Experiments on Turbulence Intensity and Bubble Frequency in Self-Aerated Open Channel Flows

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Abstract: Although spillways have been investigated experimentally by various researchers, only a few studies have been conducted on the comprehensive properties of a self-aerated air-water flow. In this study, new experimental data were recorded and discussed for the distribution of the air concentration, air-water velocity, turbulence intensity and bubble frequency in the completely developed regions for spillways. It was observed that both the turbulence intensity and bubble frequency increased from the bottom and subsequently decreased near the free surface. The positions of maximum air bubble frequency and turbulence intensity gradually approached air concentration to 0.50 in the self-aerated developed region. Self-similar relationships between the turbulence intensity and bubble frequency were proposed.

Keywords: self-aerated flow; air concentration; turbulence intensity; air bubble frequency; experiment data

1. Introduction

Self-aeration is a phenomenon occurring in high-velocity open channel flows, which is attributed to the strong interactions between the turbulent flows and the atmosphere leading to a strong air-water mixing at the interface. Along the chute, the upstream flow is clear until the outer edge of the growing boundary layer reaches the free surface. Air enters into the flow when the turbulence near the free surface is large enough to overcome both the buoyancy effects and surface tension. Downstream of the inception point, the entrained air contributes significantly to an increased air-water interface area and the aeration thickness increases in the streamwise direction. Through the air-water interface, air is continuously trapped and released and the resulting air-water mixture gives a whitish appearance to the entire flow. A drastic change in the air-water flow structure is induced and the properties are directly affected by the turbulence interactions. As the air entrainment develops, the completely developed air-water phase flow becomes a result of the equilibrium between the turbulence intensity and buoyancy. The air-water flow properties, such as air concentration, velocity and flow depth, are independent of the flow direction [1–3]. Extensive experiments have been conducted on self-aerated flows including studies of the velocity distribution, air concentration and bubble frequency and size [4–9]. Chanson [4] focused on the air concentration distribution based on the turbulent diffusion theory for the developed region and also predicted the sizes of the entrained bubbles and provided the relationship between the air concentration and bubble frequency.

The turbulence intensity plays a significant role in an aerated flow. Several researchers [10–13] have investigated the flow structure and turbulence intensity of the unaerated flow in an open channel. Brocchini and Peregrine [14] discussed the free-surface turbulence by considering the stabilizing effect of the gravity and surface tension against the disrupting effect of the turbulent kinetic energy. Daniel et al. [15] indicated that the air-water velocity difference yielded a shear region at the free surface...
that could eventually trigger the aeration and they also proposed a formula for the inception point. However, there is little literature on the turbulence intensity of a self-aerated flow in open channels.

In man-made dam projects, high speed air-water two phase flows can be seen in spillways which may be equipped with various bed roughness ranging from smooth bed to macro-roughness elements such as in stepped spillways [16]. Extensive experimental studies have been conducted in the past decades uncovering key features of the air-water flows and providing design guidelines for smooth and stepped spillways [5,17,18]. The air-water flow properties of stepped spillways, such as air concentration, bubble frequency, bubble size, turbulence intensity, velocity, integral turbulent time and length scales, have been investigated systematically [19,20]. When the outer edge of boundary layer reaches the free-surface and the turbulence forces overcome both surface tension and buoyancy, the air entrainment starts at the inception point of free-surface aeration. Compared with the smooth spillways, the flow pattern of stepped spillways is much more turbulent. Felder and Pfister [16] indicated that, independent of the spillway roughness, the physical processes leading to the air entrainment were similar. Hence, in this study, the provided methods of stepped spillways were applied to investigate the turbulence level in air-water self-aerated flows.

This study aims to investigate the air-water phase properties in the completely developed region of a self-aerated flow using a hydraulic model. An understanding of the air concentration, bubble frequency and turbulence intensity in the developed region was provided. The present study investigated the scale effects concerning physical hydraulic models and identified which air-water flow properties may be affected by the scale effects in the Froude, Reynolds and Weber similitudes.

