Investigation About Variation Law of Frost Heave Force of Seasonal Cold Region Tunnels: A Case Study

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The frost heave force (FHF) is one of the most important reasons of inducing freezing damages of tunnels in seasonal frozen regions. Analyzing the change law of FHF is helpful to design for avoiding freezing damages in the tunnel lining. Field measurement is an approved method used in mechanical analysis of tunnel lining, however, it is difficult to measure the FHF directly, which is changed coupled with the original surrounding rock pressure (OSRP). In this paper, in order to clarifying the change law of the FHF, a new measuring method was presented. The best advantage of this method is that the FHF could be separated from the OSRP by measuring the surrounding rock temperature and the lining pressure at the same time. A lot of field measurement work about the FHF was conducted based on Jichoushan tunnel and the change law of the FHF was analyzed using the presented method. The results can provide preference for the antifreeze design of tunnel in seasonal cold regions.

Keywords: cold region tunnels, surrounding rock temparature, frost heave force, field measurement, variation law

INTRODUCTION

China consists of about 54% seasonal frozen surface, where a large number of tunnels have been constructed for the increasing requirement of transportation (Feng et al., 2016; Lai et al., 2018; Feng et al., 2019; Fan et al., 2019; Fan et al., 2020). These tunnels in cold region are often affected by the seasonal frost heave force (FHF) (Zhang et al., 2004; Lai et al., 2005; Bronfenbrener, 2009; Gao at al., 2012; Lai et al., 2016; Feng at al., 2017; Hu at al., 2018; Wang et al., 2018; Xia et al., 2018; Xia et al., 2018b; Lyu et al., 2020; Zhao et al., 2020; Wang et al., 2021a; Wang et al., 2021b), causing lining cracks, drainage system failure, etc. A few tunnels even are abandoned due to the freezing damages (Feng et al., 2017). These damages induced by FHF seriously threaten the tunnel operation safety. It is of great significance to investigate the change law of the FHF on the tunnel lining for avoiding the freezing damages of tunnel in seasonal cold regions (Tan et al., 2011; Jiang et al., 2016; Li et al., 2017; Zeng et al., 2017; Liu et al., 2018; Xia et al., 2018; Huang et al., 2020).

As is known to us all, the FHF is one of the most important reasons of causing freezing damages in the tunnel lining (Lai et al., 2016), so it’s helpful for us to investigate the change law of FHF. Field measurement method is an approved approach to study the mechanical properties of tunnel lining, however, it is difficult to measure the FHF directly due to the effect of the OSRP. In fact, the OSRP and the FHF are coupled acting on the lining support, we can’t easily distinguish them. Even though, many researches about field measurement have been done to understand the change law of the tunnel temperature (Kozlowski, 2009; Tan et al., 2014; Zhou et al., 2016; Jun et al., 2017; Li et al., 2017; Xing et al., 2017; Zeng et al., 2017; Zhang G et al., 2017; Zhao et al., 2019; Wu et al., 2020). Jun et al. (2017)
measured the temperature of the air inside the tunnel, the road pavement, and the lining surfaces of a total of 104 surveyed tunnels, then he analyzed the statistical law of data. Xing et al. (2017) presented a new innovation system for temperature measurement, and monitored the temperature in the process of construction and operation of tunnel lining and insulation layer. Zhao et al. (2019) gave a distributed temperature monitoring system for tunnels in cold regions and completed a long-term, real-time and multi-channel automatic temperature test in different positions in the tunnel. As can be seen that there are few research on FHF using field measurement method. Currently, the research about the FHF is almost belong to theoretical and experimental work (Tang et al., 2011; Liu et al., 2018; Xia et al., 2018; Huang et al., 2020). Some models considering the influence of phase-change, latent heat, and other factors have been studied. Li et al. (2017) built a coupled heat-water model for the tunnel in cold regions on the basis of energy and mass conservation principles, and gave a scientific balance between the safety and the economy. Meanwhile, in order to study the distribution of FHF, some investigations using lab tests were also conducted. Zeng et al. (2017) performed a similarity simulation model test with 1:30 geometric scale based on practical horseshoe railway tunnel section to investigate the couple effect of convection-conduction. Zhang M et al. (2017) proposed a two-phase closed thermosphon group (TPC), which is applied to adjust and control the ground temperature for a shallow tunnel section.

