i(\rho_{\ell,k}^n, \rho_{r,k}^n, \rho_{r,k+1}^n) \) for \( i = \ell, r_1, r_2 \).

For both rules, the scheme amounts to the following:

\[
\begin{align*}
\rho_{\ell,k}^{n+1} &= \rho_{\ell,k}^n - \lambda \left[ F_{\ell}(\rho_{\ell,k}^n, \rho_{\ell,k+1}^n) - F_{\ell}(\rho_{\ell,k-1}^n, \rho_{\ell,k}^n) \right], \\
\rho_{r,k}^{n+1} &= \rho_{r,k}^n - \lambda \left[ F_{r}(\rho_{r,k}^n, \rho_{r,k+1}^n) - F_{r}(\rho_{r,k-1}^n, \rho_{r,k}^n) \right], \\
\rho_{\ell,-1}^{n+1} &= \rho_{\ell,-1}^n - \lambda \left[ \gamma_{r_1} - F_{\ell}(\rho_{\ell,-2}^n, \rho_{\ell,-1}^n) \right], \\
\rho_{r,0}^{n+1} &= \rho_{r,0}^n - \lambda \left[ F_{r}(\rho_{r,0}^n, \rho_{r,1}^n) - \gamma_{r_1}^n \right],
\end{align*}
\]

with the above choices of \( \gamma_i^n, i = \ell, r_1, r_2 \).

4.2. 1-to-1 junction: From 2 to 3 lanes. We consider the case of a junction with one incoming road with 2 lanes and one outgoing road with 3 lanes, see Figure 3. For the multilane model, this corresponds to problem (12)–(16)–(19) with \( M_\ell = \{1,2\}, M_r = \{1,2,3\} \) and \( S_{\ell,2}(u,w) = 0 \). For the LWR model, this is a 1-to-1 junction with maximal density on \([-\infty,0[\) equal to \( M_\ell = 2 \), while on \([0,\infty[\) it is \( M_r = 3 \).

![Figure 3. Scheme of the 1-to-1 junction with 2 lanes on the incoming road and 3 lanes on the outgoing one.](image)

Case 1. We choose \( V_\ell = 1.5, V_r = 1 \). The initial datum for the multilane model is

\[
\begin{align*}
\rho_{o,1}(x) &= 0.6, & \rho_{o,2}(x) &= 0.4, & \rho_{o,3}(x) &= 0.5 \times \chi_{[0,\infty]}(x),
\end{align*}
\]

while the initial datum for the LWR model is given by the sum of the above functions, thus

\[
\rho_o(x) = 1 \times \chi_{[-\infty,0[} + 1.5 \times \chi_{[0,\infty[}(x),
\]

Figure 5, left, displays the solutions of the considered models at time \( t = 1 \); in particular, it allows to compare the multilane model, namely the sum of the densities on the various lanes, to the LWR model. For the latter, there are many admissible solutions: indeed, depending on the physics of the problem, the solution may be different, see [3] and references therein for a thorough discussion on the choice of the “right” solution and on the physical models behind. In particular, the solutions obtained through the Godunov type scheme and through the upwind scheme are different. The former, obtained via the Godunov type scheme and thus through an ad hoc minimisation between the demand and supply function, is the solution that maximises the flux at \( x = 0 \), i.e. at the point of discontinuity of the flux: the solution connects \( f_\ell(M_\ell/2) \) and \( f_r(M_r/2) \), see also Figure 4. On the other hand, the intermediate value attained around \( x = 0 \) by the solution coming from the upwind scheme corresponds to the point \( \bar{u} \) in Figure 4: this point is such that \( f_\ell(\bar{u}) = f_r(\bar{u}) \). When higher order effects are taken into account for choosing the
Figure 4. Flux functions $f_\ell$ and $f_r$ related to the LWR model, for $M_\ell = 2$, $M_r = 3$, $V_\ell = 1.5$ and $V_r = 1$. The point $\hat{u}$ is such that $f_\ell(\hat{u}) = f_r(\hat{u})$. The value $U$ corresponds to the left trace at $x = 0$ of the sum of the solutions of the multilane model on the two incoming lanes, and $f_\ell(U)$ is the corresponding value of the flux.

