Upper Mississippi River Flow and Sediment Characteristics and Their Effect on a Harbor Siltation Case

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Upper Mississippi River Flow and Sediment Characteristics and Their Effect on a Harbor Siltation Case

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Abstract

Upper Mississippi River flow and sediment characteristics downstream of St. Louis, MO are presented in this study. Available and measured data were used to assess a harbor siltation case and dredging needs. Such data are also useful to researchers and engineers conducting work in the Mississippi River, and large rivers in general. Flows were characterized in terms of the mean annual hydrograph, the flow duration curve and the mean annual, dominant and effective discharges. Suspended and bed material sediments were characterized by grain size distributions (GSDs). Suspended sediment concentrations were characterized with a sediment-rating curve, a mean annual sediment-graph and a duration curve. The results of the analyses were used to assess harbor sedimentation by comparing GSDs of harbor bed samples with those observed in the river. Bathymetric surveys were used to determine rates and occurrence of sedimentation. The analyses showed that harbor siltation correlates with river conditions, and is driven by wash load in the river, which enters the harbor in suspension and deposits along the bottom due to the lack of flow-through velocities high enough to keep the fine sediments in suspension.
Key words
Upper Mississippi River, Wash Load, Harbor, Siltation, Dominant discharge, Effective discharge

Introduction
Sediment characteristics and loads in the Mississippi River have been the subject of numerous studies in the past decades. Their relation with land loss (e.g. Kesel 1989, 1988; van Heerden and DeRouen 1997) has been one of the key factors driving the need to better assess the sediment loads in the river and its tributaries. Some recent studies on sediment load trends in the river basin (e.g. Horowitz 2010; Meade and Moody 2010; Blevins 2006) suggest that sediment loads are declining. In spite of this, potential for building river diversions that would carry sediment to certain locations along the shoreline to prevent further land loss at the Mississippi River delta, and along coastal Louisiana, has been recognized (e.g. Paola et al. 2011; Allison and Meselhe 2010). The amount of sediment diverted is a function of the flow and sediment load in the river (Dutta et al. 2017) and proper quantification of both variables is required. As a result, research has mainly focused on the Missouri and Ohio Rivers, which are responsible for the largest tributary sediment loads (Heimann et al. 2011), or on the lower Mississippi River sediment loads (e.g. Thorne et al. 2015). This study presents a characterization of the flow and sediment in the Upper Mississippi River at St. Louis, MO, to contribute to such efforts and facilitate future water and sediment diversions. In addition, the analysis and results are used to assess a siltation problem at a harbor built in 2006 on the right bank of the Upper Mississippi River close to Ste. Genevieve, MO.

Siltation is the process by which fine sediment particles suspended in a water body settle and deposit on the bed. Harbor siltation is a common problem throughout the world, and different sediment management strategies have been proposed in the literature to prevent it or slow it down and therefore reduce the necessity for dredging (e.g. Kirby 2011; Winterwerp 2005; van Schijndel
Successful implementations of such strategies have been well documented (e.g. Kuijper et al. 2005; Winterwerp et al. 1994) but in spite of these, not all harbors, and especially not all riverine harbors, are designed with the potential consequences of siltation in mind.

Two distinct foci were established to assess the siltation problem which began a few months after the harbor started operating in 2007, namely, (i) the harbor itself and (ii) the Upper Mississippi River between St. Louis, MO and Chester, IL. Specific tasks involved the following:

- Harbor sediment sample collection and analysis. Samples were taken to the Ven Te Chow Hydrosystems Laboratory at the University of Illinois at Urbana-Champaign for their analysis.
- Determination of siltation volumes and average siltation rates from harbor bathymetric surveys provided by the harbor’s owners.
- Characterization of the Upper Mississippi River flow conditions using data from neighboring United States Geological Survey (USGS) gaging stations. The data are available through the National Water Information System (USGS 2017).
- Characterization of the material in suspension and deposited on the bed in the Upper Mississippi River. These data are also available through the National Water Information System (USGS 2017).

These tasks set the structure of this paper. It is divided into five sections, with this introductory section being the first. The second section describes the harbor, its geometry and location, and presents measured siltation rates and patterns and bed sediment characteristics. The third section focuses on the characteristics of the flow and sediment in the reach of the Upper Mississippi River in the vicinity of the harbor. The fourth section presents the key findings from the tasks enumerated above, and provides answers regarding the following questions. (i) What is
the source of the sediment responsible for siltation inside the harbor? (ii) When are these sediments most likely to be deposited? (iii) How does siltation relate to the hydraulic conditions in the river? The fifth and last section summarizes the conclusions of the analysis.

Site Characteristics

Harbor Location and Dimensions

The harbor is located between Upper Mississippi River Miles 138 and 139 as shown in Fig. 1. The site is approximately 65 km downstream of St. Louis, MO and 45 km upstream of Chester, IL. With a design maximum depth of 17 m, the harbor is 500 m long and 200 m wide. The actual depth and volume of water in the harbor varies with river stage.

