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Reliability of spring recession curve analysis as a function of the temporal resolution of the monitoring dataset

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Abstract
Mountain springs represent one of the largest and most precious sources of potable water in Italy, necessary to meet the water needs of the population. Optimizing the present and future management strategies of mountain groundwater resources has become increasingly necessary. The accuracy and frequency of the flow rate (Q) measurements determine and restrict the processes that can be studied using spring hydrograph and recession curve analysis. Therefore, to properly define mountain aquifers’ hydrogeological properties, it turns out important to highlight the variation of the error in the estimation of the hydrogeological parameters as the time interval of sampling varies. In this paper, recession curve analysis was performed on two different mountain springs (Spring 1 and Spring 2) of north-western Italy, firstly considering available 4-h resolution measuring data and subsequently by resampling data to simulate longer sampling intervals of 1, 3, 7, 15, and 30 days. The resulting distribution of errors introduced by longer acquisition intervals underlined how the percentage error increases with increasing acquisition interval. For obtaining an adequate estimation of mountain aquifer hydrodynamic parameters, in place of continuous hourly data, 1-day and 3-day sampling intervals with associated errors respectively lower than 5% and 10% were found to be valid.

Keywords Spring hydrograph · Recession curve · Mountain spring · Groundwater management · Groundwater monitoring

Introduction

Global water demand has been increasing worldwide by about 1% per year since the 1980s, driven by a combination of population growth, socio-economic development, changing consumption patterns and it will continue to grow significantly over the foreseeable future. At the same time, the global water cycle has intensified due to climate change, urbanisation, deforestation and increasingly intensive agricultural practices (Unesco 2019).

In Italy, 84.3% of the nation’s clean water derives from groundwater, including 48.0% from wells and 36.3% from springs, while 15.6% is derived from surface waters and the remaining 0.1% from marine water. Springs, therefore, represent one of the country’s largest and most precious sources of water, necessary to meet the water needs of the population (Istat 2018).

During recent decades, different hydrological issues, including the gradual drying up of many springs, low discharge rates during dry months and formerly perennial springs that have become seasonal, have been reported by authors across the Italian Alps and mountain Apennines areas. These trends have been found to be linked both to the overexploitation of groundwater resources and to climate change (Cambi and Dragoni 2000; Fiorillo et al. 2007; Gattinoni and Francani 2010).

Protecting and optimizing the present and future management of mountain groundwater resources, understanding their recharging system from both geological and hydrogeological perspectives, has become increasingly necessary for formulating adequate resource management strategies (Lo Russo et al. 2015).

A large number of methodologies have been developed over decades to derive hydrogeological information about mountain springs’ recharging systems. In the early 1900s, Boussinesq (1877, 1904), and Maillot (1905) developed analytical models to quantitatively investigate spring recession
mechanisms and estimate the hydrological properties of aquifers, establishing different mathematical relationships between spring discharge rates (\( Q \)) and time (\( t \)). Such methods were used over time to determine the potential of storage and exploitation of underground water resources (Nathan and McMahon 1990; Sugiyama 1996; Shevenell 1996; Dewandel et al. 2010).

Besides, autocorrelation and cross-correlation methods were recently elaborated and applied to spring monitoring datasets by different authors: in their works, Desmarais and Rojstaczer 2001; Fiorillo and Doglioni 2010; Galleani et al. 2011, Lo Russo et al. 2014, Banzato et al. 2017 showed how drainage models and springs’ vulnerability can be evaluated by analysing datasets of discharge (\( Q \)), precipitation (\( P \)), temperature (\( T \)) and electrical conductivity (EC). However, such statistical methods require to be applied strictly to continuous and multi-year recorded time series of data of P, Q, T and EC parameters. This aspect currently represents a key limiting factor for many practical hydrogeological spring investigations with short execution times (Kresic and Bonacci 2010).

As also reported by Tobin and Schwartz (2016), in remote alpine areas, continuous monitoring and data collection of springs’ hydrogeological parameters are often hampered by technical and logistical problems.

For these reasons, hydrograph analysis is still one of the most common and effective ways to assess the properties of a spring aquifer, such as the type and quantity of groundwater reserves (Dewandel et al. 2010; Fiorillo et al. 2012; Vashisht and Bam 2013; Cervi et al. 2014; Giacopetti et al. 2016; Jakada et al. 2019).

