Identifying thermo-mechanical induced microstructural changes

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Abstract. Robust engineering of geomaterials for energy applications requires a clear understanding of the impacts of temperatures and pressures applied to the soil on their microstructures. Such understandings will facilitate better designs of new geomaterials and technologies via ensuring accurate assessments of the performance of the existing ones. In this study, we assess the changes in the microstructure—specific surface area and pore size distribution—of a saturated clay subjected to stress and temperature cycle. Clay specimens were subjected to the desired mechanical stresses and thermal cycles in a triaxial system. Then, the specimens were swiftly extracted from the triaxial, flush frozen in liquid nitrogen, then freeze-dried to preserve their microstructure. The preserved specimens were then used for specific surface area and pore size distribution assessments using nitrogen (N2)-gas adsorption and mercury intrusion porosimetry. The results established qualitative explanations of the expected microstructural changes in geomaterials under operational conditions, which facilitate the development of new geomaterials that can overcome such alternations.

1 Introduction

Engineering new geomaterials rely on recognizing the unfavored responses of existing ones under different loading conditions and resolving the associated problems. The techniques to overcome such unfavored behaviors of existing geomaterials mandate adopting a bottom-up engineering approach. In this approach, new geomaterials will need to be engineered at the small (i.e., micro) scale to have favorable large (i.e., macro) scale behaviors. Thus, we first need to identify the effects of each external loading condition on the geomaterial micro-scale and relate these microstructural changes to macro-scale responses. In this study, we identify the thermally-induced pore structure changes of a saturated clay due to a temperature cycle with the overarching aim to guide the development of new geomaterials or treatments to limit these changes as desired.

Based on our experimental results, we recognize that:

(1) the pore size distribution is sensitive to temperature on both extremes (i.e., heating and freezing).

(2) The evolution of specific surface area is more significant when clay is subjected to elevated temperatures than freezing temperatures.

In this paper first, the experimental procedures of microstructural study are introduced and the results are discussed. Several new applications have increased the interest to investigate the thermal and mechanical behavior of soils including energy foundations, nuclear waste repository, and the deteriorating national infrastructure. Depending on the considered application, the temperature range of interest varies from temperatures below zero for arctic systems [1, 2], through soils under freezing-thawing cycles [3-5], to elevated temperatures and heating-cooling cycles [6-13].

We currently know that freezing cohesive soils increases their void ratio due to the thermal expansion of pore water as it turns into ice [14-17]. Additionally, we also know that thawing frozen clay results in an overall volume contraction associated with reductions in the void ratio [4, 18]. Despite this decrease in the void ratio, the hydraulic conductivity of a saturated clay after freezing-thaw cycles was reported to increase [19, 20]. Such unexpected contradicting behavior was attributed to changes in soil microstructure due to freezing and thawing [14]. On the other temperature extreme, we know that normally consolidated saturated clays experience thermoplastic contractions upon heating [21, 22]. These thermoplastic contractions occur as heating initially develops pore water pressures [23]. These generated pore water pressures decrease the effective stress [24] causing the physicochemical bonds between clay particles to become weak [25]. These softened bonds trigger partially collapse

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of the clay structure through particle reorientations [15, 16]. These thermally induced changes in the clay microstructure alter its mechanical response including shear strength and stiffness.

2 Materials and Sample Preparation

2.1 Materials

All the experiments performed in this study used Edgar Plastic Kaolin (EPK) from Edgar, FL. This clay has 100% of its particles finer than 25 μm as determined using hydrometer analysis according to ASTM D7928-17 [26]. More than 96% of the EPK clay minerals were identified as kaolinite minerals [16]. The EPK clay, however, classifies as clay with high plasticity (CH) according to the unified soil classification system (USCS) with plastic and liquid limits of 32 and 67%, respectively [14, 16].

2.2 Sample preparation

Initially, the clay bulk samples were prepared under one-dimensional consolidation. For this purpose, the EPK clay powder was mixed with deionized water at 1.5 the liquid limit. The slurry was then poured into a compaction mold and was subjected to one-dimensional consolidation under only the weight of the loading cap (~1 kg) for 24 hours. Afterward, the consolidation was carried out under incremental loading up to a maximum vertical stress of 100 kPa. Afterward, the clay blocks were extruded from the molds.