2. Hydraulics Model and Instrumentation

The experiments were conducted in a rectangular chute with a length of 23.6 m and a width of 0.20 m, which was fabricated using polymethyl methacrylate. The roughness height of the chute was 0.01 mm. Chute bottom angle $\alpha$ was set as 5.71$^\circ$ and 8.22$^\circ$ relative to the horizontal. A water discharge of up to 0.8 m$^3$.s$^{-1}$ was supplied using a constant head system and the accuracy was measured up to $\pm 1\%$ for a rectangular sharp-crested weir. The flow to the chute was fed with a smooth convergent nozzle. Height of nozzle $h_0$ was set as 0.20 m and 0.30 m. The flow velocity was set as $V_0 = 4$–8.2 m/s. Hence, the Froude number $F_0 (V_0/(gh_0)^{0.5})$ is within the range of 3.9 to 5.9 and the Weber number $W (V_0/(\sigma/\rho h_0)^{0.5})$ and Reynolds number $Re (V_0 h_0/\nu)$ are within the ranges of $250 \leq W \leq 490$ and $6.7 \times 10^5 \leq Re \leq 18.1 \times 10^5$, respectively, where $\nu$ was the kinematic viscosity of water, $\sigma$ was the surface tension of water and $\rho$ was the density of water (Table 1). The free surface was kept sufficiently open to the atmosphere. In this instance, $x$ was the streamwise coordinate along the chute bottom and $z$ was the perpendicular coordinate (Figure 1).

![Figure 1. Sketch of self-aeration in the open channel flow.](image-url)
Table 1. Experimental flow conditions.

| Run  | \(q_w\) (m²/s) | \(h_0\) (m) | \(\alpha\) (deg.) | \(V_0\) (m/s) | \(Re \times 10^5\) | \(F\) | \(W\) |
|------|----------------|-------------|-------------------|--------------|-----------------|------|------|
| T1–1 | 1.69           | 6.46        | 12.8              | 3.9          | 384             |
| T1–2 | 1.81           | 6.78        | 13.9              | 4.0          | 407             |
| T1–3 | 1.92           | 6.89        | 14.8              | 4.1          | 423             |
| T1–4 | 2.01           | 7.09        | 15.5              | 4.2          | 439             |
| T1–5 | 2.11 0.30      | 7.30        | 16.2              | 4.3          | 456             |
| T1–6 | 2.13           | 7.36        | 16.4              | 4.4          | 460             |
| T1–7 | 2.23           | 7.43        | 17.2              | 4.4          | 473             |
| T1–8 | 2.29           | 7.51        | 17.6              | 4.5          | 482             |
| T1–9 | 2.35           | 7.67        | 18.1              | 4.6          | 494             |
| T2–1 | 0.87           | 5.50        | 6.7               | 4.6          | 254             |
| T2–2 | 0.94           | 5.95        | 7.2               | 4.8          | 275             |
| T2–3 | 1.18           | 6.44        | 9.1               | 4.9          | 320             |
| T2–4 | 1.25           | 6.72        | 9.6               | 5.0          | 337             |
| T2–5 | 1.3 0.20 5.71  | 6.91        | 10.0              | 5.1          | 348             |
| T2–6 | 1.39           | 7.10        | 10.7              | 5.2          | 365             |
| T2–7 | 1.43           | 7.33        | 11.0              | 5.3          | 376             |
| T2–8 | 1.51           | 7.50        | 11.6              | 5.4          | 391             |
| T2–9 | 1.59           | 7.86        | 12.2              | 5.6          | 411             |
| T3–1 | 1.58 0.20 8.22 | 7.96        | 12.3              | 5.7          | 412             |
| T3–2 | 1.62           | 8.20        | 12.6              | 5.9          | 424             |

Notes: Reynolds number Re and Weber number W were defined as: \(Re = \frac{U \times h}{v}\), \(W = \left(\frac{\rho_w U^2 \times h}{\sigma}\right)^{0.5}\). The central stream-wise air-water velocity \(V\), air concentration \(C\), bubble frequency \(f\) and turbulence intensity \(Tu\) were measured with a phase-detection needle probe (CQY-Z8a, Measurement Instrument, Institute of water resources and hydropower, Bengbu, China) [21,22]. The needle probe consisted of two identical tips with inner diameter 0.25 mm (Figure 2). The working principles of the conductivity probes were based on the difference in the voltage indices between the air and water phases at the platinum tip between the air and water phases [17,23]. If the measurement is completely surrounded by air phase or water phase, the flow characteristics, such as air concentration, bubble frequency and turbulence intensity can be obtained. The turbulence intensity in the flow direction only can be observed in the aerated region. The response time of this sensor was less than 10 μs. The signals from the conductivity probe were recorded at a scan rate of 100 kHz per for a 40 s scan period. The accuracy of the air concentration was \(\Delta C/C = 3\%\) and the bubble frequency was \(\Delta f/f = 1\%\). The probe was supported by a trolley system and the accuracy in the longitudinal of the probe position was less than 0.5 cm. The elevation of the probe from normal to the chute bottom was supervised with a Vernier caliper less than 0.5 mm.