Nevertheless, a correct estimate of the hydrogeological parameters of the aquifer cannot be separated from the use of discharge (\( Q \)) values, recorded with high frequency and accuracy.

In particular, as the constancy in flow rate (\( Q \)) measurements determines and restrict the processes that can be studied through recession curve methods, it turns out to be important to know the error’s extent in the quantitative estimation of the hydrogeological parameters as the time interval of sampling varies.

Over time, the topic related to the analysis of appropriate time intervals for the analysis of the aquifer recession mechanisms has been discussed in the literature (Rupp and Selker 2006; Roques et al. 2017). In these studies, the attention turned out to be mainly focused on the type and quality of the results that can be obtained by applying available simplified analytical solutions to recession curves. Differently, the main aim of this study is to identify the minimum time interval in the recording of the spring discharge parameter that can be considered valid for a correct estimation of aquifer groundwater reserves. This information could be usefully considered by geologists in the planning of sampling campaigns in mountain environments, such as the one described, places where it is still frequently not possible to install probes for the continuous recording of spring hydrogeological parameters.

In this paper, complete recession curve analysis using Boussinesq’s (1877, 1904) and Maillet’s (1905) analytical solutions were conducted on two different porous springs (Spring 1 and Spring 2), located in the Italian Western Alps.

Recession curve analysis were firstly performed using the 4 hourly Q-measurement data with annual duration, available for both the selected case studies. Subsequently, the same analysis was carried out on different datasets with sampling intervals of 1, 3, 7, 15 and 30 days, generated from the original 4 hourly Q-measurement data.

The error in estimating the hydrogeological parameters of the groundwater volume stored in the system at the beginning and the end of the recession process (\( W_p \), \( W_d \)) and the coefficient of discharge (\( \alpha \)) were statistically analysed to understand which acquisition interval can be considered adequate to perform acceptable quantitative estimates of aquifer’s parameters.

### Methods

#### Case studies

Water reserves in mountainous areas are essentially located in extensive and thick sedimentary non-cohesive bodies (debris, landslide and glacial deposits) and/or in large volumes of intensely fractured and loosened rocks, involved in deep-seated gravitational slope deformation (DSBSD) areas (Banzato et al. 2015; Ostermann et al. 2016; De Luca et al. 2019).

The selected case studies are represented by two different mountain springs (Spring 1 and Spring 2), located in the alpine region of north-western Italy. From a geological point of view, Spring 1 and 2 are supplied by porous aquifers set in debris deposits, placed over impermeable metamorphic rocks (Figs. 1 and 2). The undeformed bedrock is characterized by lithological complexes of the tectonostratigraphic successions of the Italian Austroalpine Domain (Polino et al. 2002). The overlying debris deposits consist of sediments composed of fractured heterometric block, with dimensions from centimetre to plurimetres with chaotic disposition, mixed with variable quantities of a fine silty-sandy matrix. These deposits are associated to accumulations with very variable shapes and sizes, depending on the local morphology of the slope.

Due to the high permeability values, the described deposits turn to be very favourable to the storage of large quantities of water. The water infiltrated in the subsoil is forced to slow down its flow at the interface between the heavily
porous debris deposits until the impermeable bedrock. The appearance of springs is probably correlated to this permeability limit.

Spring 1 and Spring 2 were selected as, although they are mountain sources for which it is often difficult to find data, by means of their available hourly dataset was possible to make different evaluations by modifying the sampling intervals. Moreover, despite their similar geological structure, discharge values (l/s) of Spring 1 and Spring 2, recorded continuously by monitoring systems with an acquisition interval of four hours during 2017, show very different variation trends (Figs. 3 and 4).

According to the estimated Variability Index IV (Meinzer 1927) of 0.98, the discharge rate of Spring 1 can be considered sub-variable. The maximum recorded discharge rate is 7.8 l/s, while the minimum discharge rate value turns to be 2.7 l/s.

The discharge rate of Spring 2 is variable (Variability Index of 2.30), with a maximum recorded discharge rate that exceeds 30 l/s and a minimum discharge rate of 1.6 l/s.

Based on the available monitoring data (Fig. 3 and 4), both Spring 1 Spring 2 do not appear to have a rapid response to meteoric events or a direct correlation with rainy events.

**Hydrograph analysis**

The final graphical result of the various processes that govern the transformation of precipitation and other water inputs in the drainage area into the single output at the spring is called the spring hydrograph (Figs. 3 and 4).