Five triaxial specimens, with ~35 mm diameter and ~76 mm height, were trimmed out of these blocks. To assess the consistency of the sample preparation procedure in this study, the initial microstructure of the trimmed samples was first evaluated. For this purpose, representative samples with a thickness of 5 mm were taken from the top of the cut soil from each triaxial sample. The microstructures of the samples used for this assessment were preserved using flash-freezing by immersing them into liquid nitrogen (N₂) for 15 minutes followed by freeze-drying for 24 hours.

2.3 Samples for post thermal cycles assessment

The trimmed cylindrical triaxial specimens were mounted in a thermo-mechanical triaxial apparatus manufactured by GDS instruments. This apparatus is able to change the temperature of the specimens between -20 °C to +65 °C using an internal coil connected to an external temperature control unit. Once mounted, each specimen was first saturated to a minimum Skempton pore water parameter B of at least 0.95. Then, the specimens were isotopically consolidated under 400 kPa effective stress at 20 °C constant temperature. These saturation and consolidation stages were performed under the top “single” drainage condition, i.e., the bottom drainage line was closed. After consolidation, each specimen was subjected to its respective thermal cycles (Table 1) with access to water supply from the top only (i.e., single drainage).

When the thermal cycle for each specimen was completed, the specimen was swiftly extracted from the triaxial cell. The triaxial sample for Isothermal Consolidation was extracted out of the triaxial cell right after the consolidation stage at 20 °C, i.e., without any temperature cycles, to evaluate the effects of saturation and consolidation on the microstructure of the soil. Evaluating these effects allows comparing the initial pre- and final post-thermal cycle fabrics. Representative samples were taken from the top of each specimen using a thin wire. These representative samples were then freeze-dried following the same procedure used for the initial representative samples taken from the triaxial sample.

Subsequently, N₂-gas sorption was performed using NOVA 2200e surface area analyzer apparatus to measure the specific surface area (SSA) using Brunauer-Emmett-Teller (BET) method. This method explains the physical adsorption of gas molecules to a solid surface by extending Langmuir’s unimolecular layer theory [27]. Additionally, the pore size distributions of the representative samples were measured using mercury intrusion porosimeter (MIP) Quantachrome Poremaster.

3 Results and discussions

BET analysis and mercury intrusion experiments were performed on representative samples obtained from the cut soil from the triaxial samples to assess the uniformity of the initial state of microstructure among all triaxial samples. Table 2 presents the results of specific surface area measurements for samples at the top of the clay bulk mold which was prepared under one-dimensional consolidation under 100 kPa. The average specific surface area was 34.81 m²/gm and the standard deviation was ±1.27. Fig. 1 demonstrates the results of mercury intrusion for 4 samples representing the initial microstructure. There exists a good agreement between all pore size distributions in Fig. 1 indicating that in this study is satisfactory to produce comparable microstructures.

Table 1. Temperature cycles used for each sample in this study.

| Sample Name           | Temperature Cycles (°C) |
|-----------------------|-------------------------|
| Isothermal Consolidation | 20                      |
| Freezing (F)          | 20 → (-20)              |
| F-Heating (FTH)       | 20 → (-20) → 65         |
| FTH-Cooling (FTHC)    | 20 → (-20) → 65 → 20    |

Table 2. Specific surface area (SSA) measurement results of initial samples from soil cut from triaxial samples. 

| Sample | SSA (m²/g) |
|--------|------------|
| Initial| 34.407 34.407 33.922 |
|        | 34.139 34.923 35.438 |
3.1 Consolidation impact on PSD

The pore size distribution (PSD) as well as the volume of intruded mercury normalized by the sample initial weight for Isothermal Consolidation at the top and its respective trimming sample are presented in Fig. 2. The peak of the curves in Fig. 2(a) represents the dominating pore diameter within each sample [2, 28].

3.2 Impacts of thermal cycles on PSD

Fig. 3 overlays the pore size distribution and normalized intruded volume of mercury into the representative samples taken from the top of Isothermal Consolidation and Frozen samples. The results suggest that during freezing process, pores with sizes ranging 10 to 100 nm were generated while the pores with diameter more than 100 nm remained about the same. These newly generated pores increased the volume of pores with diameters less than 100 nm and as a result the dominant pore diameter became smaller comparing to the sample after mechanical consolidation.

