The rockfall hazard is one of geological hazards in mountainous zone. The life and property safety of people near the town or residential zone is often threatened by the collapse of dangerous rock mass [1], for example, abrupt landslide [2] in Nanzhang county, Hubei province, China in January 20, 2017. The scale of fractured rock mass is about 3000m³, the part buildings in a hotel are collapsed, and the death toll is twelve, direct economic loss arrives at more than 20 millions yuan. So the risk assessment of rockfall hazards has great significance in reality [3].

The risk assessment of the rockfall hazards has been investigated by many researchers [4]. The qualitative investigation on the assessment method is primary at the early stages. More and more mathematical models are applied to assess the risk level of rockfall hazards as deep investigations are performed. The hierarchical analytical method is used to analyze the stability of dangerous rock mass in Chongqing by Dong etc [5]. And then the norm grey theory is provided by Li, etc [6] to assess the influential factor of dangerous rock formation. Variable fuzzy set theory is suggested by...
Wu, et al [7], and it is compared with other methods, the results demonstrate that variable fuzzy set theory has better applicability relative to other methods to assess risk level of rockfall under earthquake loading [8]. And then the probability index method [9], the method about information amount [10], logistic regression model [11] are respectively applied to assess the risk level in different zones, and the precision about three models is compared [12]. The hierarchy analytical synthetic index theory is provided by Wang, et al. [13] to assess the risk level of single hidden trouble point. The risk assessment of rockfall hazard area in highways is performed by Wang, et al. [14] using GIS based on superposition theory about regional influential factors; AHP-fuzzy synthetic assessment theory is suggested by Ye, et al. [15] to predict the stability of dangerous rock mass in combination with hierarchy analysis and fuzzy comprehensive evaluation method. Although the risk assessment is improved enormously because of the application of above methods, these methods still exists some limitations [16], for instance, their computational loads are great, and the correlation between assessment indices is not considered [17], and great complexity, fuzziness and randomness of rockfall hazards occurrence are ignored. The above shortcomings are solved successfully by using AHP-Normal cloud theory. The inner relationship between fuzziness and randomness can not only be expressed [18], but also the conversion between qualitative concepts and quantitative characteristics can also be realized because of the application of AHP-Normal cloud model. Nowadays, the cloud model is widely used in many fields, it is applied to assess the risk level of rockfall hazards in Laoying Yan in the paper.

The paper is organized as follows: in Section 1, the engineering background in the study area is introduced at first. In Section 2, a new risk assessment method of rockfall hazards is introduced based on the cloud model and the AHP algorithm. In Section 3, the AHP-Normal cloud model is established about rockfall hazards in Laoying Yan, and the assessment results of the proposed cloud model are discussed. In Section 4, conclusions are drawn.

Materials and Methods

Study Area

Jiang Jin zone is located in the southwestern region in Chongqing province, China. And it was sitting at the upper stream of Yangtze River, the end of three gorges reservoir area. The area arrives at 3200 square kilometers. Laoying Yan is lied in the Shuangxi village, Longhua town, Jiangjin zone. The village load XC98 goes through between the Longhua town and Jiangjin zone, its topography is gentle. The survey area is plotted in Fig. 1. The geomorphology in the survey area belongs to denudation and erosion valley and hill, its terrain is high in the East and low in the West. Its highest point lies at the top of dangerous rock mass W1 in Laoying Yan, its elevation is 326.11m. The lowest point lies at the slope angle of ramps, its elevation is 142.21 m, their relative elevation difference is 183.9 m, the terrain is very steep, the slope angle is about 20°~45°, the local slope angle represents the upright states, it is shown in Figs 2 and 3. The stratigraphic lithology in the survey area are composed of four types: quaternary holocene colluvial soil, residual soil layers of slope, middle jurassic bright red mudstone and sandstone interbed. The strata is continual and stable in the survey area, the inclination angle of rock stratum is gentle. The altitude of rock stratum are between 65°∠4° and 113°∠12°, there are not faults and fracture zones, the regional tectonic stability is good. But once the dangerous rock mass collapsed, the life and property safety of people will be seriously endangered, so it is very essential to assess the risk of rockfall hazards in Laoying Yan.
Firstly, a complete assessment index systems are built up. Secondly, the weight coefficients of different assessment indices are calculated by using AHP method. Finally, the comprehensive certainty degree is determined based on normal cloud model, and then the risk level of rockfall hazards can be obtained.

