Effective elastic properties of solids with irregularly shaped inclusions

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Abstract A unit rectangular cell is usually cut out from a medium for investigating fracture mechanism and elastic properties of the medium containing an array of irregularly shaped inclusions. It is desirable to clarify the geometrical parameters controlling the elastic properties of heterogeneous materials because they are usually embedded with randomly distributed particulate. The stress and strain relationship of the rectangular cell is obtained by an ad hoc hybrid-stress finite element method. By matching the boundary condition requirements, the effective elastic properties of composite materials are then calculated, and the effect of shape and arrangement of inclusions on the effective elastic properties is subsequently considered by the application of the ad hoc hybrid-stress finite element method through examining three types of rectangular cell models assuming rectangular arrays of rectangular or diamond inclusions. It is found that the area fraction (the ratio of the inclusion area over the rectangular cell area) is one dominant parameter controlling the effective elastic properties.

Keywords Composite material · Effective elastic properties · Cell model · Hybrid-stress finite element method

1 Introduction

With the rapid development of material science and technology, many composite materials of lightweight, high-strength and multiple-function are now developed to meet the requirement of materials in practical industry engineering. Composite materials are usually classified as two kinds of fiber-reinforced composites and particle-reinforced composites. Since effective elastic properties of composite materials vary with different shapes and arrangements of inclusions-reinforced, to manufacture desirable composite materials, it is necessary to discuss the effect of shape and arrangement of inclusion on the effective elastic properties, which is helpful for optimizing the design of composite materials.

To study effective elastic properties and ductile fracture mechanism of composite materials, both the analytical and numerical methods were applied together with unit cell models on the assumption of periodic arrangement of voids, particles, inclusions and fibers in matrix. Among the works on the analytical solution, the representative ones that should be mentioned are those of Nemat-Nasser and Hori (1993), Meguid and Zhong (1997), Deng and Meguid (1999), Zhong and Meguid (1999), Jiang et al. (2004), Hashin and Shtrikman (1962, 1963) and later publications, Budiansky (1965), Eshelby (1957) and later publications, Chen and Lee (2002) as well as Buryachenko (2007). Analytical solutions are either limited to very simple geometries such as elliptical

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inclusions or require high level of mathematical competence. As to numerical solutions, the conventional finite element method (FEM) together with unit cell models has been widely used to analyze effective elastic properties and ductile fracture mechanism of heterogeneous materials containing periodically arranged voids, particles, inclusions and fibers in matrix (Needleman 1972; Christman et al. 1989; Tvergaard 1990; Bao et al. 1991; Zahl and McMeeking 1991; Levy and Parazian 1990; Li et al. 1995; Meguid and Hu 1999; Ghosh and Mukhopadhyay 1993; Zhang and Katusbe 1995 and so on). However, due to the need for domain discretization, a lot of finite elements must be used for more accurate numerical results. Compared with the conventional FEM, the boundary integral equation method (BIEM) is a more efficient and accurate one for solving inclusion problems. The BIEM has been successfully applied for the solution of inclusion problems of various shapes (Chen 1994; Noda et al. 2000, 2003; Dong 2006). However, the method also needs the fundamental solution, which is not easy-to-obtain or impossible-to-obtain for composite materials; and the boundary integral equation must be formulated for each inclusion, which might be inconvenient for solving problems containing many inclusions of irregular shapes. In additional, it might be difficult for the method to be extended to domains having circular or curved boundary parts.

However, it should be noted that the cell models used in the previous studies always assume periodic arrangement of inclusions having the same shape and dimension. If the arrangement or shape of inclusions changes even a little, the results may be different from those shown in the publications. Recently, Noda et al. (2005) employed the conventional FEM to discuss the behavior aforementioned, and such conclusions are drawn that the projected area fraction and volume fraction of inclusions are two dominant parameters controlling effective elastic modulus of composite materials. However, only is the comparison of rectangular inclusions with ellipsoidal or elliptic inclusions shown in their study. As for the other comparisons, they did not discuss further.