Going to thawing, the excess water in the sample, sucked into the sample due to the negative pore water pressure during freezing, seeped out of the sample as it thaws. Because of this outflow of pore water, the pores tended to shrink and recover their initial diameter. The comparison between the microstructure at the end of mechanical consolidation and freezing thawing cycle is presented in Fig 4. As per Fig. 4(b), the total volume of pores after freezing thawing was higher comparing to Isothermal Consolidation sample. On the other hand, Fig. 4(a) shows almost identical pore size distribution before and after freezing and thawing cycle. X-ray diffraction studies on effect of temperature change on particle orientation of kaolinite clay reveals that the particles were reoriented during freezing; in the other words, freezing a sample deformed the shape of the pores by particle reorientation and sliding, on the other hand, the induced reorientation during freezing was more or less recovered under thawing [23]. Considering this phenomenon and the porosimetry results, in a freezing-thawing cycle the...
plastic reorientation of clay particles as well as the generated microcracks during freezing prevents a complete pore size recovery after thawing. Moreover, the microstructural change due to heating at the top of the sample is presented in Fig. 5. While the dominant pore diameter was smaller after the elevated temperature, the total volume of the pores increased. This observed phenomenon suggests that while the water flows out during heating, the large pores not only shrank but also collapsed into a greater number of smaller pores. Finally, Fig. 6 presents the comparison of pore size distribution at the top of the sample at the end of a full temperature cycle. A plastic contraction resulting in smaller pores can be observed at the end of cooling which agrees with previous studies on heating normally consolidated clays [17, 18].

3.3 Temperature cycle impact on SSA

Three gas sorption experiments were done on representative samples taken from the top and bottom of triaxial samples presented in Table 1 and the measured values are listed in Table 3. The results were then averaged for Isothermal Consolidation sample as

Fig. 4. Pore size distribution (a) and normalized intruded volume (b) for samples from the top of Isothermal Consolidation and Freezing-Thawing.

Fig. 5. Pore size distribution (a) and normalized intruded volume (b) for samples from the top of Isothermal Consolidation and Freezing-Thawing-Heating.

Fig. 6. Pore size distribution (a) and normalized intruded volume (b) for samples from the top of Isothermal Consolidation and Freezing-Thawing-Heating-Cooling.
pore volume. Moreover, heating decreased the dominant plastic changes of microstructure maintained the higher almost recovered the initial pore size distribution, the however freezing increased both. Furthermore, as thawing pores decreased due to mechanical consolidation, the method. The dominant pore size and total volume of the area of measured using mercury intrusion and BET after each stage of a temperature cycle was compared to the initial value. This phenomenon displays the non-elastic change in the microstructure of the normally-consolidated clay.

Table 3. Measure and average values of specific surface area.

| SAMPLE                        | SSA (m²/g) | Avg. (m²/g) |
|-------------------------------|------------|-------------|
| Isothermal Consolidation      | 33.995     | 33.020      |
| Freezing                      | 35.386     | 34.626      |
| Freezing-Thawing              | 34.995     | 33.721      |
| Freezing-Thawing-Heating      | 34.475     | 33.896      |
| Freezing-Thawing-Heating-Cooling | 33.083   | 33.757      |

well as each temperature change. The measured values of SSA for each temperature change were compared to the microstructural measurements of the Isothermal Consolidation samples. The percentages of the change in the specific surface area for each sample are presented in Fig 7. Gas sorption results suggest that specific surface area increases as the temperature of the sample varies. Finally, the increase in specific surface area at the end of the thermal cycle was irreversible and did not recover the initial value. This phenomenon displays the non-elastic change in the microstructure of the normally-consolidated clay.

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4 Conclusion

In this study, the evolution of the microstructure of a saturated kaolinite clay under mechanical consolidation and under each stage of a temperature cycle was evaluated. For this purpose, five triaxial kaolinite clay samples were consolidated under 400 kPa, and then each sample was subjected to a specific thermal path. These temperature changes completed a freezing-thawing-heating-cooling cycle. Representative samples from the top and bottom of triaxial samples, extracted after mechanical consolidation, and each thermal path was taken. Flash freezing and freeze-drying techniques were then used to preserve the microstructure of each sample. afterward, the pore size distribution and specific surface area of measured using mercury intrusion and BET method. The dominant pore size and total volume of the pores decreased due to mechanical consolidation, however freezing increased both. Furthermore, as thawing almost recovered the initial pore size distribution, the plastic changes of microstructure maintained the higher pore volume. Moreover, heating decreased the dominant pore size while increased the total volume of the pores which require more investigation. Finally, after cooling, the irreversible change in pore size distribution and total volume of pores was observed. However, the specific surface area increased throughout the thermal cycle comparing to the initial value suggesting the change of not only pore volume but also pore shape and number of the pores.

Fig. 7. Change in specific surface area versus temperature at the top of the sample.

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