Review Article

Tian Su, Chenxia Wang, Fubo Cao, Zhenghao Zou, Chunguang Wang, Jun Wang, and Haihe Yi*

An overview of bond behavior of recycled coarse aggregate concrete with steel bar

https://doi.org/10.1515/rams-2021-0018
Received Dec 01, 2020; accepted Dec 30, 2020

Abstract: In this paper, an overview focusing on the bond behavior of recycled coarse aggregate concrete (RAC) with steel bar is carried out. The results show that the failure modes of RAC specimen were not only influenced by a single factor, it is necessary to comprehensively consider the influence of various factors on the failure modes of RAC specimen; The steel bar diameter, the concrete cover to diameter ratio and the replacement rate of RCA all affect the load-slip curve of RAC specimen; Bond strength predictive equations and bond-slip relationship models of RAC with steel bar has been summarized to ensure engineers to better understand their applicability; The influencing factors of the bond behavior of RAC specimens including the replacement rate of RCA, the concrete compressive strength, the steel bar type and steel bar diameter, the steel bar embedment length and the aggregate performance; The freeze-thaw cycles and the steel bar corrosion all affect the bond behavior of RAC with steel bar. When freeze-thaw damage and steel bars corrosion reach a certain degree, the bond behavior of RAC with steel bar deteriorates.

Keywords: recycled coarse aggregate concrete, bond strength, prediction model, freeze-thaw cycles, steel bar corrosion

1 Introduction

In recent years, the problem of environmental pollution has attracted more and more attention [1]. A large amount of construction and demolition waste (CDW) will be produced during the demolition of the old building, of which the demolished concrete waste accounts for a large proportion, and it will cause serious environmental pollution [2]. In addition, the new buildings will consume a lot of natural resources. The recycling and reusing of the CDW has become a focus of research work in recent years.

Scholars also try to apply new technologies to concrete [3]. Recycling CDW through the development and application of RAC is a new way [4]. The RAC refers to a new type of concrete that uses coarse aggregates and fine aggregates extracted from CDW instead of natural aggregates [5, 6]. The technology of RAC can solve the problem of environmental pollution caused by CDW [7, 8]. At the same time, it can reduce the consumption of natural aggregates [9]. Therefore, the technology of RAC is beneficial from the perspective of resources saving and environment protection [10]. More and more scholars began to investigate RAC [11].

The bond behavior between concrete and steel bar is a key factor to ensure the two materials work together. Unlike NCA, RCA has more porous and higher water absorption, which is because the old cement paste on its surface will influence the bond behavior between concrete and steel bar [12–15]. Some studies on the bond behavior between RAC and steel bar have been conducted through beam-type test or pull-out test. It can be seen that many factors could influence the bond behavior between RAC and steel bar, such as replacement rate of RCA, concrete compressive strength, steel bar diameter, steel bar embedment length, etc [16–18]. In addition, the bond strength prediction equations and the bond-slip prediction models of RAC and with bar were proposed by considering different influencing factors [16]. However, there was no universal bond strength prediction equations and bond-slip prediction models between RAC and steel bar. Therefore, it was necessary to summarize the existing bond strength prediction equations and bond-slip relationship prediction models between RAC
and steel bar to ensure engineers to better understand their applicability.

Moreover, laboratory tests also revealed that the harsh environments (such as freeze-thaw cycles and steel bar corrosion) could accelerate the degradation of bond behavior of RAC with steel bar, which made the research more complicated [19]. Therefore, it was necessary to further research on the bond behavior of RAC with steel bar for the application of RAC in reinforced concrete (RC) structures.

In this paper, the failure modes, the bond-slip curves, the bond strength prediction equations, the bond-slip relationship prediction models and the critical influencing factors of the bond behavior of RAC with steel bar were reviewed. Furthermore, the bond behavior of RAC specimens after freeze-thaw cycles and steel bar corrosion were presented, respectively.

2 Failure modes and bond-slip curves of RAC with steel bar

2.1 Failure modes

The failure modes of RAC with steel bar are shown in Figure 1, it can be seen that the failure modes of RAC specimens were similar to that of NAC specimens. The main failure modes of RAC with steel bar are as follows:

1) Pullout failure. The relative slip occurred between the RAC and steel bar, and the steel bar was slowly pulled out from RAC. There was no crack on the surface of RAC specimen, which was shear failure.

2) Splitting failure. The RAC specimens were suddenly split into two or three parts, which was brittle failure.

3) Pullout-splitting (P-S) failure. The splitting cracks occurred on the surface of RAC specimen when the steel bar was pulled out from RAC.

4) Rebar yielding (RY) failure. The steel bar occurred yielding failure during the loading process.

The author compared the bond failure modes of NAC specimens and RAC specimens through pullout test [19], and the results are shown in Figure 2. The bond failure modes of RAC specimens and NAC specimens were the pullout-splitting failure, but the cracks of NAC specimens were larger than those of RAC specimens, the reason is that the fracture energy of NAC is greater than that of RAC.

![Figure 1: Failure modes of RAC specimens: (a) Pullout failure; (b) Splitting failure; (c) Pullout-splitting failure; (d) Rebar yielding failure.](image)

![Figure 2: Failure modes of NAC and RAC specimens.](image)
Table 1: Factors for determining the failure modes

| w/c  | Concrete strength (MPa) | Replacement rate of RCA (%) | Steel bar diameter (mm) | Steel bar type | Cover depth (mm) | Stirrups | Embedment length (mm) | Failure modes |
|------|-------------------------|-----------------------------|-------------------------|----------------|-----------------|----------|-----------------------|--------------|
| 0.51 | 29.26 26.52 28.53 2708 | 0 30 60 100                | 16                      | Deformed       | 67              | -        | 64                    | Pullout       |
| Kim et al. [18] | 33.42 31.46 30.66 29.49 | 0 30 60 100               | 16                      | Deformed       | 67              | -        | 64                    | P-S          |
|       | 44.13 39.50 43.80 42.44 | 0 30 60 100               | 16                      | Deformed       | 67              | -        | 64                    | Splitting     |
|       | 48.5 49.3 47.9 43.4 | 0 20 50 100               | 12                      | Deformed       | 94              | -        | 5d/10d/15d            | Splitting     |
| Guerra et al. [20] | 48.5 49.3 47.9 43.4 | 0 20 50 100               | 16                      | Deformed       | 92              | -        | 5d/10d/15d            | Splitting     |
| 0.52 | 43.1 41.2 45.2 43.2 | 0 50 14 100               | 14                      | Deformed       | 68              | -        | 75                    | P-S          |
| Jun [23] | 45.2 45.2 42.8 | 100 16                    | 14                      | Deformed       | 68              | -        | 75                    | Splitting     |
|                | 0.43 | 42.8 | 100 | 20 | Deformed | 65 | - | 100 | 125 | Splitting |
|----------------|------|------|-----|----|----------|----|---|-----|-----|-----------|
|                | 0.43 | 42.8 | 100 | 20 | Plain    | 65 | - | 100 | 125 | Pullout   |
| Zhao and Wang [24] | 0.51 | 30   | 0/30/50/100 | 20 | Deformed | Plain | 75 | - | 3d/5d/7d | Pullout |
| Ren [26]       | 0.35 | 40   | 100 | 16 | Deformed | 67 | - | 5d/8d/10d | Pullout |
| Butler et al. [28] | 0.60 | 30   | 0   | 25.2 | Deformed | 30 | ✓ | 125 | 375 | P-S |
| Wang et al. [29] | 0.50 | 55.2 | 30  | 18  | Deformed | 91 | - | 180 | 360/450 | P-S |
|                | 0.50 | 49.2 | 50  | 18  | Deformed | 91 | - | 180 | 360/450 | P-S |
| Wardeh et al. [30] | 0.67 | 25-30 | 0   | 10/12 | Deformed | 50/49 | - | 5d  | Pullout |
|                | 0.59 | 35-40 | 0   | 10/12 | Deformed | 50/49 | - | 5d  | Pullout |
|                | 0.55 | 35-40 | 100 | 10/12 | Deformed | 50/49 | - | 10d | RY |
|                | 0.55 | 35-40 | 100 | 10/12 | Deformed | 50/49 | - | 10d | RY |
Several investigations have been performed on the influence of various factors on the failure modes of RAC with steel bar. Table 1 shows the results of Literature [18, 20, 23, 24, 26, 28–30]. The test results of Kim et al. [18] showed that the water-to-cement ratio (w/c) was the critical influencing factor on the failure mode of RAC specimens, the failure mode was pullout failure when the water-to-cement ratio was 0.51; the failure mode was pullout-splitting failure when the water-to-cement ratio was 0.46; the failure mode was splitting failure when the water-to-cement ratio was 0.33. The reason is that the fracture energy of higher strength concrete is greater than that of lower strength concrete. A noticeable point is that when the replacement rate of RCA was 100%, the failure modes of the specimens with 0.46 water-to-cement ratio was pullout failure. Therefore, it can be speculated that the replacement rate of RCA was another factor affecting the failure mode of RAC specimens. The test results of Guerra et al. [20] showed that the steel bar diameter was the critical influencing factor on the failure mode of RAC specimen. Guerra et al. [20] point out that the RAC specimens with smaller steel bar diameter (12 mm) exhibited more radial cracks and lower crack widths, while the RAC specimens with larger steel bar diameter (12 mm) exhibited fewer radial cracks and wider crack widths. Jun [23] investigated the influence of the steel bar type, the steel bar diameter, the replacement rate of RCA, the concrete cover depth and the embedment length on the failure mode of RAC specimens, and found that the steel bar type was the critical influencing factor on the failure mode of RAC specimen, the failure mode of RAC specimens with deformed steel bar was splitting failure or pullout-splitting failure, while that of RAC specimens with plain steel bar was pullout failure. This agrees with the findings of Zhao and Wang [24] and Li et al. [25]. The test results of Ren [26] showed that the concrete strength, the embedment length and the stirrup could affect the failure mode of RAC specimen, and the stirrup was the critical influencing factor. The result was similar to that reported by Xu et al. [27]. Butler et al. [28] discovered that when the concrete cover depth was small, the splitting cracks occurred on the surface of RAC specimen when the steel bar was pulled out regardless of concrete compressive strength or embedment length. The test results of Wang et al. [29] showed that the embedment length is the critical influencing factor on the failure mode of RAC specimen, the failure mode of RAC with larger embedment length was rebar yielding failure, while that with smaller embedment length was splitting failure. The test results of Wardeh et al. [30] discovered a similar conclusion that the failure mode of RAC specimens with smaller embedment length (equal to 5 times the steel bar diameter) was pullout failure regardless of the steel bar diameter, the concrete strength and the cover depth, while that with larger embedment length (equal to 10 times the steel bar diameter) was rebar yielding failure.