“right” solution, the choice of the vanishing viscosity approximation leads to this particular solution.

Concerning the solution of the multilane model, the low value of the sum of the densities right downstream $x = 0$ is due to the fact that, before $x = 0$, the third lane does not exist. Thus, even though vehicles pass from the second to the third lane, the solution in the third lane is given by a shock with positive speed. On the other hand, the queue right upstream $x = 0$ is caused by the congestion of lanes 1 and 2, since after $x = 0$ the maximal speed decreases. Moreover, notice that the solution of the multilane model maximises the flux on each lane at the junction, see also Figure 6, right.

We make a second comparison between the LWR and the multilane model, namely we take the latter in the form of the average of the densities on the various lanes. In this way, the maximal density for the LWR model equals 1 everywhere.
Figure 5. The dashed blue line corresponds to the multilane model (12)–(16)–(19): on the left, it is the sum of its solutions, on the right it is the average. The dash-dotted orange line corresponds to the solution to the LWR model (1) obtained via the Godunov type scheme; the dotted green line is the solution to the LWR model (1) obtained through the upwind scheme. Here: $V_\ell = 1.5$, $V_r = 1$, initial datum (25).

The initial datum is therefore given by the average of (25):

$$\rho_o(x) = 0.5,$$

which corresponds to the critical density of the flux function on both incoming and outgoing lanes. Figure 5, right, displays the solutions at time $t = 1$. We notice how the solutions resemble each other on the incoming lane: the queue upstream $x = 0$ is well captured by all models. On the other hand, also in this case the behaviour of the multilane model on the outgoing lane strongly differs from the LWR model, for which in this case the supply-demand and the vanishing viscosity solutions coincide. Recall that, by [14, Lemma 2.3], the multilane model (12)–(16)–(19) preserves the total number of vehicles over time. This property is clearly valid when considering the multilane model in the form of the sum of densities on the various lanes. However, this second type of comparison considering the average of the densities on the various lanes implies that the total number of vehicles is not conserved anymore through the junction: indeed, the total mass is divided by the number of lanes, which is equal to 2 upstream $x = 0$ and 3 downstream.

**Case 2.** We choose now $V_\ell = 1$ and $V_r = 1.5$ and the same initial data of Case 1. The flux functions for the LWR model are displayed in Figure 7. In this case, the solution obtained with the Godunov type scheme is identical to that obtained through the upwind method, see Figure 8. Notice that the stationary shock at $x = 0$ is slightly smoothed out when using the upwind scheme, due to numerical viscosity. The small slope right before $x = 0$ in the sum of the solutions to the multilane model is caused by the higher flux of vehicles in lane 2 through $x = 0$, since right after $x = 0$ the speed is higher and vehicles can pass also in lane 3.

### 4.3. 2-to-1 junction: From 2+2 to 2 lanes.

We consider the case of a junction consisting of two incoming roads with 2 lanes each and one outgoing road with 2 lanes, that is problem (12)–(17)–(19), with $\mathcal{M}_\ell = \{1, 2, 3, 4\}$, $\mathcal{M}_r = \{2, 3\}$ and...
Figure 6. Left: Flux functions $f_\ell$ and $f_r$ related to the LWR model when comparing it to the multilane model in the form of the average of the densities on the various lanes: $V_\ell = 1.5$ and $V_r = 1$, in both cases the maximal density is 1. The orange line represents the solution to the Riemann problem with initial datum $\rho_o(x) = 0.5$. Right: flux functions related to the multilane model for the incoming ($f_\ell$) and the outgoing ($f_r$) lanes. The magenta line represents the solution on lane 1, with initial datum $\rho_{o,1} = 0.6$; the dotted blue line corresponds to the solution on lane 2, with initial datum $\rho_{o,2} = 0.4$.