In present work, a medium matrix reinforced by a great or an infinite number of rectangular or diamond in cross-section fibers will be treated as a two-dimensional model containing an array of rectangular or diamond inclusions, as shown in Fig. 1. Figure 1 gives three types of models with varying the shape and arrangement of inclusions on the basis of mechanical and physical consideration. The novelty of this paper is to combine the ad hoc finite element method with a rectangular cell model cut from the medium for a solution to the problem of a rectangular array of rectangular or diamond inclusions. The present study can be considered as a complement of

![Fig. 1 Three types of rectangular cell models. a Periodic rectangular inclusions. b Periodic diamond inclusions. c Disordered inclusions by group A inclusions and group B inclusions](image-url)
the study (Noda et al. 2005). The rectangular cell model may contain single or multiple inclusions.

2 Ad hoc hybrid-stress finite element formulation

Consider an inclusion corner under in-plane deformation as shown in Fig. 2. In Fig. 2, two dissimilar wedges with subtending angles of \( \alpha \) and \( \alpha_1 + \alpha_2 \) (\( \alpha + \alpha_1 + \alpha_2 = 2\pi \)), respectively, are bonded perfectly along both of their interfaces. Each of which is made of an isotropic material. Wedge 1 is occupied by domain \( \Omega^1 \), and wedge 2 by domain \( \Omega^2 \). Let \((r, \theta)\) be a local polar coordinate system centered at the corner \( o \) and the axis of \( \theta = 0 \) the bisector of the two wedges. Therefore, when setting \( \theta = 0 \), we have \( \alpha_1 = \alpha_2 \).

Generally speaking, the number of singularity orders is usually greater than one, and the singularity orders may be real or complex. Therefore, for multiple singularity orders \( \lambda_n \) \((n = 1, 2, \ldots, N + M)\), the total asymptotic displacements \( u(r, \theta) \) and stresses \( \sigma(r, \theta) \) around the inclusion corner in elastic materials can be expressed as:

\[
\begin{align*}
\mathbf{u}(r, \theta) &= \sum_{n=1}^{N+M} \beta^{(n)} r^{\lambda_n + 1} \mathbf{U}^{(n)}(\theta) \\
&= \sum_{n=1}^{N} \left( \beta^{(n)}_{R} \mathbf{U}^{(n)}_{R}(r, \theta) + \beta^{(n)}_{I} \mathbf{U}^{(n)}_{I}(r, \theta) \right) \\
&+ \sum_{n=1}^{M} \beta^{(n)} \mathbf{U}^{(n)}(r, \theta) = \mathbf{U}(r, \theta) \mathbf{\beta}
\end{align*}
\]

(1)

where subscripts R and I indicates the real and imaginary part of a complex variable, respectively and \( i = \sqrt{-1} \); \( \mathbf{\beta} = \left[ \beta^{(1)}_{R} \beta^{(1)}_{I} \ldots \beta^{(N)}_{R} \beta^{(N)}_{I} \beta^{(1)}_{M} \ldots \beta^{(M)}_{M} \right]^T \) are coefficients to be determined; \( N \) represents the number of complex singularity orders \( \lambda_n \) truncated and \( M \) the number of real singularity orders \( \lambda_n \) truncated; moreover, it has been arranged as \( \lambda_n \leq \lambda_{n+1} \) (herein equal means multiple roots), and in the scope of fracture mechanics, \( \text{Re}(\lambda) < -1 \) should be excluded in the series. \( \mathbf{U}^{(n)}(r, \theta), \mathbf{\Sigma}^{(n)}(r, \theta) \) and \( \mathbf{U}^{(n)}_{I}(r, \theta), \mathbf{\Sigma}^{(n)}_{I}(r, \theta) \) \((l = R, I)\) can be respectively computed from the non-dimensional angular distributions of \( \mathbf{U}^{(n)}(\theta) \) and \( \mathbf{\Sigma}^{(n)}(\theta) \) depending on the singularity orders \( \lambda_n \) and eigenvectors \( \mathbf{q}^{(n)} \). The singularity orders \( \lambda_n \) and the eigenvectors \( \mathbf{q}^{(n)} \) are numerically solved from the following characteristic equation by the one-dimensional finite element technique given by Sze and Wang (2000) as well as Chen and Ping (2008)

\[
\sum_{\epsilon^1 + \epsilon^2} \left[ (\delta \mathbf{q})^T \left( \lambda^2 \mathbf{P}^c + \lambda \mathbf{Q}^c + \mathbf{R}^c \right) \mathbf{q} \right] = 0
\]

(3)

in which \( \sum_{\epsilon^1 + \epsilon^2} \) represents the assemblage of elements belonging to wedge domain 1 and 2; \( \lambda \) is eigenvalue (also called as singularity order) and \( \mathbf{q}^c \) eigenvector, Matrices \( \mathbf{P}^c, \mathbf{Q}^c \), and \( \mathbf{R}^c \) are referred to references (Sze and Wang 2000; Chen and Ping 2008).