Actually, the failure modes of RAC specimen were not only influenced by a single factor [31]. Therefore, it is necessary to comprehensively consider the influence of various factors on the failure modes of RAC specimen, such as the water-to-cement ratio, the concrete strength, replacement rate of RCA, the steel bar diameter, the steel bar type, the concrete cover depth, the stirrups, the embedment length. In addition, the water-to-cement ratio and the concrete strength can be regarded as the same influencing factor; Because the steel bar diameter can affect the concrete cover depth, the ratio between the concrete cover depth and the steel bar diameter (c/d) can be used as an influencing factor.

2.2 Bond-slip curves

Previous research has shown that the development and deterioration of bond behavior of RAC specimens was fundamentally similar to that of NAC specimens, and there was no noticeable difference in the load-slip curves between RAC and NAC specimens [42–46].

Generally, the load-slip curves of RAC specimens with pullout failure or pullout-splitting failure include the micro-slip branch, the internal cracking branch, the pullout branch, the descending branch and the residual branch. In the micro-slip branch, the load-slip curve was almost linear, the load was small and the slip at the steel bar free end was not obvious; In the internal cracking branch, the slip at the steel bar free end began to appear. In this branch, the adhesion force had nearly been exhausted; In the pullout branch, the slip increased rapidly, the load-slip curve became non-linear, and then the pullout load reached the peak value. In this branch, the splitting cracks occurred in the concrete cover in some cases; In the descending branch, the pullout load decreased rapidly and the slip increased rapidly; In the residual branch, the pullout load was almost constant when the slip increased to a certain value. However, the bond-slip curves of RAC specimens with splitting failure or rebar yielding failure was incomplete with only ascending branch.

However, Kim and Yun [36] reported that the ascending branch of the load-slip curve of RAC specimens was more non-linear than that of NAC specimens. The result was similar to that reported by Xiao and Falkner [34]. Several investigations have been performed on the influence of various factors on the load-slip curves of RAC with steel bar. Prince [32, 33, 35] point out that the ascending branch of the load-slip curves of RAC with larger steel bar diameter
almost linear up to peak load, the internal cracking branch and the pullout branch were indistinguishable. Pour and Alam [37] reported that the pullout branch became distinguishable when the concrete cover to diameter ratio was larger, and the slope of the descending branch was relatively small, which was due to the higher toughness. Li et al. [25] investigated the influence of the replacement rate of RCA on the load-slip curves of RAC, and found that there was not obvious relationship between the slope of the ascending and descending branch and the replacement rate of RCA.

3 Bond strength predictive equations and bond-slip relationship models

Currently, several bond strength predictive equations and bond-slip relationship models of RAC with steel bar have been established according to experimental data, as described in the following sections.

3.1 Bond strength predictive equations

Kim et al. [18] proposed a multivariable model as shown in Equation (3) taking into account the average density of the coarse aggregates and the concrete compressive strength.

\[
\tau = 1.039 \times \left(0.925 \times \frac{\rho_{\text{AVE}}}{\rho_{\text{NCA}}} \right) \left(\frac{\rho_{\text{AVE}}}{\rho_{\text{NCA}}}\right)^{1/2} \]  

(1)

\[
\tau = \frac{\tau_{\text{RAC}}}{\tau_{\text{NAC}}} \]

\[
F_c = \begin{cases} 
30, & f_c \leq 30 \\
 f_c, & \text{other case}
\end{cases}
\]

Where \(\rho_{\text{AVE}}\) is the average density of the coarse aggregates; \(\rho_{\text{NCA}}\) is the density of the NCA; \(f_c\) is the concrete compressive strength; \(\tau_{\text{RAC}}\) is the bond strength of RAC specimens; \(\tau_{\text{NAC}}\) is the bond strength of NAC specimens.

Wardeh et al. [30] proposed a new model taking into account the concrete compressive strength, the steel bar diameter, the concrete cover, the embedded length and the replacement rate of RCA:

\[
\tau_{\text{max}} = \frac{-0.33 + 0.4 \left(\frac{\gamma}{925}\right) + 9.5 \left(\frac{\gamma}{100}\right) \sqrt{f_{\text{cm}}}}{1 + 0.125\gamma} 
\]

(2)

Where \(\gamma\) is the replacement rate of RCA; \(l_d\) is the embedded length; \(d\) is the steel bar diameter; \(f_{\text{cm}}\) is the cylinder compressive strength;

Seara-Paz et al. [38] proposed a modified expression based on the prediction equation in Model Code-2010 [39]:

\[
\tau_{\text{max}} = 2.5 \sqrt{f_{\text{cm}}(1 - 0.124\gamma/100)} 
\]

(3)

Kim et al. [40] proposed a predictive equation taking into account the concrete compressive strength, the replacement rate of RAC and recycled fine aggregates (RFA), the steel bar diameter and the concrete cover depth.

\[
\tau_{\text{max}} = 0.614 \sqrt{f_{\text{c,k}}(c/d - 0.55)} - \left(0.4203e^{0.0172S} + 0.007\gamma\right) 
\]

(4)

Where \(f_{\text{c,k}}\) is the mean value of cylinder compressive strength; \(c\) is the concrete cover depth; \(S\) is the replacement rate of RFA.

A comparison among these bond strength predictive equations is summarized and listed in Table 2, to better understand the factors that affect bond strength considered in the predictive equations, and their applicability.