The Establishment of Index Systems

Rockfall hazards are caused by many factors, these factors are very complicated. According to relevant investigations [19], they can be categorized as three types: topographic and geological condition $U_1$ (including the height of stiff cliff $U_{11}$, construction feature of still cliff $U_{12}$ and the geological structure of stiff cliff $U_{13}$). Geometry character of dangerous rock mass $U_2$ (including the scale of rockfall mass $U_{21}$, the inclination angle of main control plane $U_{22}$ and the through degree of main control plane $U_{23}$). Other factors $U_3$ (including the daily maximum rainfall $U_{31}$, weathering action $U_{32}$, the earthquake intensity $U_{33}$ and human engineering activity $U_{34}$). In the established evaluation system, indices $U_{11}$, $U_{21}$, $U_{22}$, $U_{23}$ and $U_{31}$ are quantitative indices, and the index values were obtained by the measured data in the projection. The rest indices are qualitative, and their values are determined by

![Fig. 4. The risk assessment process of rockfall hazards based on a AHP-Normal cloud model.](image-url)
the expert investigation. In combination with rockfall hazard classification, the ten risk assessment indices were divided into four levels: low risk (I), medium risk (II), high risk (III), and higher risk (IV), as shown in Table 1.

To determine the risk level of rockfall hazards, the degree of membership about five quantitative indices can be shown in Table 2 as follows:

The Height of Still Sliff ($U_{11}$)

According to the geological survey, the development and collapse of dangerous rock often happens in stiff and high slope area. The stress redistribution is aroused by the still cliff. The tension stress concentration formed at the shoulder of cliff. The shear stress formed at the foot of cliff.

### Table 1. The standard classification of assessment index about rockfall hazards.

| Assessment index | Code | The risk level |
|------------------|------|----------------|
| The height of stiff cliff (m) | $U_{11}$ | I | II | III | IV |
| Construction feature of still cliff | $U_{12}$ | No weak structural plane | Inclined slope weak structural plane | Horizontal weak structural plane | Inclined out-slope weak structural plane |
| The geological structure of still cliff | $U_{13}$ | Horizontal rock stratum | Anticline rock stratum | bedding rock stratum | Anticlinal nucleus or fracture zone |
| The scale of rockfall mass ($10^4$ m$^3$) | $U_{21}$ | [0 1] | [1 10] | [10 100] | ≥100 |
| The inclination angle of main control plane (°) | $U_{22}$ | [0 45) | [45 60) | [60 75) | [75 90] |
| The through degree of of main control plane | $U_{23}$ | [0 0.25) | [0.25 0.5) | [0.5 0.75) | [0.75 1] |
| The daily maximum rainfall (m) | $U_{31}$ | [0 10) | [10 25) | [25 50) | ≥50 |
| Weathering action | $U_{32}$ | no weathering | Weak weathering | Medium weathering | Strong weathering |
| The earthquake intensity | $U_{33}$ | ≤V | VI | VII | ≥VIII |
| Human engineering activity | $U_{34}$ | No influence | Weak influence | Medium influence | Strong influence |