Following the Hellinger-Reissner principle and our work (Chen and Ping 2008), we define the following hybrid functional for a specified mixed boundary value problem in a domain around the inclusion corner consisting of two sub-domains \( \Omega^1 \) and \( \Omega^2 \), as shown in Fig. 3:

\[
\pi^c = \frac{1}{2} \int_C (\mathbf{t}^c)^T \mathbf{u}^d dS - \int_C (\mathbf{t}^e)^T \mathbf{u}^d dS
\]

(4)

where \( C = C^1 + C^2 \), \( C^1 \) and \( C^2 \) are the domain’s outer boundary corresponding to domain \( \Omega^1 \) and \( \Omega^2 \), respectively; \( \mathbf{u}^d \) and \( \mathbf{t}^e \) are, respectively, the
displacement and boundary traction vectors in the Cartesian coordinate system; based on the conventional hybrid-stress finite element method, the boundary displacements $\varepsilon$ is assumed separately from $\bar{u}$ and expressed in terms of the nodal displacements of the boundary element through an interpolation method.

Using Eqs. 1, 2 and 4, we have the following stiffness matrix of a super $n$-sided polygonal inclusion corner element embedded with the inclusion corner (see Fig. 3):

$$K_{S-n} = (G^e)^T (H^e)^{-1} (G^e)$$

where $G$ and $H$ are one-dimensional integrals along the boundaries of the super inclusion corner element as shown in Fig. 4, namely:

$$H^e = \frac{1}{2} \int_C \left[ (n^e \Sigma^e(x,y))^T U^e(x,y) 
\quad + (U^e(x,y))^T (n^e \Sigma^e(x,y)) \right] dS,$$

$$G^e = \int_C (n^e \Sigma^e(x,y))^T L dS$$

$L = \left[ \begin{array}{c} \left( 1 - \frac{s}{l} \right) I_2 \end{array} \right]$

in which $n^e$ is a $2 \times 3$ matrix of the unit normal to boundary $C$; $s$ is the distance measured from a point $p$ on the integration boundary segment to the first node of integration boundary segment as shown in Fig. 4, and $l$ is the length between two nodes of the integration boundary segment; $I_2$ is the second order identity matrix.

This ad hoc super element is used to model the near-field region and incorporated with the conventional standard four-node hybrid-stress elements in the far-field region to constitute an ad hoc hybrid-stress finite element method for the analysis of local elastic field around the inclusion corners and of macro effective elastic properties of heterogeneous materials.

Notice that the local singular stress field around the inclusion corner has been well simulated by using the super inclusion corner element. Therefore, compared with the standard four-node hybrid-stress finite element method and the conventional finite element method, the present ad hoc hybrid-stress finite element method gives generally more accurate numerical results. The reason is due to the fact that, in the standard four-node hybrid-stress finite element analysis and the conventional finite element analysis, relatively refined meshes are required to obtain the same as the accuracy of numerical solutions by the proposed method. Furthermore, the ad hoc hybrid-stress finite element approach is especially suitable for solving the elastic problems containing single or multiple singular points, for example, inclusion problems (Zhang and Katusbe 1995; Chen and Ping 2008).

3 Analysis of rectangular cell models

Following Tvergaard’s work (1990) and Noda’s study (Noda et al. 2005), the geometry and the boundary
A rectangular inclusion in a rectangular cell

conditions of a rectangular cell corresponding to longitudinal tension load can be described by Fig. 5, in which \(2l_{x2}\) and \(2l_{y2}\) are respectively the side length of the rectangular cell along x-axis and y-axis, and \(2l_{x1}\) and \(2l_{y1}\) the size of the rectangular inclusion along x-axis and y-axis in the rectangular cell, respectively.

