1. Introduction

Acoustic emission (AE) method is a quite unique non-invasive and passive non-destructive technique which identifies defects only while these defects develop during the monitoring. The evaluation of acoustic emission events may be continuous (during the experiment) or can be made subsequently after the experiment [1, 2].

Every fracture in a specimen takes place with the release of stored strain energy. This energy is emitting elastic (AE) waves which are detected by sensors. After the detection of AE signals, as dynamic motions at the surface of a material, these signals are converted into the electric signal that is further amplified (in cement-based materials the signals are usually amplified from 60 dB to 100dB in total to be detected) and filtered [3, 4].

Because it is not always possible to place the acoustic emission sensors directly on the measured objects (due to reasons such as extreme surface temperature of the specimen that would damage the sensor, possible inaccessibility of the monitored part in relation to its construction, or unstable surface – fresh mixture), acoustic waveguides are used. Their great disadvantage is the loss of the resulting signal size on the interface and signal attenuation in the material [5].

The AE method is commonly used during mechanical loading of structures, however, it has been shown that this method may also be well-suited for the investigation of other processes such as setting, hardening, and curing taking place in cement-based composites. These early processes are important and significantly influence the future properties of the material [6].

The setting process can be characterized as a state in which the slurry loses its workability and gradually acquires strength. This process is followed by the
hardening process, when the newly formed solid material gradually acquires higher strength [7].

Depending on the specific degree of the reaction of the Portland cement with water, it is possible to divide hydration of cement into five stages: I. mixing with water, II. induction period, III. start of setting, IV. hardening, V. hydration deceleration [8].

When mixing cement with water, ions are released into the initial solution. It is a chemical reaction with a massive release of the heat of reaction. The cement grains begin to be covered with the C-S-H gel formed from the silicate phases of clinker and ettringite.

The next stage, called the induction period, follows, in which the release of the heat slows down, the pH value increases rapidly and the concentration of Ca$^{2+}$ ions in the mixing water decreases (together with the hydrates formed on the surface of the particles) the solubility of the Portland clinker phases. The beginning of setting of the cement paste can occur in case the system is saturated with calcium ions, when the system is unstable. The Ca$^{2+}$ ions therefore reach the saturation degree and the formation of ettringite continues. The cause of the start and end of the induction period is the subject of many discussions [7–9].

During the beginning of setting, a gradual decrease of the concentration of Ca$^{2+}$ and OH- ions in the solution occurs, all the Portland cement phases begin to dissolve and heat is released again. This hydration stage is also accelerated by the presence of C3S. C-S-H is formed, which leads to the increase of the strength of the given system because the grains get closer together and join. In addition to the formation of C-S-H, CH phases are also formed and fill the space between the cement grains. The formation of ettringite continues [8].

In the next stage (hardening), most cements do not contain a sufficient amount of calcium sulphate to react with all aluminate phases of the Portland clinker. After the formation of ettringite depletes the SO$_4^{2-}$ ions, ettringite becomes unstable and recrystallizes under the formation of monosulfate, and the hydration is slowed down in the last stage [7, 8].

### 2. Composition of the composite

Two fine-grained cement composites, high-strength mortar without microsilica (composite A) and high strength mortar with microsilica (composite B) were produced for the purpose of the experiment. The composite A is a transition from conventional mortars to mortars with ultra-high strengths (Reactive Powder Concrete) based on fine-grained components. The key difference from concrete is that the mortar contains only grains up to 4 mm. This means that there should be far less intensive cracking of the shrinking hardening cement paste around large grains, as is the case in concrete.

#### Table 1. Components for 1 m$^3$ of the mixture

| Component      | Composite A | Composite B |
|----------------|-------------|-------------|
| Weight [kg]    |             |             |
| CEM I 42.5 R   | 1000        | 740         |
| Stach. 2280    | 15          | 15          |
| Water          | 280         | 260         |
| Hulín 0/4      | 940         | 1245        |
| Fibres 6 mm    | 10.5        | 10.5        |
| Microsilica    | –           | 60          |

### 3. Description of the experiment

Two additional holders were attached to the steel form to fix the acoustic emission sensor-waveguides (Fig. 2). Acoustic emission measurements were performed using the DADEL XEDO equipment.
4. Results

When monitoring the acoustic activity of the material during setting and the early phase of hardening, the composite A exhibited the largest increase in the number of acoustic emission events at the beginning of the measurement. This is approximately 8 hours after mixing the mixture. From 28 hours after the start of the measurement, the acoustic emission events are minimal. The composite B has two sharp increases in the number of the acoustic emission events, the first being from the beginning of the measurement to 3 hours and the second one between the 5th and 6th hour of the measurement.

A comparison of both samples shows that while the composite A has a slightly faster onset of the acoustic emission events, it is the mixture B which has slightly more acoustic events overall in the 48-hour measurement period. However, both cases are probably within measurement errors.

The development of the internal temperature in the composite A in regard to the ambient temperature was initially rapid, a gradual decrease followed and stopped at the temperature of 30 °C. Subsequently, an-
other considerably slower increase began, which later stabilized (after 30 hours from the start of measurement) at 34 °C, which was 4 °C above the ambient temperature.

The development of the internal temperature of the composite B was very gradual. After the first peak, a slight decrease occurred and stopped at the temperature of 36 °C, which was 8 °C above the ambient temperature.

The amplitude of the AE signal is a useful parameter because it indicates the size of the detectable acoustic emission event (the detection depends on the amplitude that exceeds the threshold value). The amplitude of the AE signal is usually given in decibels and the monitoring interval is usually between 0 and 100 dB, then we can distinguish for example events with amplitudes:

- $<35$ dB – small
- $35$–$55$ dB – medium
- $55$–$75$ dB – large
- $>75$ dB – very large
Fig. 6. Comparison of the dependence of the number of events on the amplitude (composite A – left, composite B – right)

Fig. 7. Comparison of the dependence of the number of events on the amplitude (interval 4–8 hours), (composite A – left, composite B – right)

Fig. 8. Comparison of the dependence of the number of events on the amplitude (interval 8–12 hours), (composite A – left, composite B – right)
The composite A exhibits an even distribution of the dependence of the number of events on the amplitude. Medium amplitudes are predominant with an almost negligible amount of very large amplitudes. The composite B predominantly exhibits medium amplitudes and that only in small numbers. Large amplitudes are also present, but only in negligible amounts.

The following graphs show that in the first part from the beginning of the measurement and therefore also setting, the composite A exhibits more events with higher amplitude. It can therefore be assumed that larger microcracks are being formed in it.

The greater amount of the cement paste in the mortar without microsilica can be responsible for the higher number of higher amplitudes: 1000 kg of cement + 280 kg of water ≈ 603 l, while in the other case it is: 740 kg + 60 kg microsilica + 260 kg water ≈ 526 l.

5. Conclusion

The described acoustic emission method is one of the non-destructive test methods that, unlike most other methods, only capture active events in the structure of the material or construction. The evaluation of the experiment indicated that the acoustic emission method belongs among prospective methods that can be used both in the laboratory and in practical situations for long term monitoring of the condition of constructions, as a suitable supplement to commonly used methods.

The conducted experiments indicate that the higher amplitude values of the acoustic emission signals correspond to more significant structural changes that occur in the material, whether they be new products or the formation or growth of microcracks. It can also be stated that the addition of microsilica does not have a significant influence on the amount of the AE events when compared to composites without microsilica.

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