| Assessment index | Code | The degree of membership |
|------------------|------|--------------------------|
| The height of stiff cliff (m) | $U_{11}$ | I = 1, II = 0 | I = 0, II = 1 | I = 0, II = 0 | I = 0, II = 0 |
| Construction feature of still cliff | $U_{12}$ | III = 0, IV = 0 | III = 0, IV = 0 | III = 1, IV = 0 | III = 0, IV = 1 |
| The geological structure of still cliff | $U_{13}$ | Horizontal rock stratum | Anticline rock stratum | bedding rock stratum | Anticlinal nucleus or fracture zone |
| The scale of rockfall mass ($10^4$ m$^3$) | $U_{21}$ | III = 0, IV = 0 | III = 0, IV = 0 | III = 1, IV = 0 | III = 0, IV = 1 |
| The inclination angle of main control plane (°) | $U_{22}$ | III = 0, IV = 0 | III = 0, IV = 0 | III = 1, IV = 0 | III = 0, IV = 1 |
| The through degree of of main control plane | $U_{23}$ | III = 0, IV = 0 | III = 0, IV = 0 | III = 1, IV = 0 | III = 0, IV = 1 |
| The daily maximum rainfall (m) | $U_{31}$ | ≤V | VI | VII | ≥VIII |
| Weathering action | $U_{32}$ | no weathering | Weak weathering | Medium weathering | Strong weathering |
| The earthquake intensity | $U_{33}$ | ≤V | VI | VII | ≥VIII |
| Human engineering activity | $U_{34}$ | No influence | Weak influence | Medium influence | Strong influence |
The Construction Feature of Still Cliff \( (U_{12}) \)

The development of joint and fissure in the stiff cliff of rock mass and the existence of air surface result in the collapse. The weak interlayer under the gravity of upper rock mass leads to the occurrence of plastic deformation, tension fracture will take place in the upper of rock mass, final collapse will form.

The Geological Structure of Stiff Cliff \( (U_{13}) \)

The different geological structure have different influences on the development of collapse mass around the rock: (1) anticlinal nucleus: the fracture is inclined to take place in the rock stratum at the maximum curvature, plenty of tension fissures will form as the action of rock stratum, so the collapse mass is formed; (2) because rock mass become very fragile in the fault zone, this construction is benefit for the infiltration of groundwater, rock mass will be softened, so the collapse mass will be formed easily; (3) the landslide collapse will happen easily when the rock stratum at two flanks of fold represents monoclinic state, and the inclinations of rock stratum are parallel to ones of landslides, the slip surface is often the surface of rock stratum, dislocation plane or weak interlayer.

The Scale of Dangerous Rock Mass \( (U_{21}) \)

When the scale of rockfall mass becomes greater, the normal stress and shear stress in the corresponding main control plane becomes greater, and the collapse mass lose the stability easily, so the degree of harm become greater.

The Through Degree \( (U_{22}) \) and Inclination Angle \( (U_{23}) \) of Main Control Plane

The structure feature of collapse mass has important control action on the stability. When the main control plane of collapse mass is cut deeper, the pressure action of fissure water become bigger, so the collapse mass will become unstable more and more. The magnitude of collapse mass in the main control plane will influence the unstable mode of collapse mass, so the through degree and inclination angle of of main control plane are selected as the risk indices of collapse mass.

The Influences of Rainfall \( (U_{31}) \)

The large static and dynamic crevice water pressure can be formed easily in the rainstorm season, and the rock at the base of collapse mass will be softened by the infiltrated fissure water, the strengthen parameters of rock mass are reduced obviously, this will aggregate rapid deformation of collapse mass, even result in the destroy, so the rainfall is selected as a risk index.

The Weathering Action \( (U_{32}) \)

The rock mass in the stiff cliff become more and more fragile and loose because of the weathering action, it will result in the further development of fissures, so the weathering action is selected as a risk index.

The Earthquake Intensity \( (U_{33}) \)

The influential sizes of earthquake force on collapse mass in the stiff cliff are related with the weight of collapse mass and influential coefficients of earthquake force. When the earthquake acceleration exceeds critical acceleration, the crack of main control plane about the collapse mass will enlarge. The extended length is correlate with the energy and duration time of earthquake. When energy and duration time of collapse mass become bigger, the stability of collapse mass becomes the worse.