The boundary conditions of Fig. 5 can be expressed as

(I) On \(x = -l_{x2}\) and \(-l_{y2} \leq y \leq l_{y2}\):
\[
 u_x = -u_{x0}, \quad \tau_{xy} = 0; \tag{6a}
\]

(II) On \(x = l_{x2}\) and \(-l_{y2} \leq y \leq l_{y2}\):
\[
 u_x = -u_{x0}, \quad \tau_{xy} = 0; \tag{6b}
\]

(III) On \(y = -l_{y2}\) and \(-l_{x2} \leq x \leq l_{x2}\):
\[
 u_y = -u_{y0}, \quad \tau_{xy} = 0; \tag{6c}
\]

(IV) On \(y = l_{y2}\) and \(-l_{x2} \leq x \leq l_{x2}\):
\[
 u_y = u_{y0}, \quad \tau_{xy} = 0. \tag{6d}
\]

And we also have
\[
\int_{-l_{y2}}^{l_{y2}} \sigma_y \bigg|_{y = \pm l_{y2}} dx = \sigma_{y0} \times 2l_{x2}, \quad \int_{-l_{x2}}^{l_{x2}} \sigma_x \bigg|_{x = \pm l_{x2}} dy = 0 \tag{7}
\]

We have to set constants \(u_{x0}\) and \(u_{y0}\) of Eq. 6 so as to satisfy Eq. 7. However, since they are still unknown, one has to decompose the original mixed boundary value problem in Fig. 5 into two auxiliary mixed boundary value problems as shown in Fig. 6a, b. Each of which can be solved by using the present method discussed in the proceeding section. Firstly, the first auxiliary problem given in Fig. 6a is solved under the boundary conditions presented in Eqs. 8 and 9. Herein, this \(C_1\) is an arbitrary constant.

(I) On \(x = -l_{x2}\) and \(-l_{y2} \leq y \leq l_{y2}\):
\[
 u_x = 0, \quad \tau_{xy} = 0; \tag{8a}
\]

(II) On \(x = l_{x2}\) and \(-l_{y2} \leq y \leq l_{y2}\):
\[
 u_x = 0, \quad \tau_{xy} = 0; \tag{8b}
\]

(III) On \(y = -l_{y2}\) and \(-l_{x2} \leq x \leq l_{x2}\):
\[
 u_y = -C_1, \quad \tau_{xy} = 0; \tag{8c}
\]

(IV) On \(y = l_{y2}\) and \(-l_{x2} \leq x \leq l_{x2}\):
\[
 u_y = C_1, \quad \tau_{xy} = 0. \tag{8d}
\]

Then, the resultant force \(\sigma_{x1} \times 2l_{x2}\) in the y-direction on the boundary \(y = \pm l_{y2}\) within \(-l_{x2} \leq x \leq l_{x2}\), and the resultant force \(\sigma_{x1} \times 2l_{y2}\) in the x-direction on the boundary \(x = \pm l_{x2}\) within \(-l_{y2} \leq y \leq l_{y2}\) are calculated as shown in Eq. 9.
\[
\int_{-l_{x2}}^{l_{x2}} \sigma_y \bigg|_{y = \pm l_{y2}} dx = \sigma_{y0} \times 2l_{x2}, \tag{9a}
\]
\[
\int_{-l_{y2}}^{l_{y2}} \sigma_x \bigg|_{x = \pm l_{x2}} dy = \sigma_{x0} \times 2l_{y2} \tag{9b}
\]

Next, the second auxiliary problem shown in Fig. 6b is solved under the following boundary conditions:

(I) On \(x = -l_{x2}\) and \(-l_{y2} \leq y \leq l_{y2}\):
\[
 u_x = C_2, \quad \tau_{xy} = 0; \tag{10a}
\]

(II) On \(x = l_{x2}\) and \(-l_{y2} \leq y \leq l_{y2}\):
\[
 u_x = C_2, \quad \tau_{xy} = 0; \tag{10b}
\]

(III) On \(y = -l_{y2}\) and \(-l_{x2} \leq x \leq l_{x2}\):
\[
 u_y = 0, \quad \tau_{xy} = 0; \tag{10c}
\]
(IV) On \( y = l_{x_2} \) and \(-l_{x_2} \leq x \leq l_{x_2} \):
\[ u_y = 0, \tau_{xy} = 0. \]  

(10d)

Herein constant \( C_2 \) is also arbitrary.

