Bathymetry Study of the Siltation Level of Lugu Dam Reservoir in Sokoto State, Nigeria

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ABSTRACT: This paper seeks to determine the siltation level of the Lugu dam reservoir, Sokoto State, Nigeria, using the bathymetric survey method. A total of eleven (11) ground control points were established over the study area using Hi-Target Global Navigation Satellite (GNS) Real-time Kinematic (RTK) System. The base station was set-up over the reference Bench Mark while the Rover station was moved around to predetermine locations of the ground control points. The depths to the Lugu dam reservoir bed, as well as its underwater topographic mapping with a section of the River Rima on the right flank of the reservoir area, across the collapsed spillway were conducted using Garmin Global Positioning System Map. This was mounted on a nine feet fibre boat to enhance the echo sounding. The result of the study was used to produce a digital elevation model, topographic contours and the area-elevation-capacity curve for the reservoir. This indicates that between elevations 260.5 m and 262 m, the available minimum and maximum designed storage capacities of Lugu dam reservoir ranges from 21.24 MCM and 34.25 MCM respectively. The Lugu dam reservoir maximum storage capacity at breached level stands at 25 MCM, while its active storage capacity is 20 MCM. This is to conclude that the amount of siltation at the reservoir is 9.25 MCM representing 27.01% indicating the difference between the maximum designed capacity and the current storage capacity. It is recommended that dredging be carried out to regain the initial designed storage capacity as this will no doubt put an end to the incessant flooding and erosion experienced in the area.

KEYWORDS: Lugu dam, siltation, topographic mapping, bathymetric survey, reservoir

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I. INTRODUCTION

Reservoirs are the basic source of raw water needed for domestic purposes, industrial, agricultural, fishery and other forms of usage. During the dry season, most rural communities in semi-arid regions, solely depend on these reservoirs as a means of survival economically and socially (Loucks and van 2017; Misra, A. K, 2014). Stored water in these facilities (reservoirs) is utilized by farmers during the dry season so as to ensure continued agricultural activities in the absence of the wet season, in order not result in setback, as well bridge the gap on daily demand by the people during the dry season. The constant utilisation of water from these reservoirs in the absence of the wet season helps to remedy the effect of drought (Hermanace et al., 2015; Liebe et al., 2005). These reservoirs serve as a means of flood control, as they temporarily retain this water during peak flow, thereby reducing excess surface runoff along the streams contributing downstream (Elkhrachy, 2015; De Coning and Poolman, 2011). It is believed that series of small reservoirs have the capacity to impact greatly on the neighbourhood environment, hydrologically and ecologically (Cantonati et al., 2020; Rodrigues and Liebe, 2013). The effect of these small reservoirs may be optimised to improve the negative impact of climate changes (Krol, et al., 2011). Research indicates that 0.5-1% of reservoirs globally lose their installed capacity due to sediment deposition yearly (Rahmani et al., 2018; Basson, 2009). The continual loss of reservoir capacity due to annual gradual siltation may result in loss of all dam and reservoir storages by a quarter of their capacities, if necessary actions are not taken, with a projection of fifty years from the year of its report (WCD, 2000).

Data obtained through bathymetric survey, which involves sounding or impulse transmission into water surfaces of dams and reservoirs to obtain x,y,z coordinates is adequate and remains one of the most reliable components of hydrographic survey and information gathering (Sainson, 2017).

Multiple methods can be used for bathymetric surveys including Multi-Beam and Single-Beam Surveys, Acoustic Doppler Current Profilers (ADCPs), Sub-Bottom Profilers, and the Eco-Mapper Autonomous Underwater Vehicle (EMAUV). Bathymetric surveys allow us to measure the depth of a water body as well as map the underwater features (Menna, et al., 2018).

The emphasis to adopt other advanced methods in the determination of reservoir sediment, such as the application of remote sensing using satellite altimetry may not be applicable independently. This is because other methods other than the bathymetry, do not have the capacity to establish a

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baseline needed to estimate the reservoir catchment areas at different elevations. (Erena, et al., 2020; Dost and Mannaaerts, 2008). With the application of a bathymetric survey on any dam reservoir, the elevation – area – capacity curve is plotted. The curve produced is an essential tool for reading the capacity of the reservoir at any given cross sectional area and elevation. As such, reservoir professional operators use the result of the curve to interpret the reservoir dam characterization, flood routing, sediment loading and dam reservoir classification (Rodrigues & Liebe, 2013).

Reservoirs have been widely used for flood control, meeting irrigation water demand, hydroelectric power generation, tourism and maintenance of ecosystem functions (Ainsworth, 2005). But the deposition of sediment in these reservoirs may compromise their initial designed capacities as well as its intended benefits. Reservoir sedimentation primarily is a continuous process that can be unnoticed for a significant portion of the life of a reservoir as silts and other earth material are transported from the upstream river system to downstream of the reservoir with a relatively high sediment transport velocity. The distance of deposition is dependent on decreasing water velocities. Most often the reservoir capacity tends to shrink or reduce with increased deposition and sediment over time (Radwan, 2016).