It is clear that the concrete compressive strength is taken into account for all these predictive equations. Average density of coarse aggregates is considered only in the bond strength predictive equations of Kim et al. [18]; Replacement rate of RFA is considered only in the bond strength predictive equations of Kim et al. [40]; Embedment length is considered only in the bond strength predictive

Table 2: Factors for determining bond strength predictive equations

| Predictive equations | Average density of coarse aggregates | Concrete compressive strength | Steel bar diameter | Replacement rate of RCA | Cover depth | Replacement rate of RFA | Embedment length |
|----------------------|--------------------------------------|-----------------------------|-------------------|------------------------|------------|------------------------|-----------------|
| Kim et al. [18]      | ✓                                    | ✓                           | ×                 | ×                      | ×          | ×                      | ✓               |
| Wardeh et al. [30]   | ×                                    | ✓                           | ✓                 | ✓                      | ✓          | ×                      | ✓               |
| Seara-Paz et al. [38] | ×                                    | ✓                           | ×                 | ✓                      | ×          | ✓                      | ✓               |
| Kim et al. [40]      | ×                                    | ✓                           | ✓                 | ✓                      | ✓          | ✓                      | ×               |
equations of Wardeh et al. [30]. Replacement rate of RCA is a critical influencing factor of bond strength, but which is ignored in the bond strength predictive equations of Kim et al. [18]; Steel bar diameter and concrete cover depth are ignored in the bond strength predictive equations of Kim et al. [18] and Seara-Paz et al. [38]. It is worth noting that the stirrup is not considered in those bond strength predictive equations, and it is necessary to be further investigated.

Comparison between predicted data determined using Eq. (1-4) and experimental values of [19] is shown in Figure 3. The difference between the experimental value and the predicted value determined using Eq. (1-3) was not great, while the predictive equation of Kim et al. greatly underestimated the bond strength of RAC specimens. The reason is that the predictive equation of Kim et al. [40] is suitable for RAC specimens containing RCA and RFA, while the specimens of [19] do not contain RFA.

![Figure 3: Comparison between predicted values and experimental values of bond strength.](image)

**3.2 Bond-slip relationship models**

**3.2.1 Xiao’s model**

Xiao [34, 41] proposed a normalized bond-slip relationship model of RAC with steel bar, which was obtained by modifying the equations proposed by Harajli [42] and Guo [43].

\[
\tilde{\tau} = \begin{cases} 
\bar{s}^a, & \bar{s} \leq 1 \\
\frac{b}{3(s-1)^2} + 1, & \bar{s} > 1
\end{cases}
\]

\[
\tilde{\tau} = \frac{\tau}{\tau_{\text{max}}}
\]

\[
\tilde{s} = \frac{s}{s_{\text{max}}}
\]

Where \(\tau_{\text{max}}\) is the ultimate bond strength; \(s_{\text{max}}\) is the slip-page corresponding to the ultimate bond strength; \(a\) and \(b\) are constants which determined base on the experimental results.

Many scholars established the corresponding bond-slip models based on the model proposed by Xiao and Falkner [34] and Xiao et al. [41] by considering the influence of various factors on the parameter \(a\) and \(b\). Li et al. [25] found that the parameter \(a\) of RAC with plain steel bar was 0.15, while that with deformed steel bar was 0.1. Wardeh et al. [30] investigated the influence of the steel bar diameter on the values of parameter \(b\), and found that the values of parameter \(b\) decreased with the increase of steel bar diameter. Xiao and Falkner [34] point out that the parameter \(a\) was determined to 0.3, whatever the replacement rate of RCA or mix proportion, and the values of parameter \(b\) of RAC with deformed steel bar were much higher than that with plain steel bar. Prince and Singh [32] investigated the bond-slip model of high strength RAC with steel bar, and found that the values of parameter \(a\) and \(b\) were 0.18 and 0.20 respectively when the steel bar diameter was 8 mm, while the values of parameter \(a\) and \(b\) were 0.20 and 0.15 respectively when the steel bar diameter was 10 mm. Prince and Singh [33] studied the bond-slip model of RAC with different steel bar diameter, the results showed that the values of parameter \(a\) were in a narrow range, regardless of the replacement rate of RCA. This agrees with the findings of Kim et al. [44]. Prince and Singh [33] also pointed out that there was no obvious relationship between the values of parameter \(b\) and the replacement rate of RCA. However, Xiu [45] revealed different conclusion that the parameter \(b\) decreased with the increase of replacement rate of RCA, and it increased with the increase of concrete compressive strength. Prince and Singh [49] reported that the values of parameter \(b\) of RAC with 10 mm steel bar increased with the increase of the replacement rate of RCA, while this trend was not clearly when the steel bar diameter was 8 mm.

According to the bond-slip relationship model, the smaller the value of parameter \(a\), the steeper the ascending branch of the curve; and the smaller the value of parameter \(b\), the smoother the descending branch of the curve. A comparison among these bond strength predictive equations is summarized and listed in Table 3. It was clear that key factors such as, the steel bar type, the concrete compressive strength, the steel bar diameter and the replacement rate of RCA have been considered in previous studies. Actually, the factors affecting the bond-slip curve will also affect the values of parameter \(a\) and \(b\). However, the concrete cover depth, embedded length and the stirrup are ignored, and it is necessary to be further investigated.
Table 3: Factors for determining parameter $a$ and $b$

| Predictive equations | Parameter | Steel bar type | Concrete compressive strength | Steel bar diameter | Replacement rate of RCA |
|----------------------|-----------|----------------|------------------------------|--------------------|-------------------------|
| Kim et al. [18]      | $a$       | $✓$            | -                            | -                  | -                       |
|                      | $b$       | $✓$            | -                            | -                  | -                       |
| Wardeh et al. [30]   | $a$       | -              | -                            | -                  | $✓$                     |
|                      | $b$       | -              | -                            | $✓$                | -                       |
| Prince and Singh [32]| $a$       | -              | $✓$                          | -                  | -                       |
| Prince and Singh [33]| $a$       | -              | $✓$                          | -                  | $✓$                     |
| Xiao and Falkner [34]| $a$       | -              | $✓$                          | -                  | -                       |
|                      | $b$       | -              | $✓$                          | -                  | -                       |
| Xiu [45]             | $a$       | -              | $✓$                          | -                  | -                       |
|                      | $b$       | -              | $✓$                          | -                  | $✓$                     |
| Prince and Singh [49]| $a$       | -              | $✓$                          | -                  | -                       |
|                      | $b$       | -              | $✓$                          | -                  | $✓$                     |

3.2.2 Prince’s model

Prince and Singh [35] proposed a new bond-slip relationship model between RAC and steel bar with only parameter $a$, which based on the model proposed by Xiao [34] and Xiao et al. [41]:

$$
\tau = \begin{cases} 
(3)^{a}, & 0 \leq S \leq 1 \\
\frac{1}{2} + 0.15 \tau_{\text{max}}, & S \geq 1
\end{cases}
$$ (8)

Prince and Singh [35] reported that the value of parameter $a$ was about 0.2 for the normal-strength (36 MPa) and high-strength (68 MPa) RAC specimens, while it was about 0.3 for the medium-strength concrete (51 MPa).

3.2.3 Three-stage model

Cao et al. [50] proposed a three-stage relationship model between RAC and steel bar:

$$
\tau = \begin{cases} 
\tau_{\text{max}} \left( \frac{S}{S_{\text{max}}} \right)^{a}, & 0 \leq S \leq S_{\text{max}} \\
\tau_{\text{max}} - \frac{\tau_{\text{max}} - \tau_{\text{c}}}{S_{\text{max}}} (S - S_{\text{max}}), & S_{\text{max}} \leq S \leq S_{1} \\
\tau_{\text{c}}, & S > S_{1}
\end{cases}
$$ (9)

Where $\tau_{c}$ is the residual bond strength; $S_{1}$ is the slippage corresponding to the residual bond strength; $a$ is parameter that can be determined base on the experimental results.

Wang [19] proposed another three-stage relationship model between RAC and steel bar:

$$
\tau = \begin{cases} 
(0.8 + 0.16a^{-1}) \frac{S}{S_{\text{max}}} \tau_{\text{max}}, & 0 \leq \frac{S}{\tau_{\text{max}}} \leq 0.8 \\
(-a \frac{S}{S_{\text{max}}} + a + 1) \tau_{\text{max}}, & 0.8 \leq \frac{S}{\tau_{\text{max}}} \leq 1
\end{cases}
$$ (10)

Where $a$ and $b$ are parameters that can be determined base on the experimental results.