Figure 2. Sketch needle probe.
Chanson [3] and Kipphan [24] indicated that the broadening of the cross-correlation function compared with the auto-correlation function of the leading sensor may provide information regarding the turbulence intensities in an air-water flow (Figure 3). Felder and Chanson [20] proposed the following equation for the turbulence intensity:

\[ Tu \approx 0.851 \times \sqrt{\frac{\tau_{0.5}^2 - t_{0.5}^2}{T}} \]  

where \( Tu \) was the turbulence intensity \((Tu = u/V)\) in the flow direction, \( u \) was the velocity fluctuation in the flow direction, \( V \) was the mainstream velocity, \( \tau_{0.5} \) was the time scale at which the cross–correlation function was half of its maximum value \((R_{xy}(T + \tau_{0.5}) = 0.5R_{xy}(T))\). \( t_{0.5} \) was the characteristic time at which the normalized auto-correlation function was \( R_{xx}(t_{0.5}) = 0.5 \) [17]. In this paper, we only can obtain the distribution of turbulence intensity \( Tu \) in the flow direction, rather than turbulence intensity in the vertical direction.

Scale effects are usually present in the physical modeling of air-water flows because the Froude number, Weber number and Reynolds number cannot be identical to those of the prototype [25–30]. To minimize the scale effects of the bubble frequency and turbulence intensity, all the experimental tests were within the ranges of \( 250 \leq W \leq 490 \) and \( 6.7 \times 10^5 \leq Re \leq 18.1 \times 10^6 \), which were much wider than those in other investigations (Table 2).

| Reference | Slope (deg.) | \( U \) (m/s) | \( F \) | \( Re \) \((\times 10^5)\) | \( W \) | Comments |
|-----------|--------------|--------------|--------|-----------------|--------|----------|
| Cain [1]   | 45           | 17.7         | 15.2   | 2.2             | 730    | Prototype (Aviemore dam) |
| STRAUB and LAMB [5] | 7.5–75     | 4–0          | 5.0–11.5 | 4.7–20       | 100–50 | Model |
| Chanson [31] | 4           | 3.2–5.3     | 5.5–9.9 | 1.1–1.3        | 81–109 | Model |
| Deng [32]  | 13.5         | 3.5–6.35     | 4.6–8.4 | 2.1–3.8        | 97–178 | Model |

### 3. Air-Water Flow Properties in Developed Region
#### 3.1. Distributions of Air Concentration and Velocity

The typical distributions of the air concentration and velocity along the chute are presented in Figure 4. The previous studies of Wood [7] and Chanson [4] used iso-concentration line \( C = 0.90 \) as the characteristic aerated flow depth (represented as \( h_{90} \)) for the completely developed region of the self-aerated flow. The dimensionless coordinate perpendicular to the chute bottom was defined as \( z/h_{90} \).
Definition:

\[ h = \int_{z=0}^{z=h_{90}} (1 - C) \, dz \]  

(2)

\[ U = \frac{q_w}{h} \]  

(3)

\[ C_{\text{mean}} = \frac{1}{h_{90}} \int_{z=0}^{z=h_{90}} C \, dz \]  

(4)

where \( h_{90} \) was the depth where \( C = 0.90 \), \( h \) was the clear water depth, \( U \) was the mean velocity of cross section, \( q_w \) was the water discharge per width and \( C_{\text{mean}} \) was the average concentration of the cross-section.

Figure 4. Distributions of air concentration (a) and velocity distribution (b) along the chute. Run: T1–8.

