An Open-Source Microscopic Traffic Simulator

Martin Treiber and Arne Kesting

Abstract—We present the interactive Java-based open-source traffic simulator available at www.traffic-simulation.de. In contrast to most closed-source commercial simulators, the focus is on investigating fundamental issues of traffic dynamics rather than simulating specific road networks. This includes testing theories for the spatiotemporal evolution of traffic jams, comparing and testing different microscopic traffic models, modeling the effects of driving styles and traffic rules on the efficiency and stability of traffic flow, and investigating novel ITS technologies such as adaptive cruise control, inter-vehicle and vehicle-infrastructure communication.

I. INTRODUCTION

In the late 1990s, the simulator presented in this contribution started off as an educational project. Later on, a version of it was made publicly available in form of the Internet applet at the website www.traffic-simulation.de. Besides interactive simulations of various predefined traffic situations (see Fig. 1 for a screenshot), this site discusses the applied microscopic traffic models [1], [2] and offers open access to the Java source code.

In spite (or just because) of its simplicity, the applet became popular in the course of time. It was displayed, e.g., on the German computer exhibition CeBIT 2009 and 2010, in the Wall Street Journal (July 1, 2005), and in several radio and TV broadcasts (see also Fig. 2).

As primary purpose, the simulator demonstrates the dynamics of traffic flow to the general public. For example, when simulating the “ring-road” and selecting intermediate densities, one can observe traffic instabilities eventually leading to stop-and-go waves. When choosing real open systems (scenarios 2-4), however, one needs three building blocks to cause a jam: (i) high traffic demand, (ii) a bottleneck, and (iii) a perturbation in the traffic flow [3]. The traffic demand can be controlled by sliders (“main inflow” and “ramp inflow”) while the bottleneck (on-ramp, lane closing or an uphill grade) is selected by simulating the appropriate scenario. To enhance the breakdown, perturbations to the traffic flow can be added by letting one vehicle brake without reason [1]. It is instructive to see that the car actually causing the breakdown (marked with a different color in the simulation) is not affected by it. The driver is likely not even aware of the consequences of his or her maneuver because the breakdown takes several minutes to develop [1].

The computer program can also be used to simulate the effects of roadside control. For example, running the lane-closing scenario with the default settings will result in a traffic breakdown at the bottleneck. However, imposing a speed limit of 80 km/h will prevent the breakdown. In this case, cars and trucks drive at nearly the same (desired) speed and lane changes from the lane to be closed can be performed comparatively easy. This reflects the golden rule of traffic-flow optimization stating that perturbations in the traffic flow should be minimized [3]. Finally, the effects of different driving styles can be demonstrated: “Agile” drivers (which are characterized by high values of the acceleration parameter) dampen traffic waves and help dissolving the jam. In contrast, non-anticipative drivers (braking at the very last moment and characterized by a high value of the deceleration parameter) destabilize traffic flow [6].

Extensions of the Internet simulator, mainly targeted for car and traffic engineers, are not yet available on the website but fully operating and documented. They include

- a simulation testbed for assessing the performance of a wide variety of models in standard scenarios [4]. This includes time-continuous models, iterated coupled maps, cellular automata, and mixtures thereof.
- simulating varying percentages of vehicles equipped with driving-assistance systems such as semi-automated driving (adaptive cruise control, ACC) [5], [6]
- A basis for simulating the performance and efficiency of various protocols for car-to-car (C2C) and car-to-