The bond-slip relationship model proposed by Prince and Singh [35] and the three-stage relationship model were not widely applied. Although the bond-slip relationship model proposed by Xiao [34] and Xiao et al. [41] was widely applied, there was still no uniform regulation for the selection of the values of parameter $a$ and $b$, and the influence of some factors, such as the concrete cover depth, the embedded length and the stirrup on parameter $a$ and $b$ has not been investigated, and it is necessary to be further investigated.

4 Critical influencing factors on the bond behavior

In this section, more than 30 existing literature were collected to investigate the influencing factors of the bond behavior of RAC with steel bar. This section indicated that the critical influencing factors (including the replacement rate of RCA, the concrete compressive strength, the steel bar type, the steel bar diameter, the steel bar embedment
length and the aggregate performance) and other influencing factors on the bond behavior of RAC with steel bar, and a detailed discussion is given below.

### 4.1 Replacement rate of RCA

Several investigations have been performed on the influence of the replacement rate of RCA on the bond behavior of RAC with steel bar, and there were still opposite opinions in existing research. Several previous studies point out that the bond strength increased with the increase of replacement rate of RCA [20, 29, 46–48].

It is widely accepted that the bond behavior of concrete will also increase with the improvement of mechanical properties. Therefore, it was necessary to take into account the influence of the compressive strength of RAC when analyzing the influence of RCA on the bond strength [38]. Most studies defined the normalized bond strength $\tau / \sqrt{f_c}$ ($\tau$ is the bond strength, and $f_c$ is the concrete compressive strength) to eliminate the influence of concrete strength. Prince and Singh [32, 33, 49] point out that the replacement rate of RCA had no detrimental influence on the normalized bond strength $\tau / \sqrt{f_c}$ of RAC with steel bar. On the contrary, it had a tendency to increase as the replacement rate of RCA increase. This agrees with the findings of Kim and Yun [36], Li et al. [25] and Xiao and Falkner [34]. Li et al. [25] tended to indicate the reason is that the surface of RCA is rougher and the biting force between RCA and new cement paste is greater. Xiao and Falkner [34] revealed the reason is that the elasticity modulus of RCA is similar to that of the cement paste, which improves the composite action between RCA and the cement paste.

For direct comparison, selected the results (the normalized bond strength) from the literature [25, 32–34, 36, 49], are plotted in Figure 4 which shows that the normalized bond strength increased with the increase of replacement rate of RCA.

In addition, Prince and Singh [35] point out that the traditional normalized bond strength $(\tau / \sqrt{f_c})$ overestimated the influence of concrete strength, resulting in the bond strength of high-strength concrete was overestimated. Therefore, the paper redefined a new normalized bond strength $\tau / \sqrt{f_c}^2$. The normalized bond strength $(\tau / \sqrt{f_c}^2)$ of RAC increased with the increase of replacement rate of RCA. The reason is that the brittleness index (the ratio of compressive strength to the splitting tensile strength) decreases with the increases of replacement rate of RCA, resulting in a higher fracture toughness of RCA.

However, some other test results showed that the bond strength decreased with the increase of replacement rate of RCA. Guerra et al. [20] investigated the influence of the replacement rate of RCA on the pull-out force, and found that the pull-out force of RAC specimens was lower than that of NAC, when the replacement rate of RCA was higher than 50%. Butler et al. [28] conducted a beam-type experiment on the bond behavior of RAC with steel bar, and the results showed that the bond strength of RAC was lower than that of NAC. Pour and Alam [37] demonstrated that the normalized bond strength $\tau / \sqrt{f_c}$ of RAC with steel bar decreased with the increase of replacement rate of RCA, but this trend was not obvious. Kim et al. [44] reported that the ultimate bond strength decreased with the increase of the replacement rate of RCA, while the slippage corresponding to the ultimate bond strength increased. Kim et al. [18] and Seara-Paz et al. [38] had shown similar conclusion. Kim et al. [18] also point out that the reason is that the density of RCA is lower than that of NCA, while the water absorption of RCA is higher.

For direct comparison, selected the results (normalized bond strength) from the literature [18, 37, 38, 44] was calculated and plotted in Figure 5, which shows that the normalized bond strength decreased with the increase of replacement rate of RCA.

In addition, Kim et al. [31] reported that with the increase of replacement rate of RCA, the difference between bond strength and slippage corresponding to the ultimate bond strength of high-strength concrete were not significant. Fernandez et al. [52] point out that the bond strength of the RAC specimen was comparable to that of NAC specimen. Breccolotti and Materazzi [53] revealed that the normalized bond strength of RAC was only slightly affected by the replacement level of RCA. Qiong et al. [54] demonstrated that the bond strength of RAC increased first and then de-
creased with the increased of the replacement rate of RCA, but it was always higher than that of NAC. The reason is that the RCA still absorbs the free water during the curing process, which reduces the influence of water-cement ratio. When the replacement rate of RCA is high, the strength increase caused by the reduction of the influenceive water-cement ratio becomes the secondary influencing factors, and the strength decrease caused by various unfavorable factors (such as higher discreteness, higher mud content and higher crushing index) becomes the critical influencing factors [55].

It can be seen that there were still opposite opinions about the influence of replacement rate of RCA on the bond strength in existing research, the reason is that most scholars only obtain the mix proportions of concrete with different replacement rate of RCA by replacing NCA with RCA, and maintained the same water-cement ratios, without considering the influence of different replacement rates of RCA on the concrete compressive strength. Butler et al. [28] investigated the bond behavior between RAC with two types of concrete mixture proportions and steel bar. The first type maintained the same water-cement ratios, while the second type was designed to obtain the same concrete compressive strengths. The results showed that the bond strength of NAC specimens was 10.4 to 19% higher than the first type RAC specimens, and 9.4 to 21.3% higher than the second type RAC specimens. It can be seen that it was necessary to take into account the influence of different replacement rates of RCA on the concrete compressive strength when analyzing the influence of replacement rate of RCA on the bond strength. Although the normalized bond strength \( \frac{\tau}{\sqrt{f_{cc}}} \) was used to eliminate the influence of concrete compressive strength on bond strength, the best way was to adjust the mix proportions of concrete to ensure that the concrete with different replacement rate of RCA. The bond strength of RAC with has same concrete compressive strength. This method can be widely used in future research.

4.2 Concrete compressive strength

Concrete compressive strength was another critical factor that influence the bond behavior of RAC with steel bar. Jun [23] and Ren [26] investigated the influence of the concrete compressive strength on the bond strength of RAC specimens, the results showed that the bond strength of RAC specimens increased with the increase of concrete compressive strength, the reason is that the chemical adhesion and the mechanical bite force between RAC and steel bar increases with the increase of concrete compressive strength. Xiu [45] revealed the similar conclusion that the ultimate bond strength and the normalized bond strength of RAC specimens increased with the increase of compressive strength of RAC. Seara-Paz et al. [38] reported that the ultimate bond strength of RAC specimens with 0.5 water-cement ratio (relatively high concrete compressive strength) was higher than that with 0.65 water-cement ratio (relatively low concrete compressive strength). Prince and Singh [32] had shown similar conclusion that the normalized bond strength of high-strength RAC was higher than that of normal-strength RAC by comparison with Prince and Singh [49], which was in agreement with the findings of Esfahani and Rangan [56] for NAC specimens. Although Kim et al. [18] point out that the ultimate bond strength tended to decrease as the concrete compressive strength increased, the normalized bond strength (calculated in this paper) generally increased with the increase of concrete compressive strength.

In order to reduce the influence of mechanical properties on the bond strength, selected the results (normalized bond strength) from the literature [18, 23, 33, 38] was calculated and plotted in Figure 6 which shows that the normalized bond strength increased with the increase of compressive strength of RCA.

However, Prince and Singh [35] point out that the normalized bond strength \( \frac{\tau}{\sqrt{f_{cc}}} \) of normal-strength (36 MPa) RAC was higher than that of medium-strength (51 MPa) and high-strength (68 MPa) RAC.

