Seismic analysis of Kunshan Xintiandi super high-rise residential building

Xiaomeng Zhang¹, Wenting Liu¹, Qingying ren¹, Yilun Zhou, Ziao Liu¹, Xiaoyang¹

¹China Architecture Design & Research Group, Beijing 100044, China

Abstract: The Kunshan urban investment project includes 1 office tower, 4 high-rise residential buildings and 1 high-rise commercial building. Each monomer shares a large basement chassis, a total of three basement floors. This design is the second phase (4# residence and corresponding basement). The height of the structure is more than 90 meters, and it is a shear wall structure system. The selection and structure of the structure system are analyzed, and the mechanical performance of the structure is verified through calculation and analysis.

1 Page layout

1.1 Project Overview

The construction site of the project is located in Kunshan City, Jiangsu Province. The proposed site is located on Qianjin West Road to the north, Louchuang Road to the south and Sichang Road to the east. Zu Chong Road to the west. The project includes one office tower, four residential towers and one commercial tower. Each unit shares the basement, which has three floors in total. This design is 4# super high-rise residential building and corresponding basement.

Figure 1. Architectural renderings.
2.1 Arrangement of high-rise structures

The project is located in the 7 degree of seismic fortification intensity area, belongs to the standard fortification class building, should be 7 degrees of seismic calculation and seismic measures.

This project tower, the height of the structure is 84.45 meters (27 floors), the main structure of the shear wall system. As the main lateral force resistance system, the core tube not only provides resistance to wind load and horizontal earthquake action, but also bears the additional torsion effect caused by the incoincidence of the center of mass and the center of stiffness. The outer frame mainly bears the vertical load, and also bears the role of the second seismic defense line.

2.2 The setting of structural joints and post-cast belt

2.2.1 Shrink post-cast strip

The plane size of the basement of this project is 118mx470m, which is a super-long structure.

In order not to affect the use function of the building, this project does not set expansion joints, according to the "Concrete Structure Design Code" [2] (GB50010-2010) using every 30m ~ 40m interval to set up shrinkage post-pouring belt construction measures, reduce the early concrete shrinkage of the adverse impact on the structure. The width of the post-pouring strip is 0.8m. Shrinkage post-pouring belt on both sides of the members of the concrete pouring for two months, most of the concrete shrinkage after the completion of supplementary pouring. The non-shrinkage concrete with strength grade higher than that of the two side members is used for the supplementary casting of the post cast belt.

The following construction measures and construction measures are adopted in the design to reduce the adverse effects of temperature action and concrete shrinkage in the later period on the structure:

(1) The reinforcement should be as close as possible, and the proportion of the reinforcement should be increased appropriately.

(2) Strengthen the insulation measures of the skirt roof layer and the top floor of the main building, and set the outer insulation layer on the outer wall.

(3) Use of low shrinkage, low hydration heat of cement, appropriate to reduce the amount of cement. Add proper amount of fly ash to concrete.

2.3 earthquake resistant constructional measure

This project is a standard fortification building in a 7 degree zone (0.10g). As the tower is a composite structure with a high height, its seismic measures will be appropriately improved to ensure the realization of "strong column and weak beam, strong shear and weak bending, strong joint and weak component". In view of this, in the subsequent construction drawing design stage, based on accurate calculation, it is planned to adopt the method of performance design to improve the seismic performance of key components and related structures, appropriately strengthen the structural reinforcement ratio of related components, and rationally deal with the beam-to-column joints to improve the ductility of the structure. Take effective measures to reliably tie the knot with the main body for non-structural components, rear building envelope wall and partition wall.

1) Among them, the seismic grade of the shear wall is 1, and the bottom strengthening parts and key components shall be strengthened in accordance with the measures of the extra-level member to strengthen the seismic structure.

2) The floor around the large opening should be properly thickened to increase the overall stiffness of the floor and ensure the reliable transfer of horizontal force of the floor. The plate thickness should not be less than 150mm, and the two-way double-layer reinforcement should be adopted, and the reinforcement ratio should be appropriately increased. The single-layer one-way reinforcement ratio should not be less than 0.3%.

3) the use of lightweight filled wall, as far as possible to reduce the weight of the structure, reduce the earthquake action.

3 Calculation and analysis

According to the technical code for concrete structures of high-rise buildings (JGJ 3-2010) [1], The 3d analysis and design software YJK (version 3.0.3 were used for the structural analysis and design[3]. The influence of floor cavity is considered in the analysis. The superstructure is embedded in the basement roof of the first floor. The earthquake action and the wind load act in two main directions and the torsional effect under the two-way earthquake action is taken into account.
This project uses YJK as the main tool for structural design and calculation. Table 1 shows the main data of the northwest high-rise building:

Table 1. Two types of software modal cycles and vibration types

| Vibration model | Period (s) | Vibration type | period ratio [0.90] |
|-----------------|------------|----------------|---------------------|
| YJK             | 2.96       | Y              | 0.61                |
|                 | 2.598      | X              |                     |
|                 | 1.809      | torsion        |                     |

Table 2. Interlayer displacement Angle comparison of software

| small earthquakes | YJK result |                |               |
|-------------------|------------|----------------|---------------|
|                   | Interlayer displacement Angle | displacement ratio |
| X                 | 1/1154     | 1.06           |               |
| Y                 | 1/1028     | 1.06           |               |
| Specification limits | 1/1000     | 1.4            |               |

Table 3. Seismic force

| direction | The bottom shear of the super tall building (kN) | Minimum shear weight ratio for floors | allowable value |
|-----------|--------------------------------------------------|--------------------------------------|-----------------|
| X         | 4415.1                                           | 4.70%                                | 2.40%           |
| Y         | 3651.9                                           | 5.10%                                | 2.40%           |

After analysis, the overall design index is good, the first and second period is translational period, the third period is torsional period, the stiffness of the two directions of the structure is not much different, and the ratio of the first torsional period to the first translational period of the structure is less than 0.85, which meets the requirements of the code. The shear weight ratio of the two directions at the bottom of the structure meets the requirements of not less than 1.2% in the "Anti-Gauge". Both directions were less than the standard limit of 1/1000. The calculation models of the two software are basically consistent and the dynamic characteristics of the structure are basically consistent. The natural vibration period, displacement Angle between layers, displacement ratio and other indexes of the structure are similar, and all of them are in a reasonable range, which meet the requirements of the current code.

4 Conclusion

Through the reasonable selection of the structural system and the analysis by YJK software under the action of small earthquakes, all the design indexes obtained can meet the requirements of anti-gauge. Because the structure for reinforced concrete shear wall high-rise structure, in order to improve its seismic ability and ductility, through reasonable layout, the formation of the space overall stress system, and take some local strengthening measures, make the structure in the earthquake has a good seismic performance.
References

1. Technical specification for concrete structures of tall building JGJ 3 -2010 [s] Beijing China building industry press
2. Code for seismic design of buildings GB 50011-2010 [s] Beijing China building industry press
3. Load code for the design of building structures GB 5 0092012[s] Beijing China building industry press
4. Code for design of concrete structures GB50010-2010: GB 50010—2011 [S] Beijing China building industry press, 2011.
5. Xu PF, Fu XY, Wang CK. Structural design of complex high-rise buildings. Beijing China building industry press