The Human Engineering Activity \( (U_{34}) \)

The human engineering activity is an important factor resulted in the formation and development of collapse mass. For example, manual blasting, unreasonable excavation and underground mining, etc, will influence the stability of collapse mass about dangerous rock. So the human engineering activity is selected as a risk index.

The AHP Theory

The Construction of Evaluation Hierarchy Diagram

Because 10 risk indices are selected in the paper, and their influences on the collapse of dangerous rock mass are non-linear, AHP method is adopted to estimate the weight coefficients of different indices. The collapse of dangerous rock mass are selected as target layer at first. Secondly, topographic and geological condition \( U_1 \), Geometry character of collapse mass \( U_2 \) and Other factors \( U_3 \) are selected as criterion layer. Thirdly, 10 risk indices are selected as sub-criterion layer. Their relations are plotted in Fig. 5 as follows:

The Construction of Weight Coefficients about Different Indices

The consistent checking formula is expressed as follows:

\[
CR = \frac{CI}{RI}
\]

...where, CR is the random consistent ratio of judgement matrix; CI is the consistent index of judgement matrix. When CR<0.1, it means that judgement matrix has good consistency, the distribution of weight coefficients is rational, otherwise the judgement matrix need be
adjusted until it meets with the consistency [20]. CI can be expressed as follows:

\[ CI = \frac{\lambda_{\text{max}} - n}{n - 1} \] (2)

...where, \( \lambda_{\text{max}} \) is the maximum characteristic root; \( n \) is the order number of judgement matrix; RI can be obtained in Table 3 for the low-order judgement matrix.

The Classification of the Assessment Index

The specific classification standards about the assessment index are shown in Table 1 and 2.

The Normal Cloud Theory

The cloud model theory is constructed on the idea of fuzzy sets theory [21] and probability concepts [22]. The cloud is defined as an uncertain transformation between some qualitative concepts and quantitative ones by using language value to react to uncertainty of knowledge concept for things or person in nature: the randomness and fuzziness. It constitutes the mapping between qualitative concepts and quantitative ones [23].

Let \( U \) be a quantitative universe including exact values, and \( M \) be the qualitative concept connected with \( U \). If the qualitative value \( x \) belongs to \( U \), and \( x \) is a random implementation by using the qualitative concept \( M \). The certainty degree \( \mu(x) \in [0, 1] \) of \( x \) relative to qualitative concept \( M \) is the random variable with steady tendency. When it can be expressed as:

\[ \mu : U \rightarrow [0, 1] \quad \forall x \in U \quad x \rightarrow \mu(x) \] (3)

...where, the distribution of \( x \) in qualitative domain \( U \) is called as the cloud, and every \( x \) is called as a cloud droplet, which is a tool as a quantitative meaning to describe a qualitative concept.

The digital characteristics of cloud is defined as the representative of the whole about the concept of cloud based on the normal cloud and membership function distributions [24]. The distribution of \( x \) can be determined by three numerical eigenvalues \((E_x, E_n, H)_x\); \( E_x \) is an expectation value in the universe of discourse and the best characterization of a qualitative concept [25]. \( E_n \) is the entropy of \( E_x \); it represents variation range of a cloud droplet in the distribution; \( H \) represents the measure of uncertain degree for entropy, namely, it is the entropy of entropy. Their eigenvalue in the normal cloud distribution can be shown in Fig. 6.

The transformation process from qualification to quantification is called as the positive cloud generator; On the contrary, it is called as the negative cloud generator [26]. Only the forward cloud generator in

| \( n \) | 1  | 2  | 3  | 4  | 5  | 6  | 7  | 8  | 9  | 10 | 11 | 12 |
|-------|----|----|----|----|----|----|----|----|----|----|----|----|
| RI    | 0  | 0  | 0.58 | 0.9 | 1.12 | 1.24 | 1.32 | 1.14 | 1.45 | 1.49 | 1.52 | 1.54 |
the paper is used, the backward cloud generator is omitted. Its transformation process is listed as follows: at first, the suitable cloud droplet is generated by using $CG \rightarrow N^3 (E_x, E_n, H_e)$, the production process of cloud drops represents the uncertainty of conversion between qualitative concept and quantitative values. Then $n$ cloud droplets are composed of cloud, so the qualitative concept is transformed as quantitative expression by using the uncertainty of cloud model, its process is shown in Fig. 7.