Then, the resultant force \( \sigma_{y_2} \times 2l_{x_2} \) in the \( y \)-direction on the boundary \( y = \pm l_{x_2} \) within \(-l_{x_2} \leq x \leq l_{x_2} \), and the resultant force \( \sigma_{x_2} \times 2l_{x_2} \) in the \( x \)-direction on the boundary \( x = \pm l_{x_2} \) within \(-l_{x_2} \leq x \leq l_{x_2} \) are calculated from the following Eq. 11.

\[
\int_{-l_{x_2}}^{l_{x_2}} \sigma_{y_2} \mathbf{1}_{y=\pm l_{x_2}} \, dx = \sigma_{x_2} \times 2l_{x_2},
\]

\[
\int_{-l_{x_2}}^{l_{x_2}} \sigma_{x_2} \mathbf{1}_{x=\pm l_{x_2}} \, dy = \sigma_{x_2} \times 2l_{x_2}
\]  

(11)

In order to recover the original problem from the two auxiliary problems, the solution for Fig. 5 can be obtained by superposing the solution for Fig. 6a and that for Fig. 6b. Comparing Eqs. 11 and 9 with Eq. 7 and denoting the solutions of Figs. 5, 6a, b as \( \sigma_\text{a}, \sigma_\text{b}, \) and \( \sigma_\text{c} \), respectively, one has

\[
\sigma_a = \sigma_{y_0} \left( \frac{\sigma_b - \sigma_{x_1} \sigma_{y_1}}{\sigma_{x_2}} \right) \left( \frac{\sigma_a - \sigma_{x_1} \sigma_{y_1}}{\sigma_{x_2}} \right)
\]  

(12a)

\[
u_a = \sigma_{y_0} \left( \frac{\sigma_b - \sigma_{x_1} \sigma_{y_1}}{\sigma_{x_2}} \right) \left( \frac{\sigma_a - \sigma_{x_1} \sigma_{y_1}}{\sigma_{x_2}} \right)
\]  

(12b)

4 Effective elastic properties of composite materials

Locally, composites are heterogeneous. However, sufficiently large samples behave homogeneously. Therefore, similar to doubly periodic inclusion problems (Dong 2006), a composite material containing disordered array of inclusions of irregular shapes discussed in this paper can also be considered as a homogeneous orthotropic material. Its constitutive relation is given as follows (Lekhnitskii 1963):

\[
\varepsilon_{11} = \frac{1}{E_1} \sigma_{11} - \frac{\nu_{21}}{E_1} \sigma_{22}
\]  

(13a)

\[
\varepsilon_{22} = \frac{1}{E_2} \sigma_{22} - \frac{\nu_{12}}{E_2} \sigma_{11}
\]  

(13b)

\[
\gamma_{12} = \frac{1}{G_{12}} \sigma_{12}
\]  

(13c)

In order to obtain the effective elastic properties \( E_2 \) and \( \nu_{21} \), the remote loading can be assumed as

\[
\sigma_{11}^\infty = 0
\]  

(14a)

\[
\sigma_{22}^\infty = \sigma_{y_0}
\]  

(14b)

\[
\sigma_{12}^\infty = 0
\]  

(14c)

For the effective homogeneous orthotropic material, its strain and stress states are given as follows:

\[
\bar{\varepsilon}_{11} = \frac{\bar{u}_a}{l_{x_1}}
\]  

(15a)

\[
\bar{\varepsilon}_{22} = \frac{v_a}{l_{x_1}}
\]  

(15b)

\[
\bar{\sigma}_{11} = 0
\]  

(15c)

\[
\bar{\sigma}_{22} = \sigma_{y_0}
\]  

(15d)

\[
\bar{\sigma}_{12} = 0
\]  

(15e)

in which the bar over the above given components denotes the average values of components in unit cell; \( \bar{u}_a \) and \( \bar{v}_a \) are calculated from Eq. 12b.

Substitution of Eq. 15 into Eq. 13, yields

\[
E_2 = \frac{\sigma_{22} l_{x_1}}{\nu_{a}}
\]  

(16a)

\[
\nu_{21} = -\frac{l_{x_1} \bar{u}_a}{l_{x_1} \bar{v}_a}
\]  

(16b)

Similarly, we have

\[
E_1 = \frac{\sigma_{11} l_{x_1}}{\nu_{a}}
\]  

(17a)

\[
\nu_{12} = -\frac{l_{x_1} \bar{v}_a}{l_{x_1} \bar{u}_a}
\]  

(17b)

\[
G_{12} = \frac{\sigma_{12} l_{x_1}}{\left( \frac{l_{x_1} \bar{u}_a}{l_{x_1} \bar{v}_a} + \frac{l_{x_1} \bar{v}_a}{l_{x_1} \bar{u}_a} \right)}
\]  

(17c)

For composite materials consisting of various inclusions, macro effective elastic properties can be calculated from Eqs. 16 and 17 by using the proposed ad hoc hybrid-stress finite element method based on the computational models presented in Sect. 3.