Figure 7. Flux functions $f_\ell$ and $f_r$ related to the LWR model, for $M_\ell = 2$, $M_r = 3$, $V_\ell = 1$ and $V_r = 1.5$. The point $\tilde{u}$ is such that $f_r(\tilde{u}) = f_\ell(1)$.

initial data
\[ \rho_{o,1}(x) = 0.6 \chi_{[\infty,0]}(x) + 1 \chi_{[0,\infty]}(x), \quad \rho_{o,2}(x) = 0.4, \]
\[ \rho_{o,3}(x) = 0.6 \chi_{[\infty,0]}(x) + 0.8 \chi_{[0,\infty]}(x), \quad \rho_{o,4}(x) = 0.4 \chi_{[\infty,0]}(x) + 1 \chi_{[0,\infty]}(x), \]
with the additional assumption that there is no flow of vehicles between the second and the third lane on $[\infty,0]$, i.e. $S_{\ell,2}(u, w) = 0$ (we also impose $S_{r,1}(u, w) = S_{r,3}(u, w) = 0$). The situation under consideration is depicted in Figure 9. Concerning the LWR model, we denote by $\ell_1$ and $\ell_2$ the two incoming roads, and by $r$ the outgoing one. Then, the initial data for problem (1) corresponding to (26) read
Figure 8. The dashed blue line is the sum of the solutions to the multilane model (12)–(16)–(19); the dash-dotted orange line corresponds to the solution to the LWR model (1) obtained via the Godunov type scheme; the dotted green line is the solution to the LWR model (1) obtained through the upwind scheme. Here: $V_ℓ = 1$, $V_r = 1.5$, initial datum (25).

Figure 9. Scheme of the 2-to-1 junction with 2 lanes on each incoming road and 2 lanes on the outgoing one.

as follows:

$$
\rho_{o,\ell_1}(x) = 1, \quad \rho_{o,\ell_2}(x) = 1, \quad \rho_{o,r}(x) = 1.2,
$$

since road $\ell_1$ is given by lanes 1 and 2 for $x \in ]-\infty, 0[$, road $\ell_2$ is given by lanes 3 and 4 for $x \in ]-\infty, 0[$ and road $r$ is given by lanes 2 and 3 for $x \in ]0, +\infty[$. Moreover, we prescribe the priority $P = 1 - P = 1/2$ on each incoming road. Observe that the maximal density on each road is equal to 2.

Figure 10 displays the solution on each road at time $t = 1$: in each picture we can see both the LWR model and the multilane model, in the form of the sum of the densities on the involved lanes. Notice the asymmetric behaviour of the multilane model: this is due to the asymmetry of the initial data (26) with respect to the lanes that are left after the junction. Indeed, the initial datum in lane 2 has a lower value than in lane 1, while the opposite is true for lane 3 with respect to lane 4. In the former case, this implies that more vehicles have to pass form lane 1 to lane 2 in order to go through the junction, causing the formation of a longer queue, which become visible when summing the two densities, see Figure 10. Figure 11 provides
a detail of the multilane model at the same time $t = 1$, allowing a better insight on the queue formation.

Figure 10. In each picture, the dashed blue line is the sum of the solutions to the multilane model (12)–(16)–(19): from left to right, lanes 1 and 2; lanes 3 and 4; lanes 2 and 3. The dash-dotted orange line corresponds to the solution to the LWR model (1), obtained through a Godunov type scheme, with priorities $(1/2, 1/2)$: from left to right, incoming roads $a$, $b$, outgoing road $c$. The initial data are given in (26) and (27) respectively. In each lane we set $V = 1.5$.

Figure 11. Solution to the multilane model (12)–(16)–(19) at time $t = 1$, with initial data (26) and $V = 1.5$ on each lane.

5. **A multilane model for a diverge.** In this section, we aim to compare the LWR model on a one-to-two (diverging) junction with a multilane model different from the one considered in Section 3. Indeed, we need to account for the drivers’ routing preferences upstream, a feature that is not included in the multilane model (12)–(18).
The specific multilane junction is displayed in Figure 12: there are one incoming road with two lanes and two outgoing roads, with one lane each. The core idea is to consider a multi-population model [5, 10], where each population is identified by its desired final destination. In the particular case under consideration, we have two populations, which coexist upstream $x = 0$. We assume that, in each lane of the incoming road, a fraction $\alpha$ of the drivers belong to the first population, i.e. wants to go in lane 1 downstream, while the rest is directed towards lane 2. Overtaking is possible only upstream $x = 0$, and, differently from the multilane model considered in Section 3, only vehicles in lane 1 (respectively lane 2) targeting lane 2 (respectively lane 1) change lane. The rate of vehicles changing lane depends on the total density in the target lane: the more the target lane is crowded, the less vehicles are allowed to move into that lane. Moreover, the desire to go in the target lane increases as drivers approach the junction ($x = 0$). Since we focus on the dynamics at the junction, we do not consider the classical overtaking dynamics, which can occur before $x = 0$. This could be added in the source term with minor changes in the overall approach.