Fig 1. Harbor Location. Figure prepared by the authors based on Navigation Chart No. 135 for the Upper Mississippi River (US Army Corps of Engineers 2011).
**Measured Harbor Siltation Rates and Patterns**

Siltation volumes inside the harbor are available for different dates due to bathymetric measurements conducted by the dredging company that was hired by the harbor’s owners. Table 1 shows the siltation volumes measured between February of 2008 and June of 2010. The number of days between surveys and the harbor’s maximum surface area (10,000 m²) were used to compute mean siltation rates, equivalent deposit thicknesses and daily siltation depths. Siltation periods (dredging campaigns) are reflected by an increase (a decrease) in the excess volume of sediment reported in Table 1. This volume corresponds to the difference between a given bathymetric survey and the harbor design geometry.

**Table 1.** Siltation volumes in the harbor and corresponding mean siltation rates for the period between February 2008 and June 2010.

| Date      | Excess Volume of Sediment [m³] | Volume Increase [m³] | Time [days] | Mean Daily Siltation Rate [m³/day] | Equivalent Deposit Thickness [m] | Avg. Daily Siltation Depth [cm/m²/day] |
|-----------|--------------------------------|---------------------|-------------|-----------------------------------|---------------------------------|---------------------------------------|
| FEB 28 2008 | 166,444                         | -                   | -           | -                                 | -                               | -                                     |
| AUG 13 2008 | 363,540                          | 197,095             | 167         | 1,180                             | 3.6                             | 1.2                                   |
| OCT 04 2008 | 337,617                          | 0                   | 52          | 0                                 | 3.4                             | 0.0                                   |
| JAN 26 2009 | 52,699                           | 0                   | 114         | 0                                 | 0.5                             | 0.0                                   |
| MAR 10 2009 | 57,271                           | 4,572               | 43          | 106                               | 0.6                             | 0.1                                   |
| JUN 04 2009 | 159,829                          | 102,557             | 86          | 1,193                             | 1.6                             | 1.2                                   |
| JUL 28 2009 | 211,052                          | 51,224              | 54          | 949                               | 2.1                             | 1.0                                   |
| DIC 16 2009 | 199,168                          | 0                   | 141         | 0                                 | 2.0                             | 0.0                                   |
| FEB 10 2010 | 207,347                          | 8,178               | 56          | 146                               | 2.1                             | 0.2                                   |
| JUN 02 2010 | 299,332                          | 91,986              | 112         | 821                               | 3.0                             | 0.8                                   |

1 Mean siltation rate determined by dividing the volume increase between consecutive surveys by the number of days between them.
2 Equivalent deposit thickness computed by dividing the excess volume of sediment by the total harbor area (500 m by 200 m - 10,000m²) assuming it is uniformly distributed.
3 Average daily siltation depth is computed by dividing the mean daily siltation rate by the total harbor area.

The patterns of sediment deposition inside the harbor are shown in Fig. 2. The original harbor bathymetry is shown alongside bathymetric surveys conducted in January 26th, March 10th, June 04th and July 28th of 2009. A small insert is included to indicate the relative position of the harbor.
with respect to the Mississippi River and the direction of flow. An aerial image of the harbor, taken from Google Earth, is also included. The amount of sediment deposited on the bed of the harbor in January 26th, 2009 corresponds to material that had accumulated previously and was not removed during the dredging efforts conducted in the second semester of 2008 (see Table 1).

![Aerial Image of the Harbor](image)

**Fig. 2.** Sedimentation patterns inside the harbor based on original harbor bathymetry and bathymetric surveys at four different dates in 2009.

**Harbor Bed Sediment Characteristics**

**Grain Size Distributions**

Samples from the harbor bed were extracted during two separate campaigns conducted in December of 2010 and December of 2011. Fig. 3 shows the sampling locations from both campaigns, and Table 2 indicates the size of the samples. Grain size distribution analyses for samples 1 and 5 were conducted according to the “Standard Test Method for Particle-Size
Analysis of Soils” (ASTM 2002), referred to herein as the hydrometer method, and with the LISST-ST settling tube (Pedocchi and Garcia 2006) for comparison. Results from both methods compared well (Fernández et al. 2012) and therefore samples 1-9 were analyzed only with the LISST-ST for simplicity. Samples from the 2011 campaign and from a drum collected in 2010 were analyzed with the hydrometer method. To assess the role of flocculation in the harbor, the analyses were conducted on samples dispersed with sodium hexametaphosphate (NaPO₃)₆, as well as on non-dispersed samples. Results are shown in Fig. 4.

![Fig. 3. Sediment sampling locations. Samples 1-8 and the drum were extracted in December of 2010. Samples A, B and C were extracted in December of 2011.](image)

Table 2. Harbor bed sediment sample dates and volumes.

| Sample(s) | Dec. 2010 | Dec. 2011 |
|-----------|-----------|-----------|
| Drum      | 189 L     | 1.9 L     |
| A-C       | 3.8 L     |           |
Results from the grain size distribution analyses on dispersed and non-dispersed samples suggest that flocculation occurs inside the harbor. