Investigation of Multiple Hole Grouting for Reducing Water Inrush Risk in Underground Engineering

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Received 1 November 2021; Accepted 25 November 2021; Published 11 December 2021

Academic Editor: Bingxiang Yuan

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Reinforcement with grout as an environmentally friendly technology has played a key role in underground coal mining. The risk of the water inrush into coal mines can all be reduced by grouting. A model that integrates a geographic information system (GIS), distribution of quantity of grouting injected, and water inflow correlation method is proposed here to evaluate the effects of grouted reinforcement in coal mining. The temporal and spatial characteristics of the volume of grout injected and water yield of aquifers are analyzed by using the GIS, and the rate of filling of cement slurry and its distribution characteristics are determined. The effects of grouting on the aquifers which has been carried out to reduce their permeability are determined by comparing the spatial temporal variations in the volume of the grout injected, water yield, and rate of filling of the cement slurry. The method was applied in a case study in a coalmine in Henan province, China, in which the risk of the water inrush from karst aquifers has been reduced by grouting. There are three limestone aquifers, namely, L8, L10, and L11, which underlie an exploitable coal seam. The result indicates that most of the cement slurry is consumed when the water yield is 20 to 30 m³/h; and that there are minimal changes of the electrical properties of the rock stratum under coal seam when the water yield of L11 is low within the range of 40 m. The resistivity of the aquifers before and after grouting and their spatial characteristics are tested by using the transient electromagnetic method (TEM), and this shows that there are no areas with low resistivity. The electrical properties of the strata at a depth that ranges from 40–80 m with transverse homogeneity show that Aquifers L10 and L11 have been transformed into aquicludes. The reinforcement effect of aquifers with grout is good.

1. Introduction

There are three main components in mining engineering: characterizing the rock mass structure, applying engineering geology, and understanding the process of data transformation in engineering geology [1, 2]. The mechanical properties and integrity of the structure of the rock mass can be improved by using grouting as reinforcement, in which the overall structure of the rock can be preserved, and fractures in the rock mass can be sealed. When grouted, the weathering of rock mass is prevented. In recent years, as much as 25 billion tons of coal reserves in Chinese coal mines have been threatened by flooding, more than 50% are mainly found in the coalfields in North China that date back to the Permo-Carboniferous period [3]. Cambrian or Ordovician limestone is usually found at the bottom of the Permo-Carboniferous strata in the coalmines [4–6]. With increased depth in mining, the coal mine panels face the risk of water inrush from the mine floor [7–9]. Two methods are used when mining is carried out over karst aquifers under high pressure, which are to facilitate drainage to reduce the water pressure and apply grout [10–12]. However, the cost of draining the coal seam increases with increased depth, and the groundwater will be affected. The application of grout to stop water inrush from the coal seam floor and the reconstruction of aquifers are effective methods for preventing and controlling water inrush into the coal seam from the karst aquifers when mining over confined aquifers [13]. The
techniques for reducing the permeability of aquifers and aquifer reconstruction have gradually matured and were widely used in projects that controlled water inrush into coal mines in the 1990s in China [14, 15]. However, the most direct way to reduce incidents of water inrush is to apply grout, which will seal the water flow channels of fractures, faults, and karsts to reduce permeability or create an aquiclude [16–18]. The effects of grouting are very important. An analysis method with the use of waveform graphs based on radar sensors was proposed to examine the effects of grouting. The pre- and post-plate tectonic eras were detected by using the direct current method, and the effects of grouting were analyzed by comparing the resistance value [19–21]. The transient electromagnetic method (TEM) can be used to determine the position of water on the floor of the work face and found that the permeability is significantly decreased after grouting the rock on the mine floor [22, 23].

Based on the literature, there are 14 different systems to determine the effects of grouting, in which 5 are analyzed in this study, which are the analytical, inspection, excavation sampling, displacement, and geophysical methods as listed in Table 1 [24].

However, the previous research work has been mainly based on one type of method or only use one method, so that the evaluation results are subjective, which means that the accuracy of the results is greatly reduced. There are also no standardized methods for evaluating the quality of the effects of grouting. In fact, the spatial characteristics of the grouting may not be evident. Therefore, the objective of this paper is to analyze the spatial and temporal effects of the distribution of the quantity of grouting injected in accordance with a spatial analysis approach which utilizes a GIS. Moreover, the TEM is also used to determine the electrical resistivity of the rock stratum under coal seam in the process of grouting.