5 Numerical results and discussion

Numerical solutions will be shown for the example of the boundary conditions around the inclusion corners tip A (see Fig. 1). Similar procedures can be given for
other boundary conditions. Figure 7 illustrates the element mesh divisions around the inclusion corner.

5.1 Doubly periodic square inclusions

Geometric data of the cell of a doubly periodic square inclusions is taken as \( h/l = 0.8 \), and the side length \( a \) of square inclusions is varied (see Fig. 8a). The elastic modulus and the Poisson ratio of the matrix are set to be \( E_M = 1 \) and \( \nu_M = 0.3 \). The elastic modulus and the Poisson ratio of the inclusion are taken as \( E_I \) and \( \nu_I = \nu_M = 0.3 \), respectively.

This problem has been analyzed by using the boundary element method (Dong 2006). In the present analysis, because of the symmetry of the geometry and the loading conditions, it is sufficient analyze only a quarter of the cell. The novel hybrid-stress finite element mesh division and the standard four-node hybrid-stress finite element mesh division are given in Fig. 8b, c, respectively. At four different element number levels, comparisons of the effective elastic properties obtained by using the novel ad hoc hybrid-stress and standard four-node hybrid-stress finite element methods with Dong’s numerical results (Dong 2006) are given in Table 1. It can be seen from Table 1 that the numerical solutions from the novel ad hoc hybrid-stress and standard four-node hybrid-stress finite element methods are in excellent agreement with those of Dong (2006); either from the number of elements or from the accuracy of results the novel ad hoc hybrid-stress finite element method is better than the standard four-node hybrid-stress finite element method, which confirms the observations in last paragraph of Sect. 2.

Under tension load in \( y \) direction, effective elastic properties of the composite material for various square inclusions are shown in Fig. 9a, b with solid lines. Dong’s numerical results (Dong 2006) are also

| Table 1 | Comparison of present results of equivalent elastic modulus with those of Dong (2006) when \( h/l = 0.8 \), \( a/l = 0.8 \) and \( E_I/E_M = 10 \) |
|-----------------|-----------------|-----------------|-----------------|-----------------|
|                | Elem. size      | Elem. number    | Equ. elastic modulus | Relative errors (%) |
| Novel hybrid-stress FEM | 0.02l          | 252             | 2.834            | 2.838            | −0.11          |
|                  | 0.04l          | 192             | 2.834            | 2.834            | −0.11          |
|                  | 0.08l          | 140             | 2.834            | 2.834            | −0.11          |
|                  | 0.16l          | 69              | 2.835            | 2.835            | −0.07          |
| Conventional hybrid-stress FEM | −              | 255             | 2.841            | 2.838            | 0.14           |
|                  | −              | 195             | 2.842            | 2.842            | 0.18           |
|                  | −              | 143             | 2.842            | 2.842            | 0.18           |
|                  | −              | 72              | 2.843            | 2.843            | 0.21           |
marked in Fig. 9a, b with open circles. One can find the results from the ad hoc hybrid-stress finite element method coincide with those of Dong (2006) within error of 0.2%.

5.2 Doubly periodic rectangular inclusions

Consider a composite material consisting of a matrix filled with a square array of rectangular inclusions. A square cell \((l_x/l_x = 1)\) containing single rectangular inclusion is cut from the composite, as shown in Fig. 10a. The rectangular inclusion has dimensions \(a\) and \(b\). Plane stress condition with Poisson ratio \(v_t = v_M = 0.3\) is assumed for the matrix and the inclusions, whose elastic modulus are \(E_M\) and \(E_I\), respectively. Similarly, only is a quarter of the cell used for element mesh division in the present numerical calculations (see Fig. 10b). In Fig. 11, the present solutions and Noda’s results (Noda et al. 2005) are compared. It can be seen from Fig. 11 that their both results agree well; the effective elastic modulus is not identical even though the volume fraction \(V_I = \text{constant}\), in which \(V_I = ab/(l_x l_y)\); the effective elastic modulus is dominantly controlled by the geometric area fraction \(A_I(a/l_x\ or\ b/l_y)\) and volume fraction \(V_I\).

In the numerical calculations, 221 elements including one super inclusion corner element are employed.