Schematically, spring hydrographs are defined by a rising limb (AP), a flood peak (P) and a falling limb or recession curve (PC) (Fig. 5). The rising limb characterises the first part of a hydrograph, where an increase in discharge ($Q$) is recorded and very different responses can occur in relation to countless hydrogeological factors.

On the opposite, the falling limb or recession curve is characterized by a gradual decrease in discharge. It extends from the discharge peak to the base of the next rising limb and corresponds to a period with no significant precipitation.

By analysing the recession curve, it is generally possible to identify an initial section where the decrease rate is more marked called decrease curve, followed by a more slowly decreasing flow, called depletion curve.

The decrease curve corresponds specifically to the decrease in spring discharge that results from the persistence of the short circuit infiltration phenomenon, during which the unsaturated zone of the aquifer is still unaffected.

The gradual decrease in discharge in the depletion curve is instead related to the emptying of the saturated zone of the system in an unaffected regime. Some authors used to utilize the term infiltration or quick-flow curve to identify the decrease curve, while the depletion curve was sometimes called slow flow, exhaustion or the base flow curve (Civita 2008).

The analysis of spring discharge hydrographs is still nowadays one of the most suitable tools for the study of mountain springs and the definition of aquifer characteristics, such as the type and quantity of its groundwater reserves.

Over the decades, many authors have worked on recession curve modelling, establishing different mathematical relationships between spring discharge ($Q$) and time ($t$) that allowed to estimate the hydrological significance of the
discharge function parameters and the hydrological properties of aquifers (Amit et al. 2002).

Boussinesq (1904) and Maillet (1905) proposed two analytical formulas that described the dependence of the flow rate at a specified time ($Q_t$) on the flow at the beginning of the recession ($Q_0$), making it possible to correctly calculate the volume of water discharged over time (Kresic 2007).

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**Fig. 3** Monitoring data for the Spring 1-test site (2017). Comparison between discharge rate trend (in blue) and rain trend (in grey). Automatic acquisition interval of 4 h

**Fig. 4** Monitoring data for the Spring 2-test site (2017). Comparison between discharge rate trend (in blue) and rain trend (in grey). Automatic acquisition interval of 4 h
In detail, Boussinesq (1904) developed an exact analytical solution of the diffusion equation that describes flow through a porous medium, by considering the simplifying assumptions of a porous, free, homogeneous and isotropic aquifer, with the aquifer being limited by an impermeable horizontal layer at the level of the outlet (Eq. 1):

$$Q_t = \frac{Q_0}{(1 + \alpha(t - t_0))^2}$$

where $t$ [d] is the time since the beginning of recession for which the flow rate is calculated; $t_0$ [d] is the time at the beginning of recession usually (but not necessarily) set equal to 0, $Q_t$ [m$^3$s$^{-1}$] is the flow rate value at $t \neq t_0$; $Q_0$ [m$^3$s$^{-1}$] is the flow rate at the beginning of recession $t = t_0$; $\alpha$ [d$^{-1}$] is a constant, called recession coefficient, depending on the aquifer hydraulic systems as aquifer transmissivity, storage coefficient and the catchment area (Eq. 2).

$$\alpha = \frac{\sqrt{Q_0} - \sqrt{Q_t}}{\sqrt{Q_t}}$$

Maillet (1905) showed instead that the recession of a spring can be represented by an exponential formula, implying a linear relationship between the hydraulic head and flow rate (Eq. 3):

$$Q_t = Q_0 e^{-\alpha(t-t_0)}$$

where the recession coefficient ($\alpha$) can be determined through the following equation (Eq. 4).

$$\alpha = \frac{\log Q_0 - \log Q_t}{t(t_0)}$$

Both in Boussinesq (1904) and Maillet (1905), the recession coefficient equations were used to determine important hydrogeological parameters: $W_0$, the groundwater volume stored above the spring level at the end of the recharging season (beginning of the recession) and $W_d$, the groundwater volume stored at the end recession period (Table 1).

The analysis of mathematical models elaborated by Boussinesq (1904) and Maillet (1905), leads to understanding how correct estimates of the aquifers hydrogeological parameters cannot be obtained without using discharge ($Q$) values recorded with a certain frequency and accuracy. However, for springs located in mountain environments, such as analysed springs, it is often not possible to perform the continuous recording of the hydrogeological parameters.