Note the following: (1) upstream of section \( x = 2 \) m, air is negligibly entrained into the flow through the upper surface and the section consists almost entirely of unaerated black water. (2) Although the air concentration shapes were almost similar, the characteristics of the aerated flow were found to vary at different locations of \( x \leq 12 \) m along the chute. For \( h_0 = 0.20 \) m, the inception point \( x/h_0 \) approached 9.0, boundary of developing/developed regions \( x/h_0 \) approaches 45. For \( h_0 = 0.30 \) m, the inception point \( x/h_0 \) approaches 8.3, boundary of developing/developed regions \( x/h_0 \) approaches 40. (3) Figure 5 shows that the mean air concentration increases slightly, with \( C_{\text{mean}} = 0.01 \) at \( x/h_0 = 6.7 \) increasing to \( C_{\text{mean}} = 0.051 \) at \( x/h_0 = 40 \). (4) A good agreement was also observed in terms of the dimensionless distribution of the velocity, whereas the velocity profiles followed closely a 1/6th power law [1,31]. (5) Figure 5 shows that dimensionless velocity \( U/V_0 \) and depth-averaged air concentration \( C_{\text{mean}} \) seem to reach some asymptotic values.
In addition, various studies have investigated the air concentration in a self-aerated flow and found it to agree with the characteristic two-phase flow profiles. Chanson [33] proposed the theoretical equation for the air concentration distribution based on the turbulent diffusion theory. The velocity distribution closely followed a 1/6th power law [1,31]. Cain [1], who proposed $n = 6.3$ for prototype investigations and Chanson [31] for model investigations, with $n = 6.0$.

$$C(x, z) = 1 - \tanh^2 \left( K' \frac{z'}{D'} \right)$$  \hspace{1cm} (5)

$$\frac{V}{V_{90}} = \left( \frac{z}{z_{90}} \right)^{1/6}$$  \hspace{1cm} (6)

where $D'$ and $K'$ were dimensionless coefficients, $z' = z/h_{90}$ and tanh() was a hyperbolic tangent function.

### 3.2. Turbulence Intensity and Bubble Frequency

The dimensionless distributions of bubble frequency $f_{h}/U$ and turbulence intensity $Tu$ are presented in Figure 6. Note the following: (1) the distributions of both the turbulence intensity and bubble frequency increased in the developing region and maintain equilibrium in the developed region. In the developed region, the shapes of the air bubble frequency and turbulence intensity were invariant. (2) The distribution of turbulence intensity $Tu$ was similar to the air bubble frequency distribution. Dimensionless air bubble frequency $f_{h}/U$ increased from the bottom and then decreased near the surface. Similarly, the turbulence intensity decreased slightly near the surface. (3) In the self-aerated developed region, the typical distributions of the bubble frequency and turbulence intensity showed that the maxima of $f_{\text{max}}$ and $Tu_{\text{max}}$ were observed in the intermediate flow region when $0.4 < C < 0.6$ (Figure 6b). Finally, the distribution of $Tu$ only consisted of the aerated zone (without the unaerated black water).

![Figure 5. Mean of concentration and velocity along the chute (Run: T1–8).](image)

![Figure 6. Cont.](image)
Figure 6. Distributions of air bubble frequency (a) and turbulence intensity (b). Run: T1–8.

The profiles of the air bubble frequency and turbulence intensity exhibit maximum values ($f_{\text{max}}$ and $Tb_{\text{max}}$) at each cross-section are presented in Figure 7. Note the following: (1) with the development of self-aeration along the stream-wise, the position of the maximum air-phase frequency ($z_{f_{\text{max}}}$) shifted to the air-water flow, as shown in Figure 7a; (2) For $0 < x/h_0 < 40$, the maximum bubble frequency was observed at the location where $0.50 < C < 0.80$, whereas at $40 < x/h_0 < 100$, the maximum bubble frequency was concentrated at $C = 0.50$. Observation showed that the maximum bubble frequency was at the location where $C = 0.50$ at a cross-section in the self-aerated developed region. (3) Compared with the maximum bubble frequency, the maximum turbulence intensity displayed a similar low variation (Figure 7b). Consequently, at the air-water cross-section along the chute, the bubble frequency increased with the increasing intense turbulent fluctuation.

Figure 7. Variation of maximum bubble frequency and turbulence intensity positions along the developing and developed regions.
In this paper, we only can obtain the distribution of turbulence intensity $T_u$ in the flow direction, rather than turbulence intensity in the vertical direction. There is no any data of turbulence intensity in the vertical direction in air-water two phase flows, such as hydraulic jumps, stepped flows and jet flows, et al. Many researchers have tried to explain the mechanism by which bubbles entrain into the flow through the free surface in an open channel. Up to now, despite there is no uniform theory, they all indicate the turbulence velocity $v$ in the vertical direction is the most significant factor. However, basing on current research methods, the turbulence velocity $v$ is hard to obtain in air-water two phase flows. The distribution of $T_u$ in the flow direction may be helpful to study the dynamics of bubbles and of the free surface.