The frequent accumulation of silts and sediments may end up producing a normal distribution trend in the reservoir which in most cases affects the reservoir management during peak flood event (Sedlacek, et al., 2016). Fitzpatrick et al., 1987 reported that over a period of 61 years, Lake Decatur lost 9100 acre-feet of storage capacity through increased deposition of sediment to a tonnage of 9,830,000 at the reservoir bed. On average, they estimated each acre of watershed to deliver 21.4 tons of soil material to the lake over the period of study. Chanson and James (2005) had also observed that sedimentation problems were more pronounced in small to medium size reservoirs (catchment area less than 100km2).

Measuring the level of sedimentation and reservoir capacity has been a major challenge in dam management. With the advancement in multi beam technology, echo sounders have become a major instrument in bathymetric surveys (Oke et al., 2019). Acoustic multi-beam systems have proven to be another effective means of hydro survey with series of narrow beam transducers, when mounted on boat is capable of focusing points on or beneath the reservoir water surface (Selva et al., 2013).

Echo sounder bathymetry operates with frequencies between 12 kHz and 500 kHz. Joseph et al., (2017), in the survey of Ruiru reservoir, Kenya, deployed dual echo sounder in their bathymetry, and observed different penetrating frequencies ranging between 200 KHz and 350 KHz. The implication is that the shorter the wavelength of the signal, the higher the spatial and temporal resolution of the measurements (Jakobsson, et al., 2016; Selva et al., 2013). The Lugu dam before the collapse of its spillway in 2012 provided a dependable source of water to the Wurno 12,000ha Irrigation Project. The dam is essentially an earth dam with a capacity of 19 million cubic meters of water (NEWMAP, 2019). However, the dam and its other structures are in a dilapidated state, with the spillways, embankment, slopes in various stages of distress. Due to continuous and incessant flooding, the main spillway collapsed in 2012 and the reservoir was reduced to a small and shallow pond of water. This breach has created some deep erosion in the area resulting in the collapse of the existing service spillway. The study seeks to determine the total volume of silts deposited in the reservoir area following series of flooding and dam breach in the reservoir, using bathymetric survey to generate the topographic contour of the study area as well produce the elevation-area-capacity curve for the dam reservoir which will help monitor the maximum capacity of the Lugu dam reservoir.

II. MATERIALS AND METHODS

A. Study Area

The dam is located to the north of Wurno town situated on latitude 13° 20’N and longitude 4° 55’E, within the Sudan savanna ecological zone on a tributary of the Rima River known as the Balla Creek. It was constructed in the year 1957 by which time the reservoir was being fed by the Balla Creek; extending 6 Km upstream of the Lugu Dam. The flow into the Lugu Dam reservoir at that time was not dependent on the Rima River; but on the small catchment of the Balla Creek. Figure 1 shows the Lugu Dam Reservoir before and after the dam break in 2012. The reservoir is linked to two main canals, namely; Lugu main canal that passes through Lugu village (the site of this study) and Tutudawa main canal that passes through Tutudawa village, a main drain and a number of secondary canals.

B. General Procedure

The study was carried out with the establishment of eleven (11) ground control points (Table 1) around the reservoir area using the existing bench mark as presented in Table 2. The bathymetric survey of the Lugu dam was done using Garmin Global Positioning System-Map-Chart-plotter - acoustic echo sounding device attached to a nine feet (9ft) fibre boat powered by a 40 horse power Yamaha outboard engine. Topographic mappings at various cross sections of the dam reservoir were conducted. The sounding lines were predetermined and generated over the generated image of the reservoir area and pre-loaded into the navigational GPS which were used to guide the boat navigation during the bathymetry survey. The generated coordinates for the entire survey were used to produce the topographic contour map, digital elevation model and the elevation area capacity curve. While the elevation area capacity curve was plotted using Microsoft Excel. Both the topographic contour map and digital elevation model were produced on geographical information system environment.
Table 1: Coordinates of control points with elevations.

| Point Id | Northing  | Easting   | Elevation |
|----------|-----------|-----------|-----------|
| GCP04    | 1470925.967 | 767611.034 | 255.863   |
| GCP04A   | 1470924.098 | 767610.459 | 255.836   |
| GCP01    | 1472072.035 | 767689.021 | 256.363   |
| GCP01A   | 1472072.077 | 767687.017 | 256.287   |
| GCP02    | 1472167.993 | 769184.047 | 257.405   |
| GCP05    | 1471244.977 | 769222.716 | 256.334   |
| GCP05A   | 1471246.501 | 769210.966 | 256.435   |
| GCP03B   | 1472109.935 | 769973.475 | 256.207   |
| NWRI01   | 1471339.292 | 768015.515 | 258.370   |
| NWRI02   | 1472935.958 | 769759.839 | 259.580   |