Our multilane multi-population model is the following: given the initial data $\rho_{o,1}$ and $\rho_{o,2}$ on $\mathbb{R}$, the initial data on $x < 0$ read

$$
\begin{align*}
\rho_{o,1}^1(x) &= \alpha \rho_{o,1}(x) \chi_{[-\infty,0)}(x), \\
\rho_{o,1}^2(x) &= (1 - \alpha) \rho_{o,1}(x) \chi_{[-\infty,0)}(x), \\
\rho_{o,2}^1(x) &= \alpha \rho_{o,2}(x) \chi_{[-\infty,0)}(x), \\
\rho_{o,2}^2(x) &= (1 - \alpha) \rho_{o,2}(x) \chi_{[-\infty,0)}(x),
\end{align*}
$$

where $\rho_{o,j}^i$, for $i, j \in \{1, 2\}$, denotes the amount of drivers in lane $j$ targeting lane $i$. Setting $\rho_1 = \rho_1^1 + \rho_1^2$ and $\rho_2 = \rho_2^1 + \rho_2^2$, the densities $\rho_j^i$, $i, j = 1, 2$, satisfy the following system:

$$
\begin{align*}
&\partial_t \rho_1^1 + \partial_x \left( \rho_1^1 v_1(\rho_1) \right) = K \rho_1^2 \frac{\max \{1 - \rho_1, 0\}}{|x|} \chi_{[-\infty,0)}(x), \\
&\partial_t \rho_1^2 + \partial_x \left( \rho_1^2 v_1(\rho_1) \right) = -K \rho_1^2 \frac{\max \{1 - \rho_2, 0\}}{|x|} \chi_{[-\infty,0)}(x), \\
&\partial_t \rho_2^1 + \partial_x \left( \rho_2^1 v_2(\rho_2) \right) = -K \rho_2^1 \frac{\max \{1 - \rho_1, 0\}}{|x|} \chi_{[-\infty,0)}(x), \\
&\partial_t \rho_2^2 + \partial_x \left( \rho_2^2 v_2(\rho_2) \right) = K \rho_1^2 \frac{\max \{1 - \rho_2, 0\}}{|x|} \chi_{[-\infty,0)}(x),
\end{align*}
$$

where $K$ is a dimensional constant (m/s), which can be assumed equal to 1 after rescaling space and time. In the source terms, the factor $|x|^{-1}$ represents the increase of the urgency to change lane as the junction approaches.

We observe that a close model which describes traffic on a multilane highway under the hypotheses that traffic is neither perfectly FIFO nor perfectly non-FIFO has been introduced in [22]. Here, we compare the multilane multi-population...
model (29) to the LWR model for a diverging junction, both in the case of a FIFO rule and of a non-FIFO rule at the junction, see Section 2.3. The incoming roads are paired together, and denoted by the subscript \( r \), while the two distinct outgoing roads are denoted by subscripts \( r_1 \) and \( r_2 \) respectively. The initial data for problem (1) is related to the initial data \( \rho_{o,1}, \rho_{o,2} \) for the multilane multi-population model as follows:

\[
\rho_{o,\ell}(x) = (\rho_{o,1}(x) + \rho_{o,2}(x)) \chi_{[-\infty,0]}(x),
\]

\[
\rho_{o,r_1}(x) = \rho_{o,1}(x) \chi_{[0, +\infty]},
\]

\[
\rho_{o,r_2}(x) = \rho_{o,2}(x) \chi_{[0, +\infty]}(x).
\]