5.3 Doubly periodic diamond inclusions

Geometric data of the square cell of a doubly periodic diamond inclusions is taken as \(l_x/l_x = 1\), and the diagonal length \(a'\) or internal angle \(\Delta\) of diamond inclusions is varied (see Fig. 12a). The diamond inclusion has dimensions \(a', b'\) and \(\Delta\). The elastic modulus and the Poisson ratio of the matrix are taken as \(E_M = 1\) and \(v_M = 0.3\). The elastic modulus and the Poisson ratio of the inclusion are set to be \(E_I\) and \(v_I = v_M = 0.3\), respectively. Only is the right area of the cell used for the element mesh division due to the symmetry (see Fig. 12b). Figure 13a, b depict the variations of the effective elastic modulus against \(E_I/E_M\). It can be seen from Fig. 13a, b that the effective elastic properties increase monotonously
with the increase of $E_I/E_M$, implying that inclusion filling is useful for improving the elastic properties of porous matrix; under tension load along $y$-axis, the effect of the internal angle $\Delta$ of diamond inclusion on $E/E_M$ increases with increasing $\Delta$, but the effect of the internal angle $D$ of diamond inclusion on $\nu_{21}$ is just contrary in most cases. Figure 14 confirms further such a conclusion, which is given in the proceeding section, that the effective elastic modulus is dominantly controlled by the geometric area fraction $A_I(a'/l_x$ or $b'/l_y)$ and volume fraction $V_I$. When the rectangular unit cell has the dimensions $2l_x$ and $2l_y$, the volume fractions of inclusion $V_I = ab/(l_xl_y)$ for the rectangular inclusion and $V_I = a'b'/(2l_xl_y)$ for the diamond inclusion. Figure 14 tells us such a fact that the effective elastic modulus is not equal even though $V_I = a'/l_x = V_I = a'/l_x$. This fact is in contradiction with Noda’s conclusion (Noda et al. 2005), drawn after he compared the rectangular inclusions with the elliptical inclusions, that irregularly shaped inclusions may practically be evaluated from the equivalent rectangular inclusions in the case of equal geometric area fraction $A_I$ and volume fraction $V_I$. Therefore, for composite materials containing diamond and triangular inclusions, we think that their effective elastic properties may not generally be evaluated from the equivalent rectangular inclusions.

5.4 Disordered rectangular inclusions

From the aforementioned discussions we conclude that the effective elastic properties are controlled by two dominant geometric parameters, i.e., the area fraction $A_I$ and volume fraction $V_I$. However, it should be noted that the conclusion is drawn from the analysis of periodic inclusion problems. For non-periodic or disordered inclusion problems, whether can we draw the same conclusion or not? To answer the question, we have to examine the disordered inclusion problems. Take the model of Fig. 1c as an example. Herein the position of group A is assumed to be fixed but that of group B movable. The
rectangular unit cell has dimensions \( l_x \times l_y \) and the rectangular inclusion \( 2a \times 2b \). The area fraction \( A_1 = a/l_x \) or \( b/l_y \) and the volume fraction \( V_l = 8ab/(l_x \times l_y) \). Figure 15 shows the configuration of the rectangular unit cell model containing disordered inclusions. This model simulates well a composite material consisting of a great number or an infinite number of periodically arranged groups of the disordered inclusion problems. In the numerical calculations, Poisson’s ratio \( \nu_l = \nu_M = 0.3 \) and plane stress condition are assumed. The elastic modulus ratio \( E_l/E_M \) is varied. Tables 2, 3, 4, and 5 gives respectively the effective elastic modulus \( E \) as a function of the position of group B varying within

![Fig. 15 Dimensions of two-groups-inclusion model with point \( \phi \) varying in the range of \( 0 \leq x \leq l_x/2 \) and \( 0 \leq y \leq l_y/2 \) where both group have the Poisson’s ratio \( \nu_l = \nu_M = 0.3 \) under plane stress](image)

**Table 2** Effective elastic modulus \( E/E_M \) versus the central coordinate of group B varying in range of \( 0 \leq x \leq l_x/2 \), \( 0 \leq x \leq l_x/2 \) when \( l_x/l_x = 1 \), \( E_l/E_M = 10^3 \) and \( \nu_l = \nu_M = 0.3 \)

| \( y/l_x \) | \( x/l_x \) |
|------------|------------|
| 0.0        | 1.190      |
| 0.1        | 1.260      |
| 0.2        | 1.356      |
| 0.3        | 1.508      |
| 0.4        | 1.672      |
| 0.5        | 1.844      |