As seen from above, although a lot of researches about the FHF have been done (Tang et al., 2011; Li et al., 2017; Zeng et al., 2017; Zhang et al., 2017; Liu et al., 2018; Xia et al., 2018; Huang et al., 2020; Kang et al., 2021), few researches about the FHF has been conducted by field measurement method. Up to now, the field measured data about the FHF is very scarce, since we can’t easily distinguish the FHF from the OSRP. In this paper, a new method of measuring the FHF was proposed. The measuring system includes the temperature sensors, the soil pressure box and the data acquisition section. The surrounding rock temperature and the lining pressure were tested synchronously. The FHF was defined as the D-value of the average value of the month in negative temperature and the average value of the month in positive temperature. In the last, the change law of the FHF was analyzed with seasonal time, and the frost heave mechanism was discussed. The results could provide reference for the antifreeze design of tunnels in seasonal cold regions.

FIELD MEASUREMENT PRINCIPLE

The FHF refers to the force exerted on the support system due to frost heave after the frozen-thaw zone of confined surrounding rock is frozen (Bronfenbrener, 2009; Zhang et al., 2004; Lai et al., 2005; Gao et al., 2012; Feng et al., 2017; Li et al., 2017; Zeng et al., 2017; Hu et al., 2018; Liu et al., 2018; Wang et al., 2018; Xia et al., 2018a; Huang et al., 2020; Lyu et al., 2020). Based on the NATM design concept (Zhang et al., 2018), the surrounding rock has self-supporting capacity and will bear part of the FHF, but when frost heave occurs, a large part of the FHF will act on the lining support. Therefore, measuring the change of the stress of the lining support can reflect the change of FHF. However, when the surrounding rock frozen, the FHF and OSRP coupled act on the lining support, so we can’t easily distinguish the FHF from the OSRP, so we can’t obtain the FHF directly by the field testing method. In order to reflect the change law of the FHF, a method of measuring FHF was proposed in this paper. This method is measuring the surrounding rock temperature and the lining pressure at the same time via the monitoring system, which includes the temperature sensors, soil pressure box and the data acquisition section. The FHF could be calculated indirectly from the lining pressure. The principle is as follows: firstly, the variation law of surrounding rock temperature was recorded, meanwhile, the lining pressure was measured synchronously by means of the testing system; secondly, the OSRP was obtained by the lining pressure of unfrozen period, the OSRP + FHF was obtained by the lining pressure of frozen period, the D-value between the lining pressure before and after the surrounding rock frozen is the FHF. We could analyzed the change law of the FHF by the D-value. In addition, in order to make the data more representative, the mean values of lining pressure corresponding to the highest temperature from July to September are taken as the OSRP, and the mean values of lining pressure corresponding to the lowest temperature from December to February are taken as the sum of OSRP and the FHF. The D-value between the two mean values is defined as the generalized FHF. We can obtain the frost heave characteristic of the tunnel by analyze the FHF and the generalized FHF from the recorded lining pressure data.

FIELD MEASUREMENT PROGRAMME

As can be seen from the previous section, the FHF is closely related to the surrounding rock temperature of the tunnel. In order to measure the change law of the FHF, we should measure the surrounding rock temperature and the lining pressure at the same time. In this paper, two typical sections K117 + 700 (hereinafter referred to as section 700) and section K117 + 600 (hereinafter referred to as section 600) were set in the exit zone of Lichoushan tunnel (shown in Figure 2). The monitoring system includes the temperature sensors, the soil pressure box and the data acquisition section. The surrounding rock temperature and the lining pressure of the two sections were measured.
measured by this system respectively. The surrounding rock temperature was measured using temperature sensors. In each section, two hole of 4 m depth was set at side wall and invert part respectively. Every hole includes five measuring points (CW1~CW5 at side wall, DW1~DW5 at invert), where installed the temperature sensors. The interval of each point is 1 m. The lining pressure was measured using the soil pressure box. There are eight points around the lining for measuring lining pressure in each section. The data was recorded four times a day at eight, twelve, sixteen and twenty respectively. In the last, the synchronous change law of the surrounding rock and the lining pressure was obtained, then we can analyse the change law of the FHF by the method proposed above.