1M. Martin and A. Kesting are with the Department of Transport and Traffic Sciences, Technische Universität Dresden, Würtzburger Str. 35, 01062 Dresden, Germany. e-mail: (see http://www.mtreiber.de and http://www.aesting.de).
2Since further perturbations are inherently provoked by lane changes, traffic can also break down without external action.
3The default parameter settings correspond to extremely unstable traffic. For more realistic values, it would take even longer [4].
The IDM acceleration combines the free-road acceleration strategy $a_{\text{free}}(v) = \alpha_1 - (v/v_0)^\delta$ with a deceleration strategy $a_{\text{brake}}(s, v, \Delta v) = -\alpha a(s^*/s)^2$ that becomes relevant when the gap to the leading vehicle is not significantly larger than the effective “desired” (safe) gap $s^*(v, \Delta v)$. The free acceleration is characterized by the desired speed $v_0$, the maximum acceleration $\alpha$, and the exponent $\delta$ characterizing how the acceleration decreases with the speed ($\delta = 1$ corresponds to a linear decrease while $\delta \to \infty$ corresponds to a constant acceleration). The effective minimum gap $s^*$ is composed of the minimum distance $s_0$ (which is relevant for low velocities only), the speed-dependent distance $vT$ which corresponds to following the leading vehicle with a constant desired time gap $T$, and a dynamic contribution which is only active in non-stationary traffic corresponding to situations in which $\Delta v \neq 0$. This latter contribution implements an intelligent driving behavior that, in normal situations, limits braking decelerations to the comfortable deceleration $b$. In critical situations, however, the IDM deceleration becomes significantly higher, making the IDM collision-free [11]. Notice that the IDM acceleration function (1) is continuous and the different driving situations are connected by implicit smooth transitions, not by explicit conditions.

The IDM parameters $v_0$, $\alpha$, $b$, $T$, and $s_0$ (the first four can be changed interactively, see Fig. 1) have a reasonable interpretation, are known to be relevant, are empirically measurable, and have realistic values [10]. Furthermore, the model is complete in the sense that it describes all traffic situations (such as free and congested traffic including all transitions) on freeways as well as in cities. For two situations, it has two minor imperfections:

1) IDM drivers overreact in response to very small gaps that may be caused by “cut-in” lane-changing maneuvers of other drivers. This is relevant for ACC applications and for realistic lane-changing behavior. A modification resolving this issue is proposed in Ref. [6].

2) In car-following situations near the desired speed, the equilibrium time gap becomes significantly larger than the time-gap parameter $T$. This is undesirable in ACC implementations where the time gap can be set by the driver (“you should get what you want”). Moreover, an unrealistic spreading is observed in some platooning situations. A solution consists in replacing Eq. (1) with

$$\frac{dv}{dt} = \begin{cases} \alpha \left[ 1 - \left( \frac{s^*}{s} \right)^2 \right] & \frac{s^*}{s} \geq 1, \\ a_{\text{free}} \left[ 1 - \left( \frac{s^*}{s} \right) \right] & \text{otherwise}. \end{cases}$$

Both modifications neither change the parameter set nor the dynamics in other situations nor the continuity of the acceleration function. Nevertheless, for the sake of simplicity, we
will retain the original IDM acceleration $a$ with Eq. (2) in future versions, it will be possible to choose between several models and variants.

**B. Lane-Changing Model MOBIL**

Like the acceleration model, the lane-changing model is selected according to the Einstein postulate which can be paraphrased by *make it as simple as possible – but not simpler*. Consequently, all the complicated strategic and tactical preparation stages are discarded focussing exclusively on the operational (ad-hoc) lane-changing decision. Furthermore, complicated traffic rules (such as forbidding overtaking on the right lane) are ignored. However, speed differences play a crucial role and must be retained as explanatory variable (gap-acceptance models where lane-changing decisions are just based on speed-dependent gaps are not sufficient). Moreover, the lane-changing decision must be compatible with the acceleration model. In particular, the combined models should be accident-free if this is the case for the longitudinal model alone. In commercial simulators this is achieved by nesting the two submodels to a complex compound acceleration and lane-changing model.