**Table 3** Effective elastic modulus \( E/E_M \) versus the central coordinate of group B varying in range of \( 0 \leq x \leq l_x/2 \), \( 0 \leq x \leq l_x/2 \) when \( l_x/l_x = 1 \), \( E_l/E_M = 10^5 \) and \( \nu_l = \nu_M = 0.3 \)

| \( y/l_x \) | \( x/l_x \) |
|------------|------------|
| 0.0        | 1.140      |
| 0.1        | 1.170      |
| 0.2        | 1.228      |
| 0.3        | 1.266      |
| 0.4        | 1.253      |
| 0.5        | 1.223      |

**Table 4** Effective elastic modulus \( E/E_M \) versus the central coordinate of group B varying in range of \( 0 \leq x \leq l_x/2 \), \( 0 \leq x \leq l_x/2 \) when \( l_x/l_x = 1 \), \( E_l/E_M = 10^{-5} \) and \( \nu_l = \nu_M = 0.3 \)

| \( y/l_x \) | \( x/l_x \) |
|------------|------------|
| 0.0        | 0.910      |
| 0.1        | 0.901      |
| 0.2        | 0.894      |
| 0.3        | 0.902      |
| 0.4        | 0.902      |
| 0.5        | 0.907      |

**Table 5** Effective elastic modulus \( E/E_M \) versus the central coordinate of group B varying in range of \( 0 \leq x \leq l_x/2 \), \( 0 \leq x \leq l_x/2 \) when \( l_x/l_x = 1 \), \( E_l/E_M = 10^{-3} \) and \( \nu_l = \nu_M = 0.3 \)

| \( y/l_x \) | \( x/l_x \) |
|------------|------------|
| 0.0        | 0.890      |
| 0.1        | 0.876      |
| 0.2        | 0.863      |
| 0.3        | 0.854      |
| 0.4        | 0.891      |
| 0.5        | 0.923      |

range \( 0 < x \leq l_x/2 \) and \( 0 < y \leq l_y/2 \) corresponding to different elastic modulus ratio \( E_l/E_M \). Note bold value with underline in Tables 2, 3, 4, and 5 represents maximum one or minimum one. It can be seen from Tables 2, 3, 4, and 5 that the biggest difference between maximum \( E/E_M \) and minimum \( E/E_M \) within range \( 0 < x \leq l_x/2 \) and \( 0 < y \leq l_y/2 \) is 0.548 (=1.738–1.190) and occurs in Table 2.
Effective elastic properties by the ad hoc FEM analysis

Therefore, such a conclusion may be drawn that the effective elastic properties are almost independent of the position of group B if group A and group B are not overlapped. In other words, the area fraction and volume fraction of inclusion are still two dominant geometric parameters controlling the effective elastic properties of the model in Fig. 1c. Therefore, we can draw a conclusion that the effective elastic properties for all practical composite materials can be approximately and efficiently evaluated from these two dominantly controlling geometric parameters, i.e., the area fraction and volume fraction of inclusion.

6 Conclusions

In the present work, the effect of shape and arrangement of inclusions on the effective elastic properties of composite materials has been discussed by the present ad hoc hybrid finite element method together with the rectangular unit cell models. Through present analysis, some conclusions can be drawn as below:

(1) The effect of shape of inclusions is studied from the results of rectangular and diamond inclusions. The effective elastic properties are dominantly controlled by the two geometric parameters such as the area fraction $A_I$ and volume fraction $V_I$.

(2) That the effective elastic modulus of a matrix containing diamond inclusions is not equal even though equal area fraction $A_I$ and volume fraction $V_I$ is observed. This observation is in contradiction with Noda’s conclusion (Noda et al. 2005), after he compared the rectangular inclusions with the elliptical inclusions. Therefore, for composite materials containing diamond and triangular inclusions, we think that their effective elastic properties may not generally be evaluated from the equivalent rectangular inclusions.

(3) The effect of disordered inclusions is discussed by the use of the model in Fig. 1c. We have found that the effective elastic properties are almost independent of the position of group B if group A and group B are not overlapped. In other words, the area fraction and volume fraction of inclusion are still two dominant geometric parameters controlling the effective elastic properties.

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