5.1. Numerical scheme. We detail here the numerical scheme exploited for the integration of the multilane multi-population model (29). The scheme is inspired by that presented in [8, 21], and the source terms are treated through fractional step. The space and time mesh are defined as in Section 4.1. The initial data on each lane, \( \rho_{o,1} \) and \( \rho_{o,2} \), are approximated by \( \rho_{1,k}^0 \) and \( \rho_{2,k}^0 \), for \( k \in \mathbb{Z} \), as in (22), then distinguished on the negative part of the \( x \)-axis according to their target lane: for \( k \leq -1 \)

\[
\begin{align*}
\rho_{1,k}^{1,0} &= \alpha \rho_{1,k}^0, \\
\rho_{2,k}^{1,0} &= \alpha \rho_{2,k}^0, \\
\rho_{1,k}^{2,0} &= (1 - \alpha)\rho_{1,k}^0, \\
\rho_{2,k}^{2,0} &= (1 - \alpha)\rho_{2,k}^0.
\end{align*}
\]

The solution to the multilane multi-population model (29) is obtained through a Godunov type scheme, with fractional step to account for the source terms. In particular, set \( \rho_{j,k}^n = \rho_{j,k}^{1,n} + \rho_{j,k}^{2,n} \) and introduce the quantity \( \gamma_j^n = \min \{ \hat{D}_j(\rho_{j,-1}^n, \rho_{j,-1}^n), S_j(\rho_{j,0}^n) \} \), for \( j = 1, 2, i = 3 - j \), with \( S_j \) as in (3) and \( \hat{D}_j \) defined as let \( \theta_j(u) \) the point of maximum of the function \( u \mapsto uv_j(u + w) \), then

\[
\hat{D}_j(u, w) = \begin{cases} 
  uv_j(u + w) & \text{if } u < \theta_j(w), \\
  \theta_j(w) v_j(\theta_j(w) + w) & \text{if } u \geq \theta_j(w).
\end{cases}
\]

Then, for \( i, j = 1, 2 \), we set

if \( k < -1 \):

\[
\rho_{j,k}^{i,n+1/2} = \rho_{j,k}^{i,n} - \lambda \left[ \frac{\rho_{j,k}^{i,n}}{\rho_{j,k}^{n}} F(\rho_{j,k}^n, \rho_{j,k+1}^n) - \frac{\rho_{j,k-1}^{i,n}}{\rho_{j,k}^{n}} F(\rho_{j,k-1}^n, \rho_{j,k}^n) \right],
\]

if \( k = -1 \):

\[
\rho_{j,-1}^{i,n+1/2} = \rho_{j,-1}^{i,n} - \lambda \left[ \gamma_j^n - \frac{\rho_{j,-2}^{i,n}}{\rho_{j,-2}^n} F(\rho_{j,-2}^n, \rho_{j,-1}^n) \right],
\]

if \( k = 0 \):

\[
\rho_{j,0}^{i,n+1} = \rho_{j,0}^{i,n} - \lambda \left[ F(\rho_{j,0}^n, \rho_{j,1}^n) - \gamma_j^n \right],
\]

if \( k > 0 \):

\[
\rho_{j,k}^{i,n+1} = \rho_{j,k}^{i,n} - \lambda \left[ F(\rho_{j,k}^n, \rho_{j,k+1}^n) - F(\rho_{j,k-1}^n, \rho_{j,k}^n) \right],
\]

with \( F(u, w) = \min \{ \hat{D}_j(u), S_j(w) \} \). Concerning the contribution of the source terms, define

\[
G(x, u, w) = w \max \left\{ 1 - u, 0 \right\} \chi_{[-\infty,0]}(x).
\]
so that
\[
\begin{align*}
\rho_{1,k}^{1,n+1} &= \rho_{1,k}^{1,n+1/2} + \Delta t G(x_k, \rho_{1,k}^{1,n+1/2}, \rho_{2,k}^{1,n+1/2}), \\
\rho_{1,k}^{2,n+1} &= \rho_{1,k}^{2,n+1/2} - \Delta t G(x_k, \rho_{2,k}^{2,n+1/2}, \rho_{1,k}^{2,n+1/2}), \\
\rho_{2,k}^{1,n+1} &= \rho_{2,k}^{1,n+1/2} - \Delta t G(x_k, \rho_{1,k}^{1,n+1/2}, \rho_{2,k}^{1,n+1/2}), \\
\rho_{2,k}^{2,n+1} &= \rho_{2,k}^{2,n+1/2} + \Delta t G(x_k, \rho_{2,k}^{2,n+1/2}, \rho_{1,k}^{1,n+1/2}).
\end{align*}
\]