Remarkably, this complication can be avoided by simple acceleration-based lane-changing rules [2]. For a vehicle $c$ considering a lane change, the subsequent vehicles on the target (new) and present (old) lanes are represented by $n$ and $o$, respectively. The acceleration $a_c$ denotes the (IDM) acceleration of vehicle $c$ on the actual lane, while $\bar{a}_c$ refers to the prospective situation after a completed change, i.e., to the new acceleration of vehicle $c$ on the target lane. Likewise, $\bar{a}_o$ and $\bar{a}_n$ denote the acceleration of the old and new followers after completion of the lane change.

The set of lane-changing rules can be comprised by two criteria. The *safety criterion* checks the possibility of executing a lane change by allowing only changes where the subsequent vehicle on the target lane is not forced to brake at a deceleration exceeding a given safe limit $b_{safe}$,

$$\bar{a}_n \geq -b_{safe}. \tag{4}$$

Although formulated as a simple inequality, this condition contains all the information provided by the acceleration model. In particular, the admissible minimum time gap may be significantly larger than $T$ (fast target vehicle, $v_n > v_c$), or significantly smaller (slower target vehicle).

The *incentive criterion* determines if a lane change is desirable. Like the safety criterion, “desirability” is based on the accelerations of the longitudinal model before and after the prospective lane change. Furthermore, we generalize the incentive criterion to include the immediately affected neighbors as well. The *politeness factor* $p$ determines to which degree these vehicles influence the lane-changing decision (cf. Fig. 1). For symmetric overtaking rules, we neglect differences between the lanes and propose the following incentive condition for a lane-changing decision of the driver of vehicle $c$:

$$\bar{a}_n - a_o + p(\bar{a}_n - a_o + \bar{a}_o - a_o) > \Delta a_{th}. \tag{5}$$

The first two terms denote the advantage (utility) of a possible lane change for the driver of the considered vehicle $c$. The third term with the *politeness factor* $p$ denotes the total advantage (acceleration gain or loss, if negative) of the two immediately affected neighbors, weighted with $p$. Finally, the switching threshold $\Delta a_{th}$ on the right-hand side of Eq. (5) models a certain inertia and prevents lane changes if the overall advantage is only marginal compared to a “keep lane” directive. Notice that the incentive criterion acts simultaneously as a *safety* criterion for the lane-changing vehicle $c$ (in contrast to the safety criterion 4) ensuring the safety of vehicle $n$. In order to avoid erroneous decisions for the case of two vehicles driving in parallel, the acceleration model must return a prohibitively negative acceleration for this case (corresponding to negative gaps). Finally, asymmetric traffic regulations such as keep-right directives can be implemented by an additional bias term $\pm a_{bias}$ (positive for changes to the left) on the right-hand side of Eq. (5) [2].

**III. SIMULATING EMPirical PHENOMENA**

Traffic congestions cannot evolve arbitrarily in space and time. Instead, observations on freeways all over the world indicate that the dynamics of traffic jams obeys certain empirical rules, also known as *stylized facts* [3]. Taking them into account can greatly improve the traffic state estimation from sparse data [11]. Some of the facts that can be seen in Fig. 3 (top) are the following (for a complete list, see Ref. [3]):

- Nearly all breakdowns are caused by bottlenecks in combination with a perturbation in the traffic flow. In Fig. 3 the bottlenecks are constituted by two intersections, and by a temporary lane closing (accident) near road kilometer 476. Other types include junctions (scenario 2 in the Internet simulator), road works (scenario 3), or gradients (scenario 4).
- The downstream front (where cars leave the congestion) is either stationary at the bottleneck, or moves against the driving direction with a characteristic velocity $v_{cong}$ of about -15 km/h. Both cases can occur in a single jam (see Fig. 3 at road kilometer 476 km at time 10:10).
- Jams can be localized or extended. Most extended jams show internal structures propagating all at the same velocity $v_{cong}$. Furthermore, they grow in amplitude eventually resulting in stop-and-go waves or even isolated moving jams (perceived as “phantom jams” by drivers).