3.3. Froude, Reynolds and Weber Similitudes

The self-aerated air-water flow properties are presented in this section with the objective of focusing on the comparative analysis of the developed regions summarized in Table 1. Figure 8 shows the air concentration and velocity distributions at an identical Froude number $F$. The comparative results displayed that the distributions of the air concentration and velocity were more appropriately scaled with the Froude similitude.

![Figure 8](image_url)

**Figure 8.** Distributions of air concentration (a) and velocity (b) with identical $F$ ($F = 4.6$).

Figure 9 shows the distributions of the turbulence intensity and bubble frequency for identical Froude number $F$, Reynolds number $Re$ and Weber number $W$. The comparative results indicate that the turbulence intensities and bubble frequency could not be scaled with the $F$, $Re$, or $W$ similitudes.
4. Discussion

Self-similarity, which was closely linked with dynamic similarity, was a powerful tool in the research on turbulent flow and high-velocity air-water flows [34]. In the developed region of the air-water cross-section, the relationship between the air bubble frequency and air concentration was self-similar. The position of the maximum air bubble frequency caused by the air-water discontinuity gradually approached $C = 0.40–0.60$. The results of Cain [1] indicated that maximum bubble frequency $f_{\text{max}}$ corresponded to $C = 0.50$ for self-aerated flows. The relationship between air concentration and bubble frequency were best fitted by a parabolic function (Figure 10).

$$\frac{f}{f_{\text{max}}} = 4C(1 - C)$$ (7)
The air bubble frequency represented the number of air-to-water and water-to-air interfaces within the air-water activity levels of the flows. Chanson [35] suggested that the turbulence controlled the mechanism of the bubble break-up and air-water interfacial properties, such as the air bubble frequency. The interaction between the entrained bubbles and turbulence structures also increased the turbulence levels [36]. In the self-aerated developed region, the typical distributions of the bubble frequency and turbulence intensity showed that the locations of \( f_{\text{max}} \) and \( T_{\text{u max}} \) were within the intermediate flow region for \( 0.4 < C < 0.6 \).

Toombes and Chanson [28] studied the two-phase high-velocity air-water flow properties on stepped spillways and proposed a function for turbulence intensity \( T_u \) and dimensionless bubble count rate \( fh/U \):

\[
T_u = a + b \times \left( \frac{f R}{U} \right)^m
\]  

(8)

where \( a, b \) and \( m \) were constants. Felder and Pfister [16] indicated that independent of the spillway roughness, the physical processes leading to the air entrainment were similar. Observations also indicated that the shapes of air concentration, velocity, bubble frequency and turbulence intensity were similar with stepped flows [16,28]. In order to quantify the turbulence intensity in self-aerated open channel flows with smooth bed, the provided methods of stepped spillways were applied to investigate the turbulence level. Hence, in the self-aerated developed region, the data overlapped reasonably well with the exponential trend (Figure 11):

\[
T_u = 0.033 + 0.12 \times \left( \frac{fh}{U} \right)^{0.25}
\]  

(9)

Equation (9) reflected a monotonic increase in \( T_u \) with increasing bubble frequency. In Equation (9), the limit for \( fh/U = 0 \) was \( T_u = 0.033 \).

5. Conclusions

This study focused on high-velocity open channel flows. An accurate measurement technique utilizing conductivity probes provided a comprehensive analysis of the air-water flow properties in the developing and developed regions for the self-aerated flow, including the air-water flow parameters, turbulence intensities and bubble frequencies. In the air-water cross-section, values of both the turbulent intensity and bubble frequency increased from the bottom and subsequently decreased near the surface. The maxima of both the air bubble frequency and turbulence intensity were generally observed in the high-air concentration range in the self-aerated developing region. These maxima gradually approached the position of \( C = 0.50 \) in the self-aerated developed region.

Furthermore, the present investigation identified that the microscopic air-water flow properties of a self-aerated flow including the turbulence intensities and bubble frequencies could not be accurately scaled.
The present results were limited to self-aerated flows with $5.71^\circ \leq \alpha \leq 8.22^\circ$, $4.0 \leq F \leq 6.1$, $6.7 \times 10^5 \leq \text{Re} \leq 18.1 \times 10^5$ and $254 \leq W \leq 494$. This study complements the microcosmic characteristics of self-aerated flows in open channels with a smooth bed, which will be helpful for studying the dynamics of the bubbles and the free surface. However, conducting field measurements in a prototype facility remains a challenge.

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