5.2. Example 1: Fully congested outgoing road. The first example we take into account considers a fully congested outgoing road. We choose \(\alpha = 0.4\), the maximal speed on the incoming road is \(V_\ell = 1.5\) and on both outgoing roads it is \(V_r = 2\). The initial data for the multilane multi-population model (29) are
\[
\begin{align*}
\rho_{o,1}(x) &= 0.6 \chi_{[-\infty,0]}(x) + 0.4 \chi_{[0,\infty]}(x), \\
\rho_{o,2}(x) &= 0.7 \chi_{[-\infty,0]}(x) + 1 \chi_{[0,\infty]}(x),
\end{align*}
\]
and through (30) we recover the initial data for the corresponding LWR model (1).

Figure 13 displays the solution on each road at time \(t = 0.5\): in each picture we see the LWR model with both FIFO and non-FIFO rule and the multilane multi-population model, in the form of the sum of the densities for \(x < 0\). Lane 2 is initially fully congested: no vehicles can enter, and the solution on that lane is clearly the same with all the three models. In this situation, the solution of the multilane multi-population model looks very close to that of the LWR model with FIFO rule. Indeed, non-FIFO rule allows vehicles targeting lane 1 to overcome the junction: they are not queuing waiting for their turn to pass the junction.

**Figure 13.** In each picture, the dashed blue line corresponds to the solutions to the multilane multi-population model (29), and in particular to their sum on \(x < 0\). The dotted orange line corresponds to the solution of the LWR model (1) with non-FIFO rule, while the dash-dotted green line is the solution to the LWR model (1) with FIFO rule. The initial data is (31), while \(V_\ell = 1.5\), \(V_r = 2\) and \(\alpha = 0.4\).
5.3. Example 2. In this second example, none of the outgoing lane is fully congested. We keep the same parameters as before, thus $\alpha = 0.4$, $V_\ell = 1.5$, $V_r = 2$. The initial data for the multilane multi-population model (29) are

$$\rho_{o,1}(x) = 0.6 \chi_{[-\infty,0]}(x) + 0.4 \chi_{[0,\infty]}(x),$$

$$\rho_{o,2}(x) = 0.7 \chi_{[-\infty,0]}(x) + 0.8 \chi_{[0,\infty]}(x),$$

(32)

which differ from (31) only in the density on the second road downstream $x = 0$. Through (30) we recover the initial data for the LWR model (1).

Figure 14 displays the solution on each road at time $t = 0.5$: in each picture we see the LWR model with both FIFO and non-FIFO rule and the multilane multi-population model, in the form of the sum of the densities for $x < 0$. Even though lane 2 is not fully congested downstream $x = 0$, all the three models produce the same solution there. However, on the outgoing road corresponding to lane 1 the solution to the multilane multi-population model (29) is very close to the solution to the LWR model (1) with non-FIFO rule. In the incoming roads, the similarity is reduced, but nevertheless the solution to the LWR model with FIFO rule looks much different than the others, since it reaches a greater value upstream $x = 0$.

![Figure 14](image-url)

**Figure 14.** In each picture, the dashed blue line corresponds to the solutions to the multilane multi-population model (29), and in particular to their sum on $x < 0$. The dotted orange line corresponds to the solution of the LWR model (1) with non-FIFO rule, while the dash-dotted green line is the solution to the LWR model (1) with FIFO rule. The initial data is (32), while $V_\ell = 1.5$, $V_r = 2$ and $\alpha = 0.4$.