In the simulated speed field shown in Fig. 3 (bottom), the simulator is set up with upstream and downstream boundary conditions from the corresponding detectors. The two permanent bottlenecks are simulated by simple on-ramps while the temporary lane closing is modeled by a “flow-conserving bottleneck”: The model parameters $v_0$ and $T$ are changed locally and temporally such that the effective road capacity is reduced in the corresponding spatiotemporal region. The

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1 Moreover, some drivers do increase their gap when following other vehicles near the desired speed. For these drivers, the original IDM is even more realistic than the modification $\tilde{a}_c$.

2 Most other lane-changing models correspond to setting $p = 0$. 

simulation reproduces the complete set of stylized facts with just the boundary conditions and the arrangement of bottlenecks as input. However, the agreement with the data is not quantitative. Particularly, the wavelengths of the simulated stop-and-go waves are too short. This could be resolved by more elaborate models.

IV. APPLICATIONS

Intelligent traffic systems (ITS) such as adaptive cruise control (ACC) or car-to-car and car-to-infrastructure communication (C2C/C2I) are becoming increasingly widespread and begin to influence traffic operations. This opens up new vehicle-based possibilities to optimize the efficiency of traffic flow. The objective function to be maximized typically contains savings in traveling time or fuel consumption/CO\textsubscript{2} emissions. In the following subsections, we show how our software can be applied to calculate fuel consumptions, and to simulate the effects of novel ITS systems. Since these systems do not yet exist, simulation is the only means to assess their properties.

A. Fuel Consumption

When discussing how much a specific traffic situation (particularly, jams) and the driving style (including automated driving) influence fuel consumption and emissions, one needs a detailed microscopic (modal) consumption model. Our simulator implements the physics-based model described in Ref. [13].

This model contains intuitive and measurable parameters such as mass, friction and wind-drag coefficients of the respective vehicle. Moreover, its exogenous variables, namely speed \( v \) and acceleration \( \dot{v} \), are just the output variables of the actual dynamic simulation. Specifically, the consumption rate \( \dot{c} \) (liters per time unit) of a vehicle is given by

\[
\dot{c}(v, \dot{v}) = \frac{P}{\gamma(P, f)u_{\text{cal}}}
\]

where the required total power \( P \) depends, via a physical formula, on speed, acceleration, road gradient, and some vehicle properties [13]. Furthermore, the fraction \( \gamma \) of the fuel calorific energy (given per volume by \( u_{\text{cal}} \)) which is converted into mechanic energy depends, via the engine characteristics, on \( P \) and the rotation rate \( f \). The rotation rate, in turn, depends on the speed and the chosen gear.

Figure 4 shows an application of the fuel-consumption module in three situations with variable amounts of jammed traffic. Assumed are a level road, a certain type of middle-class vehicle, and a fuel-efficient gear choice for a given speed and acceleration. We obtain the remarkable result that the relative savings in travel time by jam avoidance are at least three times higher than that in consumption/emissions.

B. Adaptive Driving Behavior

As discussed in Sec. III, one “building block” for traffic breakdowns are bottlenecks, i.e., road sections with locally decreased capacity. However, the capacity, i.e., the maximum possible throughput (vehicles per hour), does not only depend on the infrastructure (number of lanes etc.) but also on the driving style. Consequently, by local changes of the driving style, it is possible to dynamically fill the capacity gaps of...
the bottlenecks, at least partially. This is the rationale behind the proposed adaptive driving behavior⁵,⁶ which can be implemented either by automated driving ("traffic-adaptive ACC⁴), or by hints about effective driving which could be taught in driving lessons.

Adaptive driving is implemented in our simulator by the “strategy matrix” coupling a set of driving situations to a set of actions (cf. Fig. 5). The five driving situations (free and congested traffic, jam entering and leaving, driving through a bottleneck) relate to traffic situations as well as to the infrastructure. The actions, i.e., changes of the driving style, are modeled by changes of the IDM parameters a, b, and T. Here it comes in handy that these parameters are intuitive and correspond to nearly orthogonal aspects of the driving style:

- The agility of the drivers is positively correlated to the acceleration parameter a,
- the degree of anticipation is negatively correlated to the comfortable braking deceleration as discussed in Sec. II.A,
- and the following distance (aggressiveness if too close) is determined by the desired time headway T.