Its specific algorithm in Fig. 7 is listed as follows:

1. Expectation $E_x$ and the standard deviation $H_e$ are respectively calculated;
2. The normal random number $E_n^*$ is generated according to the characteristic value $(E_x, E_n, H_e)$ of cloud, its expected value is $E_n$, and the standard deviation is $H_n$.
3. A normal random number $x_i$ is generated, its expected value is $E_x$, and the standard deviation is $E_n$; $x_i$ represents a quantitative value of a qualitative concept.
4. The certainty degree $\mu$ of the qualitative concept $C$ is expressed according to procedure (1) , (2) and (3).

Results and Discussion

The Establishment of Risk Assessment Model

Based on the AHP theory, according to Eqs. (1) and (2), the weight coefficients of each assessment index can be shown in Table 4 as follows:

To establish the normal cloud model, ten assessment indices in Table 1 are selected; These assessment indices are all forward assessment indices. When the magnitude of these indices increases, the risk level of rockfall hazards becomes higher, these indices are defined as forward assessment index, otherwise, they are defined as negative index. In total, ten dangerous rock masses are selected as the assessment object. Their assessment indices are shown in Table 5.

To react the randomness and fuzziness, the risk assessment model about rockfall hazards is established, its assessment procedure is shown as follows:

1. The index and evaluation sets are respectively set up to assess the risk level of rockfall hazards in Shaoying Yan at first. In the paper, the datum in Table 5 is regarded as the index sets $U = \{u_1, u_2, ..., u_n\}$; Table 1 and 2 are evaluation sets $V = \{v_1, v_2, ..., v_m\}$.
2. The fuzzy matrix $M$ is established. The upper and lower boundary of index $j$ corresponding to index $i$ are respectively $x_i$ and $x_n$, then the qualitative concept $E_x$ of index $j$'s level can be depicted as:

$$E_x = (x_i + x_n) / 2$$

Entrophy has certain fuzziness for the risk assessment of rockfall hazards, so it is assumed as:

$$E_n = (x_u - x_l) / 6$$

And the hyperentropy $H_e$ is a constant which reacts the dispersion degree of the cloud model. Where, the hyperentropy $H_e$ is set as 0.01.

Apart from that, if a variable has only a single boundary, like $[-\infty, x_u]$ or $[x_l, +\infty]$, its default boundary parameters can be determined by the value of the upper or lower bounds as follows:

$$E_x = 1.5x_l$$

$$E_n = E_x / 6$$

(3) The final mean degree of membership $T_{ij}$ is determined according to the assessment indices of rockfall hazards.

$$T_{ij} = \frac{\sum_{m=1}^{n} t_{ij}^m}{N}$$

...where, $t_{ij}^m$ is the value that generated from the forward generator in $mth$ time; $N$ is calculative time.

(4) The comprehensive certainty degree of the risk grades about rockfall hazards can be depicted as:

$$P_j(n) = \sum_{i} \omega_i M_{ij}$$
...where, $P_j(n)$ is the comprehensive certainty degree of corresponding level $j$ of the dangerous rock mass $n$; $\omega_i$ is the weight coefficient of $i$th assessment index of the $n$th dangerous rock mass. And $M_{ij}$ is the $j$th level’s average certainty degree of the $i$th assessment index.

Finally, the risk assessment grade of the dangerous rock mass $n$ can be obtained as follows:

$$L = \max (P_1, P_2, P_3, P_4)$$

The risk level of rockfall hazards is determined by the maximum synthetic certainty degree, and the distributions of the certainty degrees about each evaluation index in the four risk levels are depicted in Fig. 8. In Fig. 8, the abscissa is the value of each assessment index, and the ordinate is the corresponding value of the certainty degree.