The correct installation of sensors is the premise of obtaining accurate data. According to the site installation, we recorded some difficulties of field work. All the sensors should be installed keep up with the progress of tunnel construction (shown in Figure 2). Firstly, to put the temperature sensors on the correct location in the hole, a steel bar of 4 m long was used to fix the temperature sensors, then the steel bar was put into the hole, in which the temperature sensors were protected from damage during installation by geotextile wrapping. After the steel bar with sensor was installed in place, the hole was sealed densely with cement. Secondly, the soil box was fixed on the steel mesh using iron wire binding method, then it was covered with shotcrete. When all the sensors of each section were installed perfectly, the lines should be arranged to the data acquisition system. In the last, each sensor should be debugged before formal data acquisition to ensure its normal operation.

FIELD MEASUREMENT RESULTS

Surrounding Rock Temperature

According to the method proposed above, the surrounding rock temperature of section 700 and section 600 was measured firstly. The results of the two sections were shown in Figure 3A–D. It can be seen that the variation law of surrounding rock temperature in radial 2 m is basically the same as that of tunnel environment temperature. The frozen period is January, February, March, November, December of a year. With the increase of radial depth of surrounding rock, the temperature variation amplitude decreases gradually. When the depth over 3m, the surrounding rock temperature basically unchanged, we can infer that the maximum freezing depth of surrounding rock is between 2 and 3 m. Because of the limitation of temperature sensors layout, the true value of freezing depth could not be measured exactly. The temperature variation of CW1 closing to lining surface is most obvious recently affected by tunnel environmental temperature. In section 700 line CW, the lowest temperature in January is close to -10°C, the highest temperature in August is close to 10°C and higher than the original temperature of surrounding rock, which indicates that the closer to the lining surface, the more obvious the freeze-thaw effect is. The surrounding rock temperature is negative from Nov. to Apr. of the next year, which will lead to a significant frost heave phenomenon in tunnel lining.

From Figures 3A,B, the CW1 in section 700 line CW is -9.6°C in January, 9.9°C in August, the DW1 in section 700 line DW is -8.8°C in January, 10.2°C in August. It can be seen that the invert temperature is higher than the side wall, we could infer that the
The Lining Pressure

Though, we can’t measure the FHF directly, we can measure the lining pressure by the soil box. We can indirectly analyze the change law of the FHF through the change of lining pressure, combing the change of the surrounding rock temperature. According to the measuring plan above, section 700 and section 600 are taken as the typical measuring sections. The change law of the lining pressure in section 700 and section 600 were shown in Figures 4A,B. We can see that the lining pressure changed with seasonal time, the maximum appears in January. Because of the surrounding rock temperature is lower than 0°C in January, February, March, November and December, the lining pressure also change bigger subjected to the freezing phenomenon of surrounding rock. The lining pressure recorded by the data acquisition system contains the OSRP and the FHF. According to the principle of calculation of the FHF, if we want to get the FHF, the OSRP must be excluded. As we know, if the surrounding rock not frozen, the lining support will be acted by the surrounding rock pressure only. So, the OSRP could be get from the data acquisition in July, August and September, when the surrounding rock not frozen.