REFERENCES

[1] Adimurthi and G. D. V. Gowda, Conservation law with discontinuous flux, *J. Math. Kyoto Univ.*, 43 (2003), 27–70.
[2] J. J. Adimurthi and G. D. Veerappa Gowda, Godunov-type methods for conservation laws with a flux function discontinuous in space, *SIAM J. Numer. Anal.*, 42 (2004), 179–208.
[3] M. S. Adimurthi and G. D. V. Gowda, Optimal entropy solutions for conservation laws with discontinuous flux-functions, *J. Hyperbolic Differ. Equ.*, 2 (2005), 783–837.
[4] G. M. Coclite, M. Garavello and B. Piccoli, Traffic flow on a road network, *SIAM J. Math. Anal.*, 36 (2005), 1862–1886.
[5] E. Cristiani and F. S. Priuli, A destination-preserving model for simulating Wardrop equilibria in traffic flow on networks, *Netw. Heterog. Media*, 10 (2015), 857–876.
[6] S. Diehl, On scalar conservation laws with point source and discontinuous flux function, *SIAM J. Math. Anal.*, 26 (1995), 1425–1451.

[7] S. Diehl, Scalar conservation laws with discontinuous flux function. I. The viscous profile condition, *Comm. Math. Phys.*, 176 (1996), 23–44.

[8] A. Festa and P. Goatin, Modeling the impact of on-line navigation devices in traffic flows, *2019 IEEE 58th Conference on Decision and Control (CDC)*, Nice, France (2019), 323–328.

[9] M. Garavello, K. Han and B. Piccoli, *Models for Vehicular Traffic on Networks*, AIMS Series on Applied Mathematics, 9, American Institute of Mathematical Sciences (AIMS), Springfield, MO, 2016.

[10] M. Garavello and B. Piccoli, Source-destination flow on a road network, *Commun. Math. Sci.*, 3 (2005), 261–283.

[11] M. Garavello and B. Piccoli, *Traffic Flow on Networks. Conservation Laws Models*, AIMS Series on Applied Mathematics, 1, American Institute of Mathematical Sciences (AIMS), Springfield, MO, 2006.

[12] T. Gimse and N. H. Risebro, Solution of the Cauchy problem for a conservation law with a discontinuous flux function, *SIAM J. Math. Anal.*, 23 (1992), 635–648.

[13] P. Goatin, S. Göttlich and O. Kolb, Speed limit and ramp meter control for traffic flow networks, *Eng. Optim.*, 48 (2016), 1121–1144.

[14] P. Goatin and E. Rossi, A multiLane macroscopic traffic flow model for simple networks, *SIAM J. Appl. Math.*, 79 (2019), 1967–1989.

[15] H. Holden and N. H. Risebro, A mathematical model of traffic flow on a network of unidirectional roads, *SIAM J. Math. Anal.*, 26 (1995), 999–1017.

[16] H. Holden and N. H. Risebro, *Front Tracking for Hyperbolic Conservation Laws*, Applied Mathematical Sciences, 152, Springer, Heidelberg, 2015.

[17] H. Holden and N. H. Risebro, Models for dense multiline vehicular traffic, *SIAM J. Math. Anal.*, 51 (2019), 3694–3713.

[18] K. H. Karlsen, N. H. Risebro and J. D. Towers, $L^1$ stability for entropy solutions of nonlinear degenerate parabolic convection-diffusion equations with discontinuous coefficients, *Skr. K. Vidensk. Selsk.*, (2003), 1–49.

[19] M. J. Lighthill and G. B. Whitham, On kinematic waves. II. A theory of traffic flow on long crowded roads, *Proc. Roy. Soc. London, Ser. A.*, 229 (1955), 317–345.

[20] P. I. Richards, Shock waves on the highway, *Operations Res.*, 4 (1956), 42–51.

[21] S. Samaranayake, W. Krichene, J. Reilly, M. L. D. Monache, P. Goatin and A. Bayen, Discrete-time system optimal dynamic traffic assignment (SO-DTA) with partial control for physical queuing networks, *Transportation Science*, 52 (2018), 982–1001.

[22] B. Schnetzler, X. Louis and J.-P. Lebacque, A multiline junction model, *TRANSFORDERMETRIC*, 8 (2012), 243–260.

[23] J. D. Towers, Convergence of a difference scheme for conservation laws with a discontinuous flux, *SIAM J. Numer. Anal.*, 38 (2000), 681–698.