For this reason, in this work, a double calculation approach has been applied on both analysed spring (Spring 1 and Spring 2). Hydrogeological parameters ($\alpha$, $W_0$ and $W_d$) computations, by means of Boussinesq (1904) and Maillet (1905) methods, were firstly performed considering the original 4 h sampling resolution of the recorded dataset (Figs. 3 and 4). Subsequently, the two series with 4-h sampling resolution were resampled to simulate acquisition intervals equal to 1, 3, 7, 15 and 30 days (see Fig. 6). For each acquisition interval, a different number of datasets were created starting from the original series and by moving forward time, to obtain all the possible datasets of a fixed sampling interval. In total, the resampling produced 168 30-day sampling datasets, 84 15-day sampling datasets, 42 7-day sampling datasets, 18 3-day sampling datasets and 6 1-day sampling datasets.

Table 1 Boussinesq (1904) and Maillet (1905) equations to determine the groundwater volume stored at the beginning of recession period ($W_0$) and the groundwater volume stored at the end of the recession period ($W_d$)

|       | Boussinesq (1904) | Maillet (1905) |
|-------|------------------|----------------|
|       | $W_0 = \frac{Q_0}{\alpha(t+\alpha t)} \times 86400$ (5) | $W_0 = \frac{Q_0}{\alpha} \times 86400$ (7) |
|       | $W_d = \left[ \frac{Q_0}{\alpha} - \frac{Q_t}{\alpha(t+\alpha t)} \right] \times 86400$ (6) | $W_d = \left( \frac{Q_0 - Q_t}{\alpha} \right) \times 86400$ (8) |

Where:

$Q_t$ [m$^3$s$^{-1}$] is the flow rate value at $t \neq t_0$;

$Q_0$ [m$^3$s$^{-1}$] is the flow rate at $t = t_0$;

$\alpha$ [d$^{-1}$] is the recession coefficient.
sampling datasets. A comparison of these different datasets is represented in Fig. 6.

The validity of the tested analytical methods for estimating aquifer hydrogeological parameters ($\alpha$, $W_o$, $W_d$) also in case studies characterised by non-continuous monitoring was then evaluated by comparing the values obtained using the 4-h dataset with the ones obtained using the 1, 3, 7, 15, 30 days sampling resolution datasets.

Distributions of estimated percentage errors, calculated with respect to the original dataset (4 h), were then plotted with the aim to identify the minimum acquisition time interval that can be considered adequate to formulate acceptable estimates of aquifer reserves.

**Results**

**Recession analysis—original dataset**

For each case study (Spring 1 and Spring 2), complete recession curves (i.e., no missing data) were generated using $Q$-values recorded with an acquisition interval of 4 hours (Figs. 6 and 7).

The results of the recession analysis, obtained by applying Boussinesq (1904) and Maillet (1905) methods on the 4-h sampling resolution dataset, are reported in Table 2.

By comparing the results obtained for Spring 1 by the two methods, the $W_o$ values turn out to be fairly divergent (196,304 m³ Boussinesq method and 129,696 m³ Maillet method), while the two methods provided similar values for
Comparisons between the recorded discharge rate data and predicted rate data are presented in Fig. 7. The estimated root-mean-square error (RMSE) results were equal to 0.35 and 0.27 for the Boussinesq and the Maillet equation, respectively.

In contrast, the $W_0$ values obtained for Spring 2 by applying the two selected different methods appear comparable (117,070 m$^3$ Boussinesq method and 117,767 m$^3$ Maillet method), while the values provided for $W_d$ turn to be divergent (84,071 m$^3$ Boussinesq method and 108,410 m$^3$ Maillet method).

As for Spring 1, comparisons between the recorded discharge rate data and the predicted rates of Spring 2 are presented in Figs. 8, to understand which methods better fit the analysed recession curve.

For Spring 2, the root-mean-square error (RMSE) results were equal to 3.52 for the Boussinesq equation and 0.84 for the Maillet one.

Recession analysis—simulated datasets

Hydrogeological parameters ($\alpha$, $W_0$, $W_d$), obtained using the original dataset (4-h sampling resolution), were subsequently compared with the ones estimated by considering simulated datasets (resampled to 1, 3, 7, 15, and 30 days sampling resolutions).

$W_0$ (81,184 m$^3$ Boussinesq method and 85,092 m$^3$ Maillet method).

Comparisons between the recorded discharge rate data and predicted rate data are presented in Fig. 7.