Notice that the set of actions does not include changes of the desired velocity v₀ since this would reflect the effect of roadside control (speed limits) rather than in-vehicle traffic optimization.

The complete adaptation strategy is encoded by the fifteen matrix elements denoting the relative change of the parameter values with respect to the default driving style⁶. The matrix elements are given in relative terms. Thus, changes of the driving style are orthogonal to inter-driver variations representing the heterogeneity of drivers and vehicles.⁶ The strategy represented by the matrix of Fig. 5 correspond to comparatively modest actions (only five elements are different from unity) which, nevertheless, lead to a significant increase of efficiency. This strategy can be expressed by following rules:

- Braking early when approaching a jam (row 2),
- increasing the agility and reducing gaps (of course, without violating safety limits) at bottlenecks (row 4),
- closely following the leader and accelerating fast when leaving a jam (row 5).

Figure 4 shows three simulations with identical traffic volume where 0%, 10%, and 20% of all vehicles are equipped with traffic-adaptive ACC (or driven by traffic-aware drivers). With respect to capacity, this strategy leads to an effective sensitivity ε = 0.3, i.e., 1% more equipped vehicles will increase the capacity by about 0.3%. However, with respect to maximum travel times (TT) and consumption rates (C), the nonlinearity associated with jams constitute a significant leverage effect resulting (in this specific situation) in ε_{TT} ≈ 4 and ε_{C} ≈ 1.

C. Car-to-Car and Car-to-Infrastructure Communication

Traffic-adaptive driving strategies such as the one presented in the previous subsection are only effective if the local traffic situation is known. For example, to identify the traffic situations corresponding to the rows 2 and 5 of the strategy matrix, the precise locations of the upstream and downstream fronts of congestions must be known. Since, from the driver’s perspective (Fig. 7), one cannot distinguish between jam fronts and spurious oscillations (Fig. 3), the information must be provided by another assistance system. Figure 4 sketches a possible solution using store-and-forward car-to-car communication between equipped vehicles. In order to be effective for low percentages of equipped vehicles, cars driving in the opposite direction are used as information carriers. Theoretical calculations show that this concept is operative for equipment rates as low as 2%⁷. Integrated simulations of the communication and traffic dynamics using our simulator essentially confirm the analytical results⁷–⁹.

Another variant of local ad-hoc communication is C2I communication which is particularly effective at known bottlenecks such as road works. In this setup, the information transport is not performed by “courier vehicles” but by a set of at least two connected “road-side units” (RSU), one at the bottleneck, and one several kilometers upstream. Each equipped vehicle records its trajectory and delivers it to the downstream RSU by local communication (WLAN). After analyzing it there, relevant information (such as travel times or positions of jam fronts) is transmitted to the upstream RSU which, in turn, delivers it to passing equipped vehicles by local communication. Finally, the information can be graphically presented by the in-vehicle navigation system (Fig. 7).

V. Future Projects

Because of its open architecture and full control, the simulator can be extended in the following aspects. On the website www.verkehrsdynamik.de we will offer a publicly available platform to test the quality of existing and user-defined...
car-following models in urban and freeway scenarios [4]. Furthermore, quality criteria such as fuel consumption [13] or total travel times from both the individual and system perspective will be made available in a future version. A unique feature of the 3D version of our simulator is an intuitive visualization of the “driving comfort”. We represent this quantity which is rather elusive to define in mathematical terms, by a cup of virtual coffee which spills if the driving becomes uncomfortable [14]. The dynamics of the coffee surface is defined by a twodimensional driven damped pendulum [15] taking into account longitudinal and transversal acceleration and jerk (the time derivative of the acceleration) which all contribute to the subjective impression of discomfort.

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