The impact of coarse aggregate shape on the behavior of self-compacting high-performance concrete

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Abstract. Self-Compacting High Performance Concrete (SCHPC) presents a crucial step in the development of concrete technology. The most important features of self-consolidating concrete are flowability, segregation resistance and passing ability. Generally, the rheological properties are modified by effective superplasticisers and water to binder ratio. The aim of this study is to focus on the important aspect of the impact of shape of the coarse aggregate on fresh concrete mixture properties, strength and deformability of SCHPC. Coarse aggregate is a significant proportion of the concrete volume and therefore has a meaningful influence on its quality. By appropriate selection of the shape of the grain aggregate, it is possible to affect the rheological parameters of concrete. The results presented in this study indicated that the shape of the grains of coarse aggregate has an impact on the strength and stiffness of SCHPC. Moreover, the occurrence of irregular grains of coarse aggregate causes lower slump flow and higher plastic viscosity in comparison to concrete mixtures with regular grains only. The research presented in this article is part of the author's wider research devoted to this issue.

1 Introduction

In recent years, an increase of the interest in the use of self-compacting concrete (SCC) in civil engineering has been noticed. This technology is still developing and being considered for a wider range of applications and properties of concrete [1]. The ability to consolidate under its own weight of concrete mixture without any means of compaction is a characteristic of SCC. Due to using small size aggregate and flowability of concrete mixture, SCC spreads fluently around reinforcement [2–5]. It is worth noting that nowadays cement paste content and water to binder ratio are the most significant parameters of the mix design which influences rheological parameters and strength of concrete. Nevertheless mixed design methods are still being developed [6–8].

The quality of the aggregate is determined by grain composition and grain shape, which significantly affects its strength. The content of irregular grains in mineral aggregates depends mainly on the method of crushing the raw material, i.e. on the type of grinding equipment used, their construction and operating parameters, the degree of fragmentation and the number of crushing stages [9]. Bearing this in mind, the purpose of this study was to appraise the effect of coarse aggregate shape on the properties of SCHPC.

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2 Materials and methods

Details of the concrete mixtures used in the study were presented in Table 1. The constituent of concrete mixture were as follows: Portland-fly ash Cement type I - CEM I 52.5R (Góraźdże, Poland), fine sand $\phi$ 0–2 mm (Brzegi, Poland), separated granite $\phi$ 4–8 mm (Kamienna Góra, Poland), SikaFume additive, superplasticiser Sika ViscoCrete-20 HE, 0.28 water/binder ratio.

Table 1. Proportions of concrete mixture ingredients.

| Mix type | Cement [kg/m³] | Fine aggregate [kg/m³] | Coarse aggregate-regular grains [kg/m³] | Coarse aggregate-irregular grains [kg/m³] | Sika Fume [kg/m³] | Water [kg/m³] | Superplasticiser [kg/m³] |
|----------|----------------|------------------------|----------------------------------------|------------------------------------------|-------------------|---------------|-------------------------|
| C        | 550            | 850                    | 475                                    | 475                                      | 66                | 173           | 19.25                   |
| E        | 550            | 850                    | 950                                    | -                                        | 66                | 173           | 19.25                   |

A jaw crusher was used to prepare the coarse aggregate (Fig. 1). The feed material which was subjected to a process of shredding was granite ($\phi$ 20–150 mm). In order to receive regular and irregular grains, resulting product was correctly separated by using slotted sieves (Fig. 2) in accordance with [10].

The following narrow size fractions were extracted: 4–5 mm, 5–6.3 mm and 6.3–8 mm. This fractions were separated by using sieves with spacing between bars 2.5 mm, 3.15 mm, 4 mm, respectively. The upper products on the slotted sieve are regular grains, the lower - irregular grains (Fig. 3).