The estimated root-mean-square error (RMSE) results were equal to 0.35 and 0.27 for the Boussinesq and the Maillet equation, respectively.
The distributions of estimated errors introduced from a less dense dataset with respect to the original dataset are reported in Figs. 9, 10 and 11. Table 3 summarizes the minimum and maximum error values obtained for each analysed case.

Fig. 9 Distributions of estimated errors of the calculated coefficient of discharge with respect the value obtained with the original dataset (4-h sampling resolution). The errors are represented using class intervals with a range equal to 5%.

Fig. 10 Distribution of estimated errors in the calculated groundwater volume stored at the beginning of recession (W₀) with respect to the value obtained using the original dataset (4-h sampling interval). The errors are represented using class intervals with a range equal to 5%.
Fig. 11  Distribution of estimated errors in the calculated groundwater volume stored at the end of the recession ($W_d$) with respect to the value obtained from the original dataset (4-h sampling interval). The errors are represented using class intervals with a range equal to 5%.

Table 3  Range of errors for recession coefficient ($\alpha$), groundwater volume stored in the system at the beginning of the recession process ($W_0$), and groundwater volume stored at the end of the recession process ($W_d$) estimated using monitoring data resampled to different sampling intervals for the two monitored springs.

| Case study       | Error range [%] | $W_0$ | $W_d$ | $W_0$ | $W_d$ | $W_0$ | $W_d$ |
|------------------|-----------------|-------|-------|-------|-------|-------|-------|
| SPRING 1         |                 |       |       |       |       |       |       |
| Boussinesq formula | Min             | Max   | Min   | Max   | Min   | Max   |
| 1-day sampling   | 0.6             | 4.6   | 0.4   | 4.2   | 0.2   | 1.8   |
| 3-day sampling   | 0.6             | 10.6  | 0.4   | 9.7   | 0.2   | 4.9   |
| 7-day sampling   | 0.3             | 15.8  | 0.1   | 15.7  | 0.0   | 6.4   |
| 15-day sampling  | 2.6             | 21.5  | 2.3   | 22.3  | 0.0   | 12.1  |
| 30-day sampling  | 3.4             | 27.2  | 0.3   | 28.2  | 0.0   | 15.9  |
| Maillet Formula  | Min             | Max   | Min   | Max   | Min   | Max   |
| 1-day sampling   | 0.6             | 3.8   | 0.3   | 3.3   | 0.2   | 1.5   |
| 3-day sampling   | 0.6             | 8.4   | 0.3   | 7.6   | 0.2   | 4.4   |
| 7-day sampling   | 0.2             | 13.2  | 0.2   | 12.3  | 0.0   | 5.4   |
| 15-day sampling  | 0.3             | 19.0  | 0.3   | 18.6  | 0.0   | 11.0  |
| 30-day sampling  | 0.4             | 22.7  | 0.0   | 22.0  | 0.0   | 22.3  |
| SPRING 2         |                 |       |       |       |       |       |       |
| Boussinesq Formula | Min             | Max   | Min   | Max   | Min   | Max   |
| 1-day sampling   | 0.6             | 6.3   | 0.2   | 5.2   | 0.5   | 3.2   |
| 3-day sampling   | 2.9             | 16.3  | 0.2   | 12.2  | 0.2   | 8.3   |
| 7-day sampling   | 0.2             | 28.5  | 0.2   | 19.4  | 0.0   | 16.7  |
| 15-day sampling  | 0.2             | 27.5  | 0.4   | 26.4  | 0.0   | 32.5  |
| 30-day sampling  | 0.3             | 43.9  | 0.6   | 32.5  | 0.0   | 39.7  |
| Maillet Formula  | Min             | Max   | Min   | Max   | Min   | Max   |
| 1-day sampling   | 0.2             | 3.5   | 0.1   | 3.0   | 0.8   | 3.2   |
| 3-day sampling   | 0.1             | 10.0  | 0.6   | 9.4   | 0.2   | 10.5  |
| 7-day sampling   | 0.2             | 19.5  | 0.1   | 17.5  | 0.1   | 18.8  |
| 15-day sampling  | 0.1             | 22.5  | 0.1   | 34.3  | 0.1   | 37.0  |
| 30-day sampling  | 0.1             | 29.6  | 0.1   | 41.5  | 0.1   | 46.1  |
By considering the resampled datasets with the shortest sampling interval (1 day), the difference in recession coefficient $\alpha$ values (Fig. 9) varies between 0 and 5% for Maillet formula; between 0 and 10% if the Boussinesq formula is considered for the analysis of Spring 2.