2. Method

2.1. Reinforcement of Coal Seam Floor with Grout. Reinforcement with grout is a method for modifying the hydrogeological properties of the rock mass and has been often used in mining. Under a certain grouting pressure, the grout slurry consolidates or forms gel in the voids or channels in rocks which were originally occupied by water [25–29]. Consequently, water inrush from the rock floor is inhibited, and the strength of the aquifer and the performance of the water barrier layer are improved. The aquifer is transformed into an aquiclude, and the water inflow into the panel will be significantly reduced. Reinforcement of the coal seam floor with grout is carried out first by drilling a borehole, and then the panel with the borehole is connected to the layer in which grouting will be applied. When the aquifer on the coal seam floor has lots of water and the water pressure is high or the bottom of the coal seam floor is not thick, the floor has a broken zone in the water control layer. The transformation of the aquifer into an aquiclude by using grouting as a means of inhibiting water seepage means that the thickness of the water resistance layer will be increased and water inrush coefficient is reduced. Reinforcement with grout is used in this study on the upper and lower roadways in the panel, where geophysical exploration or drilling or other activities are carried out. By using engineering design parameters of the reinforcement, the measures for reinforcement with grout are determined for the transformation of the aquifer into an aquiclude which results in its relative impermeability or further improves water resistance (Figure 1).

2.2. Spatial Analysis. Spatial analysis is the implementation of techniques that examine the topography, geometry, or geographic properties of different studied entities, involves tools that manipulate and change spatial information, and then extracts the trends from the data [30, 31]. Spatial analysis is carried out with GIS. The spatial analysis functions of GIS (especially the extraction and transmission of implicit information) mainly differentiate GIS and common information systems [32]. Spatial analysis with the use of GIS allows the location and space of geographic objects to be determined through feature extraction and classification of spatial information and image segmentation and acquires spatial distribution information, spatial forms, and spatial changes [33].

Based on the reasonable integration of the collected parameters in the grouting construction, qualitative and quantitative evaluations of the effects of grouting are carried out through analysis and comparison methods, combined with the use of the spatial analysis function in GIS. A spatial analysis with the use of GIS to evaluate the effects of grouting is a novel method of examining underground structures. Two methods are used to characterize the spatial distribution of the water yield and volume of grout injected in a GIS environment which are examining the properties of the distribution of grout and comparing the water yield. In order to analyze the trend of variations in the water yield and volume of grout used, the water yield of aquifers and volume of grout injected into them were first normalized in the GIS. As well, the effects of grouting are evaluated.

| Table 1: Classification of evaluation methods of grouting effect. |
|---------------------------------------------------------------|
| **Method**         | **Remarks**                          |
|-------------------|--------------------------------------|
| Analytical        | Distribution of quantity of grouting injected |
|                   | Water inflow correlation              |
|                   | Rate of filling with grout            |
| Inspection of borehole | Observations                     |
|                   | Coring                               |
|                   | Permeability coefficient test         |
| Excavation sampling | Grouting mechanism analysis         |
|                   | Mechanical test                       |
| Displacement estimation | Water level predictions          |
|                   | Deformation predictions               |
| Geophysical prospecting | Gravitational prospecting            |
|                   | Magnetic prospecting                  |
|                   | Electrical prospecting                |
|                   | Seismic prospecting                   |
|                   | Radioactivity prospecting             |
|                   | Geothermal prospecting                |
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3. Case Study

3.1. Geological Background of Coalmine. The Chensilou coalmine is located in the Yongcheng city in Henan province, China (116.4 E, 33.9 N). The mean annual temperature is 14.3°C. The average annual rainfall between 1963 and 2017 is 850.65 mm, with the highest recorded rainfall of 1,518.6 mm in 1963 and the lowest of 543.7 mm in 2011. More than 50 percent of the annual precipitation is in the months of July and August. A single horizontal coal seam in the south and north zones was exploited by longwall caving with a longwall retreat system.