Fig. 1. Laboratory jaw crusher.  
Fig. 2. Slotted sieves.  
Fig. 3. Regular (left) and irregular grains (right) of coarse aggregate.
The study used grain sizes of $\phi$ 4–8 mm only. The bulk density of coarse aggregate is $\rho_a = 2.64$ g/cm$^3$. Sieve analysis of coarse aggregate is shown in Table 2.

| Granulation  | Regular coarse aggregate [%] | Irregular coarse aggregate [%] |
|--------------|------------------------------|--------------------------------|
| 4 – 5        | 27.23                        | 42.35                          |
| 5 – 6.3      | 26.37                        | 32.60                          |
| 6.3 – 8.0    | 46.40                        | 25.05                          |

The only variable in the conducted research was the shape of the coarse aggregate. A total of 24 cylindrical specimens (118 mm in height and 59 mm in outer diameter) were manufactured in a laboratory and tested under uniaxial compression. With each type of concrete mixtures were made twelve specimens; four of them were tested after 3 (C1-C4, E1-E4), 7 (C5-C8, E5-E8) and 28 days. The concrete specimens were ripened in a water bath with a temperature of $20^\circ$C. In this article selected research for two concretes (C,E), tested after 3 and 7 days were shown.

The characteristics of fresh concrete mixtures were tested by using a slump flow test, in line with [11]. Plastic viscosity was determined in the moment to reach a diameter of 500 mm by flowing concrete mixture. The density of concrete was determined by dividing the mass by the volume of specimens. The compression tests were performed using a 3,000 kN capacity walter+bai ag Testing Machine with the constant axial strain rate of the samples in all of the experiments being approximately $3 \times 10^{-5}$[s$^{-1}$], at air temperature of $20^\circ$C and humidity of 60%. The measurement of the axial force was carried out by means of a force transducer, whereas the displacements were estimated by extensometer. Axial and radial displacements were determined through the measurement of the whole specimens’ dimension changes, where the extensometer were mounted directly between compression plates. Arrangement of extensometers on the specimens is shown at Figure 4.

![Arrangement of extensometers on the specimens.](image)

### 3. Selected test results and discussion

Figure 5 presents rheological parameters of concrete mixtures. It should be noted that concrete mixture with regular grains only (E), has higher slump flow than mixture with 50% irregular grains content (C). Moreover, plastic viscosity is higher when using both
regular and irregular grains in concrete mixtures. Confirmation of the correct execution of concrete mixtures is the lack of leakage of cement paste in all concrete mixtures. What is more, separation of the components did not occur. During the flow of the mixtures, proper venting was observed.

Fig. 5. Slump Flow test for C (A) and E (B) concrete mixture.

Research indicates the important role of the coarse aggregate shape on the mechanical properties of concretes (Table 3). Average compressive strength of concrete C in comparison to concrete E is higher by 14.4% and 23.9% after 3 and 7 days, respectively. Elasticity of concrete - the Young modulus (E) was determined without preloading cycles in a range of stress ranging from 15% to 33% value of maximum stress, therefore results are qualitative [12]. The use of aggregate with irregular grains leads to higher stiffness of concrete. Average Young modulus in case concrete C is higher by 26.3% and 16.8% after 3 and 7 days in collation to concrete E.

**Table 3.** Selected physical and mechanical properties of tested concretes.

| Mix type | Density of concrete [kg/m³] | Average compressive strength at 3 days $\bar{R}_c$ [MPa] | Average compressive strength at 7 days $\bar{R}_c$ [MPa] | Average Young modulus at 3 days $\bar{E}$ [GPa] | Average Young modulus at 7 days $\bar{E}$ [GPa] |
|----------|-----------------------------|------------------------------------------------------|------------------------------------------------------|---------------------------------------------|---------------------------------------------|
| C        | 2396                        | 68.63                                                | 83.08                                                | 18.30                                      | 20.82                                      |
| E        | 2398                        | 59.97                                                | 67.05                                                | 14.49                                      | 17.82                                      |

Detailed results for each sample are shown in Table 4. It is worth pointing out that concrete C has lower values of standard deviations in case of compressive strength and Young modulus in comparison to concrete E, where we can observe a greater heterogeneity of results (Fig. 6).

Figure 7 presents typical failure modes of concrete specimens. Due to the method of testing (constant axial strain rate) and to stop the compression test after reaching the maximum compressive strength, sudden destruction of the samples was not observed.
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| Mix | Density of concrete [kg/m³] | Average compressive strength at 3 days R_c [MPa] | Average compressive strength at 7 days R_c [MPa] | Average Young modulus at 3 days E [GPa] | Average Young modulus at 7 days E [GPa] |
|-----|-----------------------------|-----------------------------------------------|-----------------------------------------------|---------------------------------------|---------------------------------------|
| C   | 2396                        | 68.63                                        | 83.08                                         | 18.30                                 | 20.82                                 |
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Table 4. Summary of the experimental results.

| Group of specimens | Specimen | Ultimate load Pmax [kN] | Nominal compressive strength R_c [MPa] | Standard deviation of compressive strength σ [MPa] | Axial strain during fracture ε [%] | Modulus of elasticity E [GPa] | Standard deviation of Young modulus σ [GPa] |
|--------------------|----------|-------------------------|---------------------------------------|-----------------------------------------------|---------------------------------|-------------------------------|---------------------------------------------|
| C                  | C1       | 168                     | 62.29                                 | 7.19                                          | 3.18                            | 17.30                         | 0.76                                        |
|                    | C2       | 178                     | 66.26                                 |                                               | 4.50                            | 19.00                         |                                             |
|                    | C3       | 180.1                   | 67.01                                 |                                               | 4.02                            | 18.78                         |                                             |
|                    | C4       | 212.2                   | 78.95                                 |                                               | 4.56                            | 18.14                         |                                             |
|                    | C5       | 220                     | 81.90                                 | 2.68                                          | 4.42                            | 18.87                         | 1.70                                        |
|                    | C6       | 215                     | 79.99                                 |                                               | 4.17                            | 21.23                         |                                             |
|                    | C7       | 231.3                   | 86.10                                 |                                               | 4.18                            | 22.92                         |                                             |
|                    | C8       | 226.5                   | 84.32                                 |                                               | 4.38                            | 20.27                         |                                             |
| E                  | E1       | 130                     | 48.53                                 | 8.38                                          | 4.00                            | 13.94                         | 3.23                                        |
|                    | E2       | 168                     | 62.72                                 |                                               | 4.52                            | 14.15                         |                                             |
|                    | E3       | 184                     | 68.46                                 |                                               | 3.61                            | 18.83                         |                                             |
|                    | E4       | 162.3                   | 60.18                                 |                                               | 4.85                            | 11.02                         |                                             |
|                    | E5       | 225.3                   | 83.87                                 | 12.09                                         | 4.01                            | 21.96                         | 2.82                                        |
|                    | E6       | 182.2                   | 67.83                                 |                                               | 3.97                            | 16.40                         |                                             |
|                    | E7       | 156                     | 58.07                                 |                                               | 3.51                            | 17.16                         |                                             |
|                    | E8       | 157                     | 58.44                                 |                                               | 3.40                            | 15.75                         |                                             |

Fig. 6. Axial stress–strain relationships for tested concretes: a) C, b) E.

Fig. 7. Typical failure modes for tested concretes C and E.
4 Summary and perspectives

This paper shows an experimental investigation of the influence of coarse aggregate shape on the properties of self-compacting high-performance concrete. The following conclusions may be drawn from the work presented in this work:
1) The shape of the grains of coarse aggregate has a significant impact to strength and stiffness of self-compacting, high-performance concretes.
2) Additive of irregular grains of coarse aggregate may cause better filling of grain skeleton. The sharper edges of this grains may cause stress concentrations, what could lead to increased compressive strength.
3) Flowability of concrete mixtures depends on shape of coarse aggregate.
4) Appropriate selection of the shape of the grains of coarse aggregate allow us to affect the rheological parameters of concrete.

The future work will focus on assessing the effect of coarse aggregate shape on the structure of pores, hardness and microstructure of modified SCHPC. It could be helpful to recognize this phenomenon in details.

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