The error increases to values of 10% (Maillet method) and 16.3% (Boussinesq method) by considering the 3-day sampling interval datasets.

Maximum errors of 29.6% and 43.9% were evaluated for the Maillet and Boussinesq methods, respectively, using the 30-day sampling resolution datasets.

The second analysed parameter was the groundwater volume stored at the beginning of recession, $W_0$ (Fig. 10).

Considering the 1-day resolution resampled datasets, the difference in $W_0$ values compared to the original dataset varied between 0 and 5% using the Maillet equation; between 0 and 10% for the Boussinesq equation.

The maximum error increases to 9.4% (Maillet method) and 12.2% (Boussinesq method) when considering the 3-day resolution resampled datasets. The error values continue to grow when analysing longer sampling intervals: the 7-day sampling interval datasets reach maximum errors of 17.5% (Maillet method) and 19.4% (Boussinesq method), while the 30-day datasets reach 41.5% (Maillet method) and 32.5% (Boussinesq method).

Finally, for $W_d$ parameter (Fig. 11), the analysis of the 1-day sampling resolution datasets yielded an error range of 0–5% for both the Maillet and Boussinesq equations. The error increases to maximum values of 10.5% (Maillet method) and 8.3% (Boussinesq method) when considering 3-day datasets.

Analysing longer sampling intervals, the maximum error value continues to increase to 18.8% (Maillet method) and 16.7% (Boussinesq method) for the 7-day datasets and 46.1% (Maillet method) and 39.7% (Boussinesq method) for the 30-day datasets.

Conclusions

Due to climate change, urbanisation, and deforestation challenges, the development of correct management strategies for the exploitation of groundwater resources associated with mountain springs has become an increasingly important topic.

However, especially in remote alpine areas, continuous monitoring and data collection of springs’ hydrogeological parameters is still often hampered by technical and logistical problems.

In many cases, the only type of data available for the derivation of geological and hydrogeological information is represented by non-continuous datasets of springs’ discharge.

In this work, to identify the minimum time interval, in the recording of the spring discharge parameter, that can be considered valid for a correct estimation of aquifer groundwater reserves, recession curve analysis was carried out on two different mountain springs (Spring 1 and Spring 2).

Recession curve analysis was performed by applying Boussinesq (1904) and Maillet (1905) analytical solutions, firstly considering the available 4-h resolution $Q$ datasets and subsequently by resampling those data to simulate longer sampling intervals of 1, 3, 7, 15, and 30 days.

The first outcome of this study highlighted how for these springs the Maillet exponential equation should be preferred to the Boussinesq equation, for both porous analysed springs: while the Maillet method fitted the entire recession and could provide correct estimates of the aquifer’s parameters, the Boussinesq solution underestimated in both cases the dynamic volume of the aquifer.

The outcomes obtained from the analysis of the errors’ distribution classes also allowed to underline that the percentage errors in the estimates of aquifer parameters ($W_0$, $W_d$ and $\alpha$) increases with the time sampling interval of the flow rate ($Q$).

The estimated percentage errors have a greater range of variation and reach the highest value in Spring 2, which is characterised by a variable discharge, than in Spring 1, associated with a sub-variable discharge behaviour.

For both analysed springs (Spring 1 and Spring 2), the ranges of error associated with the 1-day and 3-day sampling interval datasets (daily or bi-weekly datasets) were, respectively, less than 5 and 10%.

In the absence of continuous datasets, these sampling resolutions can therefore be considered valid for an adequate estimation of the hydrodynamic parameters of a porous aquifer ($\alpha$, $W_0$ and $W_d$).

In contrast, a sampling interval of 7 days recorded a range of error between 5 and 20%. This type of sampling dataset, although it is frequently used in spring monitoring operations, turns out to be not always acceptable. Due to the maximum error close to 20% its use is suggested only for qualitative estimates of available water reserves or an analysis of different error scenarios should be performed.

Finally, due to the percentages of errors that can exceed 30%, 15- and 30-day sampling intervals cannot be considered an appropriate dataset for estimating aquifer hydrodynamic parameters and porous spring groundwater resources.
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Compliance with ethical standards

Competing interests The authors declare that they have no competing interests.

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