Three karst aquifers, namely, L₈, L₁₀, and L₁₁, underlie Coal Seam II. The average width of the strata between the surface of Aquifers L₈, L₁₀, and L₁₁ and the bottom of Coal Seam II is 75.8 m, 58.6 m, and 43.6 m, respectively. L₁₀ and L₈ recharge L₁₁ through the rock crevices. The average thickness of Aquifers L₈, L₁₀, and L₁₁, under the floor of Coal Seam II is 6.5 MPa. The water inrush coefficient is calculated by using the following equation:

\[ T_s = \frac{P}{M} \]  

(1)

where \( T \) is the water inrush coefficient, \( M \) denotes the aquiclue thickness, and \( P \) denotes the water pressure of the confined aquifer, 0.15 MPa/m, which is higher than 0.1 MPa/m. In accordance with the regulations of preventing water hazards for coalmines, reinforcement of these three aquifers with grout is necessary to increase their thickness and transform them into aquicludes. The middle and upper aquifers and the coal seam are hydraulically connected in the middle and upper parts of the Taiyuan formation.

3.2. Grouting and Transient Electromagnetic Method. The grout material is cement slurry with a density of 1.25–1.35 t/m³. A total of 248 boreholes were drilled into Panel 2517 for grouting. The total water yield and volume of the grout used in Aquifers L₈, L₁₀, and L₁₁ were determined after drilling. The accumulative water yield of the aquifers is 10,887 m³/h, and the cumulative volume of grout injected is 19,476.4 tons.

The maximum water yield of a single borehole in Aquifers L₈, L₁₀, and L₁₁ is 80 m³/h, respectively. The volume of grout used in Aquifers L₈, L₁₀, and L₁₁ is 728 t, 357 t, and 122 t, respectively.

A reduplication device was used for the transient electromagnetic process, and a multiturn 2 m × 2 m rectangular return line was used to make the transmitting and receiving wire frames. The sample window is 1–34, and the number of superimpositions is 64, with the use of a standard time series. A transient electromagnetic survey system (terraTEM) was used. A Pentium III processor, which contains a low energy consumption Pentium series processor, was used with the transient electromagnetic survey system. The field data were processed by using visual software. The display is a 15-inch ultrathin LCD screen with touch function which can greatly improve the efficiency of inputting collected field data with its graphical user interface. The global positioning system (GPS) function was used because it is very convenient for mapping the results. The console was sealed in accordance with the IP66 protocol and can operate in a wet environment. There are 106 measured points in the upper cross sections and 111 measured points in the lower cross sections.

4. Results

4.1. Spatial Distribution Characteristics of Water Yield and Volume of Grout. The spatial distribution characteristics of the water yield of Aquifers L₈, L₁₀, and L₁₁ and volume of grout injected into them are shown in Figures 2 and 3. The total water yield from the 191 water exit points of L₈ is 5134 m³/h, which accounts for 47.2% of the total water yield. The potential of water from the aquifer is 55.04%, which shows that the aquifer has lots of water which is evenly distributed. The total water yield from all 133 water exit points of L₁₀ is 4966 m³/h, which accounts for 45.66% of the total water yield. The potential of water from the aquifer is 85.3%, which shows that the aquifer has lots of water which is evenly distributed. There are 23 water exit points in L₁₁, which are mainly distributed on the outside of the panel, and the water yield from the water exit points is 13.6%, which does not correspond to the abundance of water in L₁₁. L₈ and L₁₀ recharge L₁₁, as they contain lots of water and have developed fissures so that the water seeps through the cracks. The water yield and volume of the grout injected for aquifers are listed in Table 2.

The water yield and volume of grout injected into the three aquifers show the characteristics of the fissures filled with water: the proportion of the water yield and volume of grout injected into L₁₁ aquifer is the same, which indicates that the amount of grout used adequately seals the gushing water fissures. There are six types of fissures that fill with water in accordance with the water yield, in which Types I to VI have a water yield of <10 m³/h, 10–20 m³/h, 20–30 m³/h, 30–40 m³/h, 40–50 m³/h, and >50 m³/h, respectively (see Table 3).