Overall, in the aspect of the influence of concrete compressive strength on normalized bond strength of RAC specimens, it was widely accepted that the bond strength of RAC increased with the increase of concrete compressive strength. Prince and Singh [35] point out the reason is that brittleness index increases with the increase of concrete compressive strength, and the bond strength increases with
the increase of its brittleness index. The author thinks the reason is that the improvement of concrete strength improves the bonding effect on steel bars, and then improves the bond strength of RAC specimens.

4.3 Steel bar type and steel bar diameter

Similar to the replacement rate of RCA and the concrete compressive strength, the steel bar type and the steel bar diameter were other two critical factors that influence the bond strength of RAC with steel bar.

Jun [23], Li et al. [25], Xiao and Falkner [34] and Cao et al. [50] had investigated the influence of steel bar type on the bond behavior of RAC with steel bar, and found that the bond strength of RAC with deformed steel bar was much higher than that with plain steel bar. Xiao and Falkner [34] point out that the reason of this result is that the deformed steel bars can provide stronger mechanical anchorage and friction resistance. Figure 7 shows the relationship between the bond strength of RAC specimens and the steel bar type.

It can be seen from Figure 7 that the bond strength of RAC with deformed steel bar was much greater than that with plain steel bars. Therefore, the steel bar type was a critical factor affecting the bond strength of RAC with steel bar. In order to ensure good bonding performance between steel bar and RAC, the deformed steel bar should be selected in RAC members.

In addition, Pour and Alam [37] found that the bond strength between RAC and steel bar decreased with the increase of steel bar diameter, and the bond strength of RAC with 19.5 mm was about 50% of that with 11 mm when the replacement rate of RCA was 30%. The reasons are following: On the one hand, the contact interface between RAC and steel bar increases with the increase of steel bar diameter, while the bond stress transferred to RAC decreases; On the other hand, the numbers of ribs under the same embedment length decreases with the increase of steel bar diameter. This agrees with the findings of Jun [23] and Cao et al. [50]. Prince and Singh [49] revealed the similar conclusion that the normalized bond strength of RAC with larger steel bar diameter was lower than that with smaller steel bar diameter. However, Li et al. [25] found that the bond strength increased with the increase of the steel rebar diameter.

The selected results (normalized bond strength) from the literature [20, 25, 37, 49] was calculated and plotted in Figure 8 which shows that the normalized bond strength decreased with the increase of steel bar diameter except that of Li et al. [25].
The bond strength of RAC with deformed steel bar is mainly the mechanical bite force between the steel bar ribs and RAC, while the main shape parameters of steel bars that affect the mechanical bite force are rib height and rib spacing. For the steel bar with same diameter, the higher the ribs, the smaller the spacing between ribs, the better the mechanical bite effect. The rib height and rib spacing of steel bars also change with the diameter of steel bars. Therefore, it is necessary to analyze the rib height and rib spacing as influencing factors when comparing the bond strength of RAC with different steel bar diameters.

4.4 Steel bar embedment length

A number of scholars conducted research regarding the influence of steel bar embedment length on the bond strength of RAC with steel bar [18, 23, 25, 26, 29, 30, 37, 50, 51, 54]. Pour and Alam [37] reported that the bond strength decreased with the increase of steel bar embedment length. The bond strength of RAC with $10d$ ($d$ is the steel bar diameter) embedment length was about 50% to 70% of that with $5d$ embedment length. Wardeh et al. [30] and Jun [23] got the same conclusion. The reasons are following: On the one hand, the voids at the interface between RAC and steel bar increases with the increase of embedment length; On the other hand, the bond stress is distributed along a longer embedded length, resulting in the decrease in bond strength.

However, Wang et al. [29] revealed that the bond strength increased with the increase of embedment length. Qiong et al. [54], Li et al. [25], Ren [26], Cao et al. [50] and Wang et al. [51] had shown similar conclusion, but Wang et al. [51] also point out that the average bond stress decreased with the increase of embedment length, the reason is that the longer the embedment length, the less uniform the bond stress distribution is.

4.5 Aggregate performance

Kim et al. [18] observed that the average water absorption and average density of the RCA showed a strong correlation with the bond strength of RAC specimens. The average water absorption and the average density of RCA were obtained with Eq. (11) and Eq. (12), respectively:

$$A_{ave} = \frac{\sum (ew \cdot ra \%)}{T}$$  \hspace{1cm} (11)

$$\rho_{ave} = \frac{\sum (ew \cdot rd)}{T}$$  \hspace{1cm} (12)

Where $ew$ is each coarse aggregate weight; $ra$ is relevant water absorption; $rd$ is relevant density; $T$ is total weight of coarse aggregate.

Figure 9 gives the relationship between the bond strength and the average water absorption, and Figure 10 gives the relationship between the bond strength and the density values. It can be seen that the bond strength decreased with the increase of average water absorption, while it increased with the increase of average density. However, the influence of the RCA on the bond strength was not significant when the concrete compressive strength was high. Kim et al. [44] revealed the similar conclusion that the ultimate bond strength decreased and the slippage corresponding to the ultimate bond strength increased with the increase of average water absorption.

Kim and Yun [36] found that the bond strength of RAC was also influenced by the size of the RCA, the bond strength of RAC with smaller RCA size (the maximum RCA size was 20 mm) was higher than that with larger RCA size (the maximum RCA size was 25 mm), the reason is that the

![Figure 9: Relationship between bond strength and average water absorption.](image)

![Figure 10: Relationship between bond strength and density values.](image)
RCA is round shape, and the segregation is more serious when flowing. Pandurangan et al. [57] conducted the influence of RCA after different treatment methods on the behavior of RAC with steel bar though the beam-type experiment. The results showed that the bond strength of RAC specimens depended on the quality of RCA, and the quality of RCA produced by thermal treatment, acid treatment and mechanical treatment was improved. The normalized bond strength of RAC with untreated RCA was lower than that of RAC with the treated RCA. The reason is that more than 89% of the adhered mortar is removed after thermal treatment, acid treatment and mechanical treatment, resulting in a decrease in the water absorption of the RCA. Butler et al. [28] reported that the bond strength of RAC with steel bar decreased with the increase of crushing value of RCA, the reason is that the splice strength and the fracture energy of RCA with lower crushing value is higher [58, 59].

4.6 Others

Jun [23], Pour and Alam [37], Wang et al. [51] and Qiong et al. [54] conducted research regarding the influence of concrete cover depth on the bond strength of RAC with steel bar, the results showed that the bond strength increased with the increase of concrete cover depth. Pour and Alam [37] revealed that the bond strength of RAC specimens with 67.5 mm concrete cover was 50% higher than that with 42.5 mm concrete cover. The reason is that the constraint level of the specimens with larger concrete cover is higher. Qiong et al. [54] point out that the bond strength increased significantly when the concrete cover was increased from 2d to 3d, while it almost unchanged when the concrete cover continued to increase. Seara-Paz et al. [38] investigated the influence of curing time on the bond strength of RAC specimens, and found that the bond strength of RAC increased significantly from 7 to 28 days, while it almost no increase from 28 to 365 days. Hu et al. [60] investigated the influence of concrete age on the bond strength of RAC specimens, and found that the bond strength increased with the increase of concrete age. Jun [23] revealed that that the provision of the horizontal stirrups in RAC specimen had no obvious influence on the bond strength.

5 Influence of freeze-thaw cycles and steel bar corrosion on the bond behavior

5.1 Freeze-thaw cycles

Freeze-thaw cycle will affect the performance of concrete, and then affect the bond behavior of concrete with steel bar [19, 61]. Some studies on bond behavior of RAC with steel bar after freeze-thaw cycles have been investigated. Shang et al. [62] investigated the bond behavior of RAC under the action of freeze-thaw cycles though pull-out test, and the results showed that the bond strength decreased with the increasing numbers of freezing and thawing cycles (as shown in Figure 11), while the slippage corresponding to the ultimate bond strength increased. Similar conclusions were also observed from the investigated by An et al. [63], Lu [64], Cao et al. [66, 67], Wang et al. [68, 69], Meng [70] and Wang et al. [71], Shang et al. [62] revealed that the reason is that the mechanical properties of RAC decreases after freeze-thaw cycles, and the internal structure of RAC become loose [72, 73]. In addition, Cao et al. [65] analyzed the reason for the decrease of the bond strength of RAC specimens after freeze-thaw cycles from microscopic perspective, and point out that the main reason is the frost damage of RAC. Wang et al. [68] also reported that that the initial slip bond strength (the bond strength when the free end of the steel bar slips) decreased after freezing and thawing cycles due to the internal damage of RAC. Shang et al. [74] observed that the influence of freezing and thawing cycles on the bond strength of RAC specimens without air entraining admixture was significantly greater than that with air entraining admixture. Li et al. [75] investigated the bond behavior of RAC specimens with different replacement rates of RCA after freeze-thaw cycles, and found that when the number of freeze-thaw cycles was less, the bond strength of RAC specimens was lower than that of NAC specimens. However, with the increase of freeze-thaw cycles, the bond strength of RAC specimens was higher than that of the NAC specimens. Su et al. [76, 77] investigated the bond behavior of RAC with steel bar after salt-frost cycles, and found that the bond strength of the RAC specimens in salt solution decreased more quickly and the slippage was larger than that in water.

In addition, Cao et al. [67] and Su et al. [76] and Liu et al. [78] investigated the influence of freezing and thawing cycles on bond stress distribution along with steel bar, and found that the maximum bond stress had a moving trend.
5.2 Steel bar corrosion

Several investigations have been performed on the bond behaviour of RAC with corroded steel bar, and the corrosion rate was controlled by the electrochemistry accelerated method. Bin [79], Cao et al. [80, 81], Wang et al. [82], Zhao [83], Wu [84] and Xiao et al. [85] point out that the bond strength of RAC specimens increased first and then decreased as the extent of steel bar corrosion increase, as shown in Figure 12. Bin [79] also demonstrated that the corrosion induced bond strength degradation of RAC specimens was similar to that of NAC specimens [74, 75], while the degradation degree of the bond strength of RAC with corroded steel rebar was much higher than that of NAC with corroded steel rebar. Zhao [83] revealed that that the ultimate bond strength of the RAC specimens with stirrups did not change significantly with the increase of the extent of steel bar corrosion. Fernandez et al. [52] demonstrated that the bond strength of RAC specimens with uncorroded steel bar was comparable to that of NAC specimens, while the bond strength of RAC specimens with low steel bar corrosion rate was higher than that of NAC specimens. Cao et al. [80] conducted a beam-type experiment on the bond behaviour of RAC with corroded steel bar, and the results showed that the failure mode of RAC specimens was splitting failure when the steel bar corrosion rate was 0% and 1%, while it was pullout failure when the steel bar corrosion rate was 9%, and it was pullout-splitting failure when the steel bar corrosion rate was 3% and 6%. Xiao et al. [85] observed that the failure mode of RAC specimens was pullout failure when the steel bar corrosion rate was small, while it was splitting failure when the steel bar corrosion rate was higher than 1.4%.

However, Dong [88] reported that the bond strength of RAC specimens decreased as the steel bar corrosion rate increase, and it was further supported by the experiment results of Yang [89]. Yang [89] also found that the bond stiffness and the ratio of initial slip bond strength to ultimate bond strength decreased as the steel bar corrosion rate increase.

In addition, Wang [82] studied the influence of steel bar corrosion rate on bond stress distribution along with steel bar, and found that the bond stress at the load end decreased with the increase of the steel bar corrosion rate, while that at the free end increased. This agrees with the findings of Cao [80]. Cao [81] reported that the bond stress distribution along with steel bars became relatively uniform with the increase of the steel bar corrosion rate. Yang [89, 90] had shown similar conclusion. Yang [89, 90] also point out that the maximum bond stress had a moving trend to the free end with the increase of the steel bar corrosion rate, and the bond stress rigidity increased at free end and load end, and it decreased in the range of 0.15-0.85 times the anchorage length.

All the above investigations accelerated the corrosion of steel bars by electrochemical method and shortened the experiment time. But it could not reflect the actual impact of corrosion due to its poor simulation of corrosion modality [92].

Overall, the freeze-thaw cycles and the steel bar corrosion all affected the bond behavior of RAC with steel bar. When freeze-thaw damage and steel bars corrosion reached a certain degree, the bond behavior of RAC with
steel bar deteriorated, which further affected the safety of RAC structure. Therefore, it was necessary to further investigate the bond behavior of RAC with steel bar, and find out the methods to prevent the bond behavior from deteriorating. In addition, with the development of graphene technology [93, 94], scholars have found that graphene was beneficial to improve the performance of concrete. In the future research, graphene can be used to improve the durability of RAC specimens.

6 Conclusions

This paper presented an overview of bond behavior of RAC with steel bar, which could draw the following conclusions:

(1) The failure modes RAC specimens is similar to that of NAC specimens. The failure modes of RAC specimen are not only influenced by a single factor. It is necessary to comprehensively consider the influence of various factors on the failure modes of RAC specimen. The steel bar diameter, the concrete cover to diameter ratio and the replacement rate of RCA all affect the load-slip curve of RAC specimen.

(2) Bond strength predictive equations of RAC with steel bar has been summarized to ensure engineers to better understand their applicability. Comparisons between different bond strength predictive equations of RAC with steel bar show that only one factor (concrete compressive strength) is taken into account in all bond strength these predictive equations, and the stirrup is not considered in those bond strength predictive equations, so it is necessary to be further investigated.

(3) Bond-slip relationship models of RAC with steel bar is summarized to ensure engineers to better understand their applicability. There is still no uniform regulation for the selection of the values of parameter $a$ and $b$, and the influence of some factors, such as the concrete cover depth, the embedded length and the stirrup on parameter $a$ and $b$ have not been investigated, and it is necessary to be further investigated.

(4) Over 30 existing literature are collected to investigated the influencing factors of the bond behavior of RAC specimens, and the critical influencing factors including replacement rate of RCA, concrete compressive strength, steel bar type and steel bar diameter, steel bar embedment length, aggregate performance.

(5) The bond degradation of RAC specimens after freeze-thaw cycles and steel bar corrosion are presented respectively. The freeze-thaw cycles and the steel bar corrosion all affect the bond behavior of RAC with steel bar. When freeze-thaw damage and steel bars corrosion reach a certain degree, the bond behavior of RAC with steel bar deteriorate, which further affect the safety of RAC structure.

Funding information: The study was carried out with the support of the Key Research and Development Program of Shandong Province (2019 GHY12076); the Natural Science Foundation of Shandong Province (ZR2020ME269).

Author contribution: Tian Su: Investigation, Visualization, Writing – Original Draft preparation, Writing – Review & Editing; Chenzia Wang: Writing – Original Draft preparation; Fubo Cao: Investigation; Zhenghao Zou: Writing – Review & Editing; Chunguang Wang: Visualization; Jun Wang: Supervision; Haihe Yi: Methodology, Formal analysis.

Conflict of Interests: The authors declare that they have no known competing financial interests or personal relationships that could have appeared to influence the work reported in this paper.

Data availability statement: The data used to support the findings of this study are available from the corresponding author upon request.

References

[1] Sivanathan, A., Q. Dou, Y. Wang, Y. Li, J. Corker, Y. Zhou, and M. Fan. Phase change materials for building construction: An overview of nano-/micro-encapsulation. Nanotechnology Reviews, Vol. 9, No. 1, 2020, pp. 896–921.

[2] Wu, J., X. Jing, and Z. Wang. Uni-axial compressive stress-strain relation of recycled coarse aggregate concrete after freezing and thawing cycles. Construction & Building Materials, Vol. 134, 2017, pp. 210–219.

[3] Meng, T., J. Zhang, H. Wei, and J. Shen. Effect of nanomaterials on the properties and microstructure of recycled concrete. Nanotechnology Reviews, Vol. 9, No. 1, 2020, pp. 79–92.

[4] Eguchi, K., K. Teranishi, A. Nakagome, H. Kishimoto, K. Shinozaki, and M. Narikawa. Application of recycled coarse aggregate by mixture to concrete construction. Construction & Building Materials, Vol. 21, No. 7, 2007, pp. 1542–1551.