A. Data acquisition using an echo sounder
After successful setting up of the equipment (Figure 2), the echo sounder was turned on as the boat navigates and its data acquisition software (NAV 380) launched. The coordinate system was set at WGS84 (Ketut et al., 2017). The set of the echo sounder was based on the standard operational mode as used by (Ajith, 2016; Selva et al., 2013) where the recording interval was set to 5 seconds while the HIGH and LOW-frequency signal gate, pulse length, and gear were set to AUTO. Under terms menu, the High Frequency (HF) was set to 200 kHz, Low Frequency (LF) set to 15 kHz and the penetrating range set to 10 meters for optimum penetration. Also file menu for the observation was created and saved in order to commence the logging of data and tab clicking. This was based on the principle adopted by Calder and Mayer, 2003).

Table 2: Characteristics of reference bench mark.

| Word Reference; | Remark |
|-----------------|--------|
| Easting         | 767698.223 E |
| Northing        | 1469882.605 N |
| Elevation       | 258.206 amsl |
| Coordinate System | World Geodetic System- 84 (WGS) |
| Description     | Beginning of the Lugu Main Canal at Lugu Village |

Source: ENPLAN CONSULT GROUP Bench Mark Establishment for Lugu Dam (2019).

The depth of water body was calculated using the principle of acoustic depth measurement as presented in Figure 3 and using Eq. 1 (Parsons, et al., 2013).

\[ D = \frac{c \Delta t}{2} \] (1)

Where D is the acoustic depth in metre (m), c is the speed of sound in water in metre per second (m/s) and t is the time of travel of the acoustic pulse to and from the reservoir bed in second (s). The movement of the canoe parallel to the dam was assumed x-axis and the y-axis was its movement perpendicular to the dam.
III. RESULTS AND DISCUSSION

The bathymetry survey of Lugu dam reservoir area was conducted based on the sounding at different cross-sections and elevation on the water surfaces, sending impulse to the reservoir bed in the form of xyz coordinates. The data obtained was used to produce the digital elevation models as well as the topographic maps where the contour areas were generated. The transducer attached to the boat measured an average depth of 0.54 m based on the pulse travel time and velocity. The depth obtained from the transducer (0.54 m) is an indication of the shallow nature of the reservoir. Processing the Digital Elevation Model (DEM) in ArcGis environment, a bathymetric map of the reservoir and depth-area-volume curve was obtained (Figure 4). The result indicates that the existing embankment crest elevations varies between 261 m to 263 m above mean sea level (amsl) while existing archived satellite imageries downloaded from Global Mapper and United States Geological Survey (USGS) shows that the maximum reservoir shoreline before the dam breach was at elevation of 260 m amsl. Triangular Irregular Network (TIN) generated from the Digital Elevation Model (DEM) was used to produce the surface topography of the reservoir area. The curve shows that between elevations 260.5 m and 262 m, the available minimum and maximum storage capacities of Lugu dam reservoir ranges from 21.24 MCM (Million Cubic Metre) and 34.25 MCM (Million Cubic Metre) respectively. The current active storage capacity (20 MCM) is less than the minimum and maximum designed capacities by 6.2% and 71.25% respectively, indicating silt deposition. This is an indication of silt deposition.

The extent of the bathymetric survey on Lugu dam reservoir is presented in Figure 5. This shows the navigation line of the boat during the bathymetry, while the topography of the Lugu dam reservoir and environ with the surface contour generated as captured by the bathymetry is presented in Figures 6 and 7 respectively.

Digital Elevation Model (DEM) was converted to Triangular Irregular Network (TIN), a vector-based digital geographic data which helped in the presentation of the reservoir surface morphology and in the calculation of planimetric area, surface area and volume. During the bathymetry surveys, depths to reservoir bed were automatically streamed and recorded into the memory of the echo sounder by the transducer installed at a draft distance of 0.1 m below the water surface using the principles of echo sounding as shown in figure 2 above. Horizontal positions of all sounding data were referenced to the World Geodetic System Datum of 1984 (WGS 1984) using the Universal Transverse Mercator (UTM) projection Zone 31N.
IV. CONCLUSION

The bathymetric survey of Lugu dam reservoir shows that active storage capacity of the reservoir is far less than the minimum and the maximum designed capacities by 6.2% and 71.25% respectively and, this is an indication of high silt deposition and loss of storage volume. The Lugu dam reservoir maximum storage capacity at breached level stands at 25 MCM while its active storage capacity is 20 MCM. This is to conclude that the amount of siltation at the reservoir is 9.25 MCM representing 27.01% indicating the difference between the maximum designed capacity and the current storage capacity. It is recommended that rehabilitation of the dam should be focused at the elevations where the maximum and minimum capacities are expected based on the initial design. Also dredging should be carried out to regain the initial designed storage capacity, this will no doubt put an end to the incessant flooding and erosion experienced in the area.

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