Analysis and Discussion

The FHF Response to Temperature

To investigate the relationship between the FHF and the surrounding rock temperature, the temperature of CW1 and DW1, the lining pressure of P5 and P7 of section 700 and section 600 were shown in Figure 5A,B. The CW1 and DW1 could represent the temperature change in the side wall and the invert, the P5 and P7 could reflect the FHF change. From Figure 5A, the lowest temperature appeared in January, in which the FHF reached the maximum. Then, with the time go on, the temperature gradually increased and the FHF decreased. The highest temperature appeared in August, in which the FHF changed to the minimum. Further, with time goes on, the temperature become lower and the FHF become bigger. In the whole, the FHF and temperature have opposite periodic variation in 1 year. From Figure 5B, we can see that section 600 has the similar changing rules. The difference is that the peak values of temperature and FHF are smaller than section 700.
The FHF in the Lining
According to the definition, the FHF refer to the force acting on lining structure after the surrounding rock frozen. The lining pressure we got contains the OSRP and FHF. In order to more intuitive display the change of FHF before and after freezing of surrounding rock, we deem that the average value of July, August and September is the OSRP before frozen, and the average value of January, February, December is the sum of OSRP and FHF after frozen. The lining pressure of section 700 and section 600 in each point around the lining before and after frozen were shown in Figures 6A,B. We can see that the lining pressure is small before the surrounding rock frozen, it had a obvious increase in each point after the surrounding rock frozen. The increase before frozen, and the average value of January, February, December is the sum of OSRP and FHF after frozen. The lining pressure of section 700 and section 600 in each point around the lining before and after frozen were shown in Figures 6A,B. We can see that the lining pressure is small before the surrounding rock frozen, it had a obvious increase in each point after the surrounding rock frozen. The increase
in each point is different, the P5 is the minimum in section 700 and section 600. As the surrounding rock temperature, the invert temperature change is small, so the FHF in P5 is small too. But what’s interesting is that the P7 of section 700 and section 600 is small, which remind us that the surrounding rock is not the only factor affecting the FHF, also the water condition, the surrounding rock degree are the influence factors. What’s more, the average of FHF in section 700 is 18.78 kPa, and section 600 is 18.28 kPa, which is in good agreement with the surrounding rock temperature change. This part presented the lessons that we should take more attention to the tunnel portal in the insulation design, and all the factors of temperature, water and surrounding rock degree should be considered.

The FHF Change With Time
To analyse the periodic characteristics of the FHF, the average value of July, August and September was taken as the OSRP, the D-value of the lining pressure and the OSRP in each point is the FHF. The change law of the FHF in P1 to P8 around the lining were shown in Figures 7A,B. As can be seen that the FHF has a period characteristics with time in the sine function type, it become bigger with the temperature lower and reached the maximum in the January. It should be noted that the FHF in May, June, July and August is negative value, which represent no FHF in this period time. We can use a simple sine function to describe the change law of the FHF. In addition, the FHF in P4 and P6 was more obvious than in P1, which could be inferred that more protection is needed at the arch foot on both sides of the tunnel.

Discussion About Frost Heave Progress
From the measurement results above, we can see that the FHF was closely related to the change of surrounding rock temperature. The frost progress in a year was discussed (shown in Figure 8). In January, the surrounding rock temperature is the lowest, the frozen depth in the surrounding rock is biggest at this point, which mean that the frost layer is totally frozen and the FHF reaches the maximum. In March, the surrounding rock temperature became higher, the frozen depth in surrounding rock became smaller, which mean that the frost layer is
partial frozen and the FHF became smaller. In July, the surrounding rock temperature is highest, there is no frost in the surrounding rock, also no FHF exist. We can infer that the FHF changes periodically as a sine function with the time. The FHF would became obvious subjected to the lower temperature in winter, which prompt that some anti-freezing measures must be taken at this point. The change of FHF present the lessons for us to frost resistance design of tunnel in cold regions.

CONCLUSION

In this paper, a simple and effective measurement method of the FHF was proposed, some field work was conducted based on Jichoushan tunnel, and the change law of the FHF was analyzed, the main conclusions are as follows:

1) A method of measuring the FHF was proposed, the measuring system includes the temperature sensors, soil pressure box and the data acquisition section. The calculation method of FHF was presented.
2) The surrounding rock temperature and the lining pressure were measured synchronously. The frost layer is limited to 3 m, the closer to the lining surface, the more obvious the temperature change. The lining pressure, contains the OSRP and the FHF, changed with seasonal time, the maximum appears in January.
3) The average of FHF in section 700 is 18.78 kPa, and section 600 is 18.28 kPa, which is in good agreement with the surrounding rock temperature change. The FHF of each point has a period characteristics with time in the sine function type. More attention should be taken to the tunnel portal in the insulation design, and all the factors of temperature, water and surrounding rock degree should be considered.

DATA AVAILABILITY STATEMENT

The original contributions presented in the study are included in the article/Supplementary Material, further inquiries can be directed to the corresponding author.

AUTHOR CONTRIBUTIONS

YZ: Putting forward the conceptualization and methodology; SF: Writing the original draft manuscript; DY: Conducting the field measuring work; FZ: Writing and reviewing the manuscript.

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Conflict of Interest: Author DY was employed by the company Shaanxi Provincial Land Engineering Construction Group Co., Ltd.

The remaining authors declare that the research was conducted in the absence of any commercial or financial relationships that could be construed as a potential conflict of interest.

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