[5] Zaharieva, R., F. Buyle Bodin, and E. Wirquin. Frost resistance of recycled aggregate concrete. Cement and Concrete Research, Vol. 34, No. 10, 2004, pp. 1927–1932.

[6] Guo, H., C. Shi, X. Guan, J. Zhu, Y. Ding, T.-C. Ling, et al. Durability of recycled aggregate concrete - A review. Cement and Concrete Reviews
Bairagi, N. K., K. Ravande, and V. K. Pareek. Behavior of concrete incorporating carbonated recycled concrete aggregates. *Cement and Concrete Composites*, Vol. 65, 2016, pp. 67–74.

Moroshashi, N., T. Sakurada, and K. Yanagibashi. Bond Splitting Strength of High-quality Recycled Coarse Aggregate Concrete Beams. *Journal of Asian Architecture and Building Engineering*, Vol. 6, No. 2, 2007, pp. 331–337.

Xiao, J., W. Li, Y. Fan, and X. Huang. An overview of study on recycled aggregate concrete in China (1996-2011). *Construction & Building Materials*, Vol. 31, 2012, pp. 364–383.

Kim, S. W., and H. D. Yun. Influence of recycled coarse aggregates on the bond strength between concrete and rebars. *Journal of Civil. Architectural & Environmental Engineering*, s1, 2016, pp. 6–12 (in Chinese).

Ren, F. Experimental study on bond behavior between recycled concrete and steel bar. North China University of Water Resources and Electric Power, 2016 (in Chinese).

Xu, G., X. Zhang, X. Xie and, Z. Wei. Influence of stirrups on bond property between steel bar and concrete under deicer-frosting environment. *Concrete (London)*, No. 8, 2016, pp. 11–15 (in Chinese).

Butler, L., J. S. West, and S. L. Tighe. The effect of recycled concrete aggregate properties on the bond strength between RCA concrete and steel reinforcement. *Cement and Concrete Research*, Vol. 41, No. 10, 2011, pp. 1037–1049.

Wang, C., H. Wei, J. Wu, and X. Wang. Experimental research and numerical simulation on bond behavior between recycled concrete and rebar. *Journal of Guangxi University: Nat Sci Ed*, No. 4, 2013, pp.996–1002 (in Chinese).

Wardeh, G., E. Ghorbel, H. Gomart, and B. Fiorio. Experimental and analytical study of bond behavior between recycled aggregate concrete and steel bars using a pullout test. *Structural Concrete*, Vol. 18, No. 5, 2017, pp. 811–824.

Kim, S. H., S. H. Lee, Y. T. Lee, and S. U. Hong. Bond between high strength concrete with recycled coarse aggregate and reinforcing bars. *Materials Research Innovations*, Vol. 18, sup2, 2014, pp. 278–285.

Prince, M. J. R., and B. Singh. Bond strength of deformed steel bars in high-strength recycled aggregate concrete. *Materials and Structures*, Vol. 48, No. 12, 2015, pp. 3913–3928.

Prince, M. J. R., and B. Singh. Bond behaviour of deformed steel bars embedded in recycled aggregate concrete. *Construction & Building Materials*, Vol. 49, No. 6, 2013, pp. 852–862.

Xiao, J., and H. Falkner. Bond behaviour between recycled aggregate concrete and steel rebars. *Construction & Building Materials*, Vol. 21, No. 2, 2007, pp. 395–401.

Prince, M. J. R., and B. Singh. Bond behaviour of normal- and high-strength recycled aggregate concrete. *Structural Concrete*, Vol. 16, No. 1, 2015, pp. 56–70.

Kim, S. W., and H. D. Yun. Influence of recycled coarse aggregates on the bond behavior of deformed bars in concrete. *Engineering Structures*, Vol. 48, 2013, pp. 133–143.

Pour, S. M., and M. S. Alam. Investigation of compressive bond behavior of steel rebar Embedded in concrete with partial recycled aggregate replacement. *Structures*, Vol. 7, 2016, pp. 153–164.

Seara-Paz, S., B. González-Fonteboa, J. Eiras-López, and M. F. Herrador. Bond behavior between steel reinforcement and recycled concrete. *Materials and Structures*, Vol. 47, No. 1-2, 2014, pp. 323–334.

Model Code 2010-Final draft. The international federation for structural concrete, FIB, Bulletin, No 52 Fib, 2; Lausanne, 2012.

Kim, Y., J. Sim, and Cl. Park. Mechanical properties of recycled aggregate concrete with deformed steel re-bar. *Journal of Marine Science and Technology-Taiwan*, Vol. 20, No. 3, 2012, pp. 274–280.
[41] Xiao, J., P. Li, and Q. Wei. Study on bond-slip between recycled concrete and rebars. *Journal of Tongji University*, Vol. 34, No. 1, 2006, pp. 13–16.

[42] Harajli, M. H. Development/splice strength of reinforcing bars embedded in plain and fiber reinforced concrete. *ACI Structural Journal*, Vol. 91, No. 5, 1994, pp. 511–520.

[43] Guo Z. Strength and deformation of concrete-experimental foundation and constitutive relationship. Beijing: Press of Tsinghua University; 1997 (in Chinese).

[44] Kim, S. W., W. S. Park, Y. I. Jang, S. J. Jang, and H. D. Yun. Bonding behavior of deformed steel rebars in sustainable concrete containing both fine and coarse recycled aggregates. *Materials (Basel)*, Vol. 10, No. 9, 2017, pp. 1082–1098.

[45] Xiu, H. Experimental study on bond-slip behavior between bar and recycled concrete. North China University of Technology, 2018 (in Chinese).

[46] Huang, Q. F., and D. F. Wang. Experimental Study on Bond-Slip between Steel Bar and Recycled Aggregate Concrete. *Advanced Materials Research*, Vol. 250-253, 2011, pp. 1651–1656.

[47] Singla, S. Compressive strength and bond behaviour of recycled coarse aggregate concrete. Ph.D. Thesis, Thapar University, Patiala, India, 2013.

[48] Steele, A.R. Bond performance of recycled aggregate concrete. Master’s Thesis, Missouri University of Science and Technology, Rolla, MO, USA, 2014.

[49] Prince, M. J. R., and B. Singh. Bond behaviour between recycled aggregate concrete and deformed steel bars. *Materials and Structures*, Vol. 15, No. 2, 2014, pp. 154–168.

[50] Cao, W., D. Lin, Q. Qiao, G. Chen, W. Jiang, and S. Peng. Experimental study on bond-slip properties and influence factors between rebars and recycled concrete. *Ziran Zhaihai Xuebao*, No. 5, 2017, pp. 36–44.

[51] Wang, D., Q. Huang, and Y. Shi. Experimental study on bond-slip between steel bar and recycled aggregate concrete. *Concrete (London)*, No. 6, 2011, pp. 64–66 (in Chinese).

[52] Fernandez, I., M. Etxeberria, and A. R. Márl. Ultimate bond strength assessment of uncorroded and corroded reinforced recycled aggregate concretes. *Construction & Building Materials*, Vol. 111, 2016, pp. 543–555.

[53] Breccolotti, M., and A. L. Materazzi. Structural reliability of bonding between steel rebars and recycled aggregate concrete. *Construction & Building Materials*, Vol. 47, No. 5, 2013, pp. 927–934.

[54] Qiong, H., W. Chen, and C. Zou. Experimental study on bonding properties of recycled concrete. *Journal of Harbin Institute of Technology*, Vol. 42, No. 12, 2010, pp. 1849–1854 (in Chinese).

[55] Xu, Y., S. Zhou, and J. Xiao. Experimental study of recycled concrete aggregate. *Journal of Building Materials*, Vol. 7, No. 4, 2004, pp. 447–450 (in Chinese).

[56] Esfahani, M. R., and V. B. Rangan. *Studies on bond between concrete and reinforcing bars*. School of Civil Engineering Curtin University of Technology, Perth, Western Australia, 1996, id. 315.

[57] Pandurangan, K., A. Dayanithy, and S. Om Prakash. Influence of treatment methods on the bond strength of recycled aggregate concrete. *Construction & Building Materials*, Vol. 120, No. 9, 2016, pp. 212–221.

[58] Darwin, D., S. Barham, R. Kozul, and S. Luan. Fracture energy of high-strength concrete. *ACI Materials Journal*, Vol. 98, No. 5, 2001, pp. 410–417.

[59] Zuo, J., and D. Darwin. Splice strength of conventional and high relative rib area bars in normal and high-strength concrete. *ACI Structural Journal*, Vol. 97, No. 4, 2000, pp. 630–641.

[60] Hu, X., G. Peng, D. Niu, and J. Wang. Experimental study on bond properties between early-age concrete and deformed steel bars. *Construction & Building Materials*, Vol. 236, 2020, id. 117593.

[61] Reiterman, P., O. Holčapek, O. Zobal, and M. Keppert. Freeze-thaw resistance of cement screed with various supplementary cementsitious materials. *Reviews on Advanced Materials Science*, Vol. 58, No. 1, 2019, pp. 66–74.

[62] Shang, H. S., T. J. Zhao, and W. Q. Cao. Bond behavior between steel bar and recycled aggregate concrete after freeze-thaw cycles. *Cold Regions Science and Technology*, Vol. 118, 2015, pp. 38–44.

[63] An, X.-z., C. Yi, X.-x. Wang, and X.-p. Jiang. Study of bond performance between steel bar and recycled aggregate concrete subjected to freeze-thaw cycling. *Journal of Experimental Mechanics*, Vol. 28, No. 2, 2013, pp. 227–234 (in Chinese).

[64] Lu, X. *The experimental study on the bond behaviour between recycled aggregate concrete and steel bar after freezing and thawing*. Qingdao University of Technology, 2014 (in Chinese).

[65] Cao, F., Y. Hou, Z. Tian, H. Guo, G. Zhao, and C. Wang. Study of bonding performance between steel bar and recycled concrete and the analysis of microstructure of recycled concrete after freezing and thawing. *Concrete (London)*, No. 2, 2016, pp. 17–20 (in Chinese).

[66] Cao, F., L. Tang, B. Ding, and C. Wang. Study on bond-slip properties between steel bars and recycled concrete after freeze-thaw cycles. *Engineering Mechanics*, 51, 2017, pp. 257–264 (in Chinese).

[67] Cao, F., R. Yin, and C. Wang. Beam-type experimental study on bond-slip behavior between recycled concrete and steel bar after freeze-thaw damage. *Journal of Building Structures*, Vol. 38, No. 4, 2017, pp. 141–148 (in Chinese).

[68] Wang, C., J. Qian, H. Wang, K. Xu, and F. Cao. Influence of the freeze-thaw damage on the durability of recycled concrete and the bonding performance of steel. *Journal of Chongqing University*, Vol. 39, No. 2, 2016, pp. 131–139 (in Chinese).

[69] Wang, C., L. Tang, Z. Zhang, J. Wang, and F. Cao. Pullout tests research on steel bars and recycled concrete after freeze-thaw cycles. *Building Structure*, No. 9, 2018, pp. 97–102 (in Chinese).

[70] Meng, X. *Experimental study on bond behavior between steel rebars and recycled concrete after freeze-thaw cycles*. Harbin Institute of Technology, 2015 (in Chinese).

[71] Wang, Z.-H., L. Li, Y.-X. Zhang, and W.-T. Wang. Bond-slip model considering freeze-thaw damage effect of concrete and its application. *Engineering Structures*, Vol. 201, 2019, id. 109831.

[72] Shang, H. S., Y. P. Song, and L. K. Qin. Experimental study on strength and deformation of plain concrete under triaxial compression after freeze-thaw cycles. *Building and Environment*, Vol. 43, No. 7, 2008, pp. 1197–1204.

[73] Shang, H., Y. Song, and J. Ou. Behavior of air-entrained concrete after freeze-thaw cycles. *Guti Lixue Xuebao*, Vol. 22, No. 3, 2009, pp. 261–266.

[74] Shang, H., Z. Wang, P. Zhang, T. Zhao, G. Fan, and G. Ren. Bond behavior of steel bar in air-entrained RCAC in fresh water and sea water after fast freeze-thaw cycles. *Cold Regions Science and Technology*, Vol. 135, 2017, pp. 90–96.

[75] Li, Z., Z. Deng, H. Yang, and H. Wang. Bond behavior between recycled aggregate concrete and deformed rebar after Freeze-thaw damage. *Construction & Building Materials*, Vol. 250, 2020, id. 118805.
Su, T., J. Wu, G. Yang, and Z. Zou. Bond behavior between recycled coarse aggregate concrete and steel bar after salt-frost cycles. *Construction & Building Materials*, Vol. 226, No. 11, 2019, pp. 673–685.

Su, T., J. Wu, Z. Zou, and J. Yuan. Bond performance of steel bar in RAC under salt-frost and repeated loading. *Journal of Materials in Civil Engineering*, Vol. 32, No. 9, 2020, pp. 04020261–1.

Liu, K., J. Yan, X. Meng, and C. Zou. Bond behavior between deformed steel bars and recycled aggregate concrete after freeze-thaw cycles. *Construction & Building Materials*, Vol. 232, 2020, id. 117236.

Bin, L. Bond behaviour between Recycled aggregate concrete and corroded steel rebars. *Applied Mechanics and Materials*, Vol. 166-169, 2012, pp. 1391–1394.

Cao, F., W. Chen, and T. Geng. Experimental study on bond-slip behavior between recycled concrete beam reinforced with corroded bars. *Journal of Building Structures*, Vol. 37, No. 5, 2016, pp. 297–305 (in Chinese).

Zhao, Y., H. Lin, K. Wu, and W. Jin. Bond behaviour of normal/recycled concrete and corroded steel bars. *Construction & Building Materials*, Vol. 48, 2013, pp. 348–359.

Kang, W. Bond behaviour of normal/recycled concrete and corroded steel bars. Zhejiang University, 2012 (in Chinese).

Xiao, J., and L. Bin. Experimental study on bond behavior between corroded steel bars and recycled concrete. *Journal of Building Structures*, Vol. 32, No. 1, 2011, pp. 58–62 (in Chinese).

Abosrra, L., A. F. Ashour, and M. Youssefi. Corrosion of steel reinforcement in concrete of different compressive strengths. *Construction & Building Materials*, Vol. 25, No. 10, 2011, pp. 3915–3925.

Coccia, S., S. Imperatore, and Z. Rinaldi. Influence of corrosion on the bond strength of steel rebars in concrete. *Materials and Structures*, Vol. 49, No. 1-2, 2016, pp. 537–551.

Dong, H., Y. Zhang, W. Cao, and T. Ye. Experimental study on bond-slip behavior between rusted steel bars and recycled aggregate concrete. *Ziran Zaihai Xuebao*, No. 05, 2017, pp. 12–23 (in Chinese).

Yang, H., X. Li, C. Jian, Y. Zhou, and F. Xing. Experimental study on bond-slip properties between corroded rebar and recycled concrete. *Bulletin of the Chinese Ceramic Society*, Vol. 34, No. 4, 2015, pp. 902–908 (in Chinese).

Yang, H., Z. Deng, and Y. Tan. Experimental study on bond-slip relationship between corroded rebar and recycled concrete. *Engineering Mechanics*, Vol. 32, No. 10, 2015, pp. 114–122 (in Chinese).

Yang, H., Z. Deng, and J. M. Ingham. Bond position function between corroded reinforcement and recycled aggregate concrete using beam tests. *Construction & Building Materials*, Vol. 127, 2016, pp. 518–526.

Li, F., and Y. Yuan. Influences of corrosion on bond behavior between steel strand and concrete. *Construction & Building Materials*, Vol. 38, No. 2, 2013, pp. 413–422.

Sara, I. Ahmad, Hicham Hamoudi, Ahmed Abdala, Zafar K. Ghouri, and Khaled M. Youssef. Graphene-Reinforced Bulk Metal Matrix Composites: Synthesis, Microstructure, and Properties. *Reviews on Advanced Materials Science*, Vol. 59, No. 1, 2020, pp. 67–114.

Liu, Y., M. Jia, C. Song, S. Lu, H. Wang, G. Zhang, et al. Enhancing ultra-early strength of sulphoaluminate cement-based materials by incorporating graphene oxide. *Reviews on Advanced Materials Science*, Vol. 9, 2020, pp. 17–27.