Table 4 shows that the average volume of grout injected into Type I fissures is 15.9 t but they are only found in L₈. In comparison, the volume of grout injected into Type II fissures through a single hole reaches 500 t in L₈ (see Table 5).
Table 2: Proportion of water yield and volume of grout injected.

| Aquifer | Water exit points | Water yield (m³/h) | Volume of grout injected (tons) |
|---------|-------------------|--------------------|-------------------------------|
|         | No. | Proportion (%) | Water yield | Proportion (%) | Single hole | Volume of grout | Proportion (%) | Single hole |
| L₈      | 191 | 55.04        | 5134        | 47.20            | 26.9        | 8816           | 45.26          | 114.5       |
| L₁₀     | 133 | 38.33        | 4966        | 45.66            | 37.3        | 9229           | 47.38          | 124.7       |
| L₁₁     | 23  | 6.63         | 777         | 7.14             | 33.8        | 1432           | 7.35           | 79.6        |
| Total   | 347 | 100.00       | 10877       | 100.00           | 31.3        | 19476.4        | 100.00         | 115.2       |

Table 3: Types of fissures filled with water.

| Type of fissure | I | II | III | IV | V | VI |
|----------------|---|----|-----|----|---|----|
| Water yield    | <10 m³/h | 10–20 m³/h | 20–30 m³/h | 30–40 m³/h | 40–50 m³/h | >50 m³/h |

Table 6 shows that the average volume of grout injected into Type III fissures is 239.6 t, and they are mostly found in L₁₀. The volume of grout injected into Type IV fissures through a single hole reaches 191 t in L₁₀ as listed in Table 7.

Table 8 shows that the average volume of grout injected into Type V fissures is 258 t, and they are mostly found in L₈. The volume of grout injected into Type VI fissures through a single hole reaches 728 t in L₁₀ as listed in Table 9.

5. Discussion

Coal strata are generally found with different layers of rock, with relatively uniform electrical resistivity distributions. The TEM is used to measure the resistivity of coal and other important physical parameters of the properties of the surrounding rock and changes in the resistivity of the surrounding rock and coal in accordance with their
composition, structure, saturation, and other factors, in which substantial changes are found. In fact, the conductivity of coal bearing strata directly affects its resistivity. When a column collapses or fault zone is filled with water, then the conductivity of the water will show low local resistance in terms of its apparent resistivity [34]. When the volume of water in a developed structure is low or there is no water, the conductivity is poor and the local resistivity is high. The differences in the conductivity of rock are based on their geophysical characteristics; however, the variations in the relationship between the vertical and horizontal resistivities in mines can be examined by using TEM.

The TEM was developed in the mid-1980s and modified since then [35]. More recently, a new electromagnetic method was devised by David Wright and his colleagues, Anton Ziolkowski, and Bruce Hobbs, at the University of Edinburgh in 2001, called the multitransient electromagnetic method (MTEM) [36]. The MTEM involves the use of two electrodes in which a current is introduced into the ground between them. In mining, the MTEM examines the changes in the magnetic field by using an ungrounded current loop placed between the underground roadway and the surrounding space of the roadway. When excitation takes place, the induced eddy current created by the conductivity of the

| Table 4: Type 1 fracture. |
|--------------------------|
| Aquifer | Water exit points | Total volume of injected grout (t) | Volume of injected grout into one borehole (t) |
|---------|-------------------|-----------------------------------|----------------------------------|
| L8      | 10                | 127                               | 15.9                             |
| L10     | 0                 | 0                                 | 0.0                              |
| L11     | 0                 | 0                                 | 0.0                              |
| Total   | 10                | 127                               | 15.9                             |

| Table 5: Type II fracture. |
|---------------------------|
| Aquifer | Water exit points | Total volume of injected grout (t) | Volume of injected grout into one borehole (t) |
|---------|-------------------|-----------------------------------|----------------------------------|
| L8      | 38                | 1428                              | 42.0                             |
| L10     | 10                | 448                               | 44.8                             |
| L11     | 2                 | 106                               | 53                               |
| Total   | 50                | 1981.8                            | 139.8                            |

| Table 6: Type III fracture. |
|----------------------------|
| Aquifer | Water exit points | Total volume of injected grout (t) | Volume of injected grout into one borehole (t) |
|---------|-------------------|-----------------------------------|----------------------------------|
| L8      | 80                | 2921.9                            | 48.7                             |
| L10     | 50                | 3235.0                            | 95.1                             |
| L11     | 9                 | 766.0                             | 95.8                             |
| Total   | 139               | 6922.9                            | 239.6                            |