Table 5. The data about dangerous rock mass to assess.

| Assessment index | $U_{11}$ | $U_{12}$ | $U_{13}$ | $U_{21}$ | $U_{22}$ | $U_{23}$ | $U_{31}$ | $U_{32}$ | $U_{33}$ | $U_{34}$ |
|------------------|---------|---------|---------|---------|---------|---------|---------|---------|---------|---------|
| Dangerous rock mass $W_1$ | 98 | Inclined slope weak structural plane | Anticline rock stratum | 16.3 | 65 | 0.61 | 231.1 | Medium weathering | VI | Strong influence |
| Dangerous rock mass $W_2$ | 100 | Inclined slope weak structural plane | Anticline rock stratum | 3.8 | 65 | 0.69 | 231.1 | Medium weathering | VI | Strong influence |
| Dangerous rock mass $W_3$ | 98 | Inclined slope weak structural plane | Anticline rock stratum | 10 | 62 | 0.72 | 231.1 | Medium weathering | VI | Strong influence |
| Dangerous rock mass $W_4$ | 72 | Horizontal weak structural plane | Bedding rock stratum | 4.75 | 84 | 0.57 | 231.1 | Medium weathering | VI | Strong influence |
| Dangerous rock mass $W_5$ | 81 | Horizontal weak structural plane | Bedding rock stratum | 9.3 | 86 | 0.63 | 231.1 | Medium weathering | VI | Strong influence |
| Dangerous rock mass $W_6$ | 45.9 | Horizontal weak structural plane | Bedding rock stratum | 1.14 | 74 | 0.70 | 231.1 | Strong weathering | VI | Strong influence |
| Dangerous rock mass $W_7$ | 64.8 | Inclined slope weak structural plane | Bedding rock stratum | 2.75 | 78 | 0.65 | 231.1 | Strong weathering | VI | Strong influence |
| Dangerous rock mass $W_8$ | 53.4 | Inclined slope weak structural plane | Bedding rock stratum | 3.10 | 76 | 0.66 | 231.1 | Medium weathering | VI | Strong influence |
| Dangerous rock mass $W_9$ | 29.1 | Horizontal weak structural plane | Bedding rock stratum | 2.33 | 79 | 0.65 | 231.1 | Strong weathering | VI | Medium influence |
| Dangerous rock mass $W_{10}$ | 44.7 | Horizontal weak structural plane | Bedding rock stratum | 7.72 | 80 | 0.63 | 231.1 | Strong weathering | VI | Medium influence |

Risk Level Assessment

According to Table 1, and in combination with Eqs (4), (5), (6) and (7), the classification standard of normal cloud about rockfall hazards can be depicted in Table 6.

Table 6. The classification standard of normal cloud about rockfall hazards.

| Assessment index | The risk assessment of rockfall hazards |
|------------------|---------------------------------------|
|                  | I  | II | III | IV |
| $U_{11}$         | (7.5, 2.5, 0.01) | (32.5, 5.833, 0.01) | (75, 8.333, 0.01) | (150, 25, 0.01) |
| $U_{12}$         | (0.5, 0.167, 0.01) | (5.5, 1.5, 0.01) | (55, 15, 0.01) | (150, 25, 0.01) |
| $U_{13}$         | (22.5, 7.5, 0.01) | (52.5, 2.5, 0.01) | (67.5, 2.5, 0.01) | (82.5, 2.5, 0.01) |
| $U_{21}$         | (0.125, 0.042, 0.01) | (0.375, 0.042, 0.01) | (0.625, 0.042, 0.01) | (0.875, 0.042, 0.01) |
| $U_{31}$         | (5.1, 1667, 0.01) | (17.5, 2.5, 0.01) | (37.5, 4.167, 0.01) | (75, 12.5, 0.01) |
are compared in this paper [27]. The evaluation results calculated by different methods are shown in Table 7.

It can be found from Table 7 that the rest dangerous rock masses all belong to level III except W₂ and W₃ according to actual investigation on site, the accurate rate about the AHP-Normal cloud model arrives at 100\%, it is higher than 80\% of AHP-Fuzzy methods. The conclusions are drawn that it is feasible to estimate the risk level of rockfall hazards by using the normal cloud model. And the results from the AHP-Fuzzy methods agree well with those from normal cloud model and the actual Investigation on site except for sample W₃ and W₂ (Table 7). However, from the analysis of the monitoring values for W₃ (Table 5), two indices belong to level 2, four indices belong to level 3, and two indices belong to level 4, so it is more rational to specify W₃ as level 3 than level 2 or 4. This results

Fig. 8. Cloud of each assessment index generated by the forward cloud generator: a) The height of cliff stiff U₁₁, b) The scale of dangerous rock mass U₂₁, c) Inclination angle of of main control plane U₂₂, d) The through degree U₂₃, f) rainfall U₃₁.
demonstrate that the AHP-Normal cloud model is edible for predicting the risk level of rockfall hazards; According to Table 7, measured values of $U_{22}$ is closer to the classification boundary separating levels 3 and 4 for Sample W8, and measured values of $U_{12}$, $U_{13}$ and $U_{32}$ are near the mean value of corresponding classification standard of level 3 (Tables 5 and 7). The classification rating according to index values of $U_{22}$ might be deterministic based on the corresponding theory, whereas the classification rating based on $U_{12}$, $U_{13}$ and $U_{32}$ indices might be relatively uncertain when the influence of the index weight was not considered. Therefore, it was better to specify rockfall hazards of sample W8 as level 3. And it can be found in Table 7 that the certainty degrees of the scale of dangerous rock mass obtained by the normal cloud generator are $\mu_1 = 0.00$, $\mu_2 = 0.1675$, $\mu_3 = 0.2283$ and $\mu_4 = 0.006$ for sample W8. Therefore, the certainty degree by quantitative analysis is $\mu_3 > \mu_2 > \mu_1 > \mu_4$, and the the scale of dangerous rock mass $U_{22}$ of W8 only belongs to level III, and almost impossibly belongs to levels I, II, and IV. Furthermore, the level of dangerous rock mass W8 is more likely to be level III than that of W1, W2, W3, W4, W5, W6, W7, W8 and W9 because the certainty degree for level II of dangerous rock mass W8 (0.559) is higher than that of W1 (0.364), W2 (0.323), W3 (0.2), W4 (0.345), W5 (0.259), W6 (0.414) and W9 (0.462). In a word, the results based on the normal cloud model not only predict the risk level of rockfall hazards accurately, but also further determine the risk ranking of rockfall hazards for different dangerous rock mass at the same level.

**Conclusions**

Considering topographic and geological condition, geometry character of dangerous rock mass, as well as other factors, a new multi-index evaluation method is introduced in this paper to assess the risk level of rockfall hazards in Laoying Yan based on the AHP-Normal cloud model. Required cloud drops are generated based on three numerical characteristics calculated by cloud generator algorithm. The weight coefficients of different indices were obtained by using AHP weighting method, the risk level of rockfall hazards is determined by using the comprehensive degree.

The AHP-Normal cloud model is applied to perform the risk assessment of rockfall hazards in Laoying Yan. The results indicates that the rest dangerous rock masses all belong to level III except W2 and W3 according to actual investigation on site, the accurate rate about the AHP-Normal cloud model arrives at 100%, it is higher than 80% of AHP-Fuzzy methods. In other words, dangerous rock mass W2 and W3 are medium dangerous, other dangerous rock masses are high dangerous. To prevent a possible occurrence of rockfall hazards, necessary measures should be taken. In a word, not only the risk level of rockfall hazards are assessed accurately by the AHP-Normal cloud model, but also the risk ranking of rockfall hazards for different dangerous rock masses at the same level is determined. So a new method and thought is provided for the risk level assessment of rockfall hazards in the future.

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Conflicts of Interest

The authors declare that they have no conflicts of interest.

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