| Table 7: Type IV fracture. |
|----------------------------|
| Aquifer | Water exit points | Total volume of injected grout (t) | Volume of injected grout into one borehole (t) |
|---------|-------------------|-----------------------------------|----------------------------------|
| L8      | 32                | 1196.7                            | 57.0                             |
| L10     | 23                | 1529.0                            | 66.5                             |
| L11     | 5                 | 171.0                             | 34.2                             |
| Total   | 60                | 2896.7                            | 157.7                            |

| Table 8: Type V fissure. |
|--------------------------|
| Aquifer | Water exit points | Total volume of injected grout (t) | Volume of injected grout into one borehole (t) |
|---------|-------------------|-----------------------------------|----------------------------------|
| L8      | 6                 | 435.0                             | 72.5                             |
| L10     | 14                | 526.0                             | 43.8                             |
| L11     | 2                 | 58.0                              | 29                               |
| Total   | 22                | 1019                              | 145.3                            |

| Table 9: Type VI fissure. |
|--------------------------|
| Aquifer | Water exit points | Total volume of injected grout (t) | Volume of injected grout into one borehole (t) |
|---------|-------------------|-----------------------------------|----------------------------------|
| L8      | 26                | 2708.0                            | 117.7                            |
| L10     | 35                | 3490.0                            | 105.8                            |
| L11     | 5                 | 331.0                             | 66.2                             |
| Total   | 66                | 6529                              | 289.7                            |
**Figure 4:** Schematic of transient electromagnetic process in detection of electromagnetic fields.

**Figure 5:** Continued.
Figure 5: Apparent resistivity of aquifers before grouting.

Figure 6: Continued.
rock ore body around the roadway will generate two electromagnetic fields over time. Since the two fields contain geoelectric data of the measurements made around the tunnel, determining the electrical properties of the conductive media around the roadway is done by observing the two electromagnetic fields during the interval and extracting and analyzing the observed information. A schematic of the transient electromagnetic process in the detection of electromagnetic fields is shown in Figure 4.

Figure 5 shows the spatial distribution characteristics of the apparent resistivity of the different sections of study area by using the TEM before grouting.

Most of drilled holes do not have any water in the areas with relatively normal resistivity, and most of water exit points are found in the anomaly areas with low resistivity. Figure 6 shows the spatial distribution characteristics of the apparent resistivity of different sections of study area after grouting and minimal changes of the electrical properties of the rock stratum under coal seam when the water yield of L_{11} is low within the range of 40 m, and this shows that there are no areas with low resistivity. The spatial distribution of apparent resistivity in the strata in the depth that ranges from 40 to 80 m shows that L_{10} and L_{11} have been transformed into aquicludes, and there is no obvious water bearing geological anomalies or concealed water cut channels within the depth range. This shows that grouting is effective and reduces the permeability of the floor so that L_{10} and L_{11} become aquicludes.

6. Conclusions

Grouting reinforcement has played a key role in reducing the permeability of the aquifers. In order to put into place a safe plan that mining above on aquifers, the effects of grouting reinforcement assessment are essential. This paper proposes a new method that couples GIS and the TEM to evaluate the effects of grouting reinforcement of aquifers and analyzes the spatial characteristics of the volume of the grout injected into aquifers and their water yield.
The new method was applied in a case study in a coalmine which indicates that the method provides objective and accurate evaluation of the reinforcement with grout. The resistivity of the aquifers in this study before and after grouting and their spatial characteristics are examined, and there are six types of fissures that fill with water in accordance with the water yield. The electrical properties of the strata show that the aquifers have been transformed into aqicludes, and the effect of grouting reinforcement of aquifers is good. The new method can be used to evaluate the effects of grouting reinforcement in other underground engineering construction projects, such as tunnels and dam, in future work.

Data Availability
The datasets generated during the current study are available from the corresponding author on reasonable request.

Conflicts of Interest
The authors declare that there are no conflicts of interest in the submission of this manuscript.

Authors’ Contributions
All authors approved the manuscript for publication.

Acknowledgments
The authors would like to acknowledge financial support from the National Natural Science Foundation of China under Grant no. 52174128, Scientific Research Program of Colleges, Universities in Xinjiang Uygur Autonomous Region of China under Grant no. XJEDU2016S089, and Henan College Students’ Innovation and Entrepreneurship Training Program under Grant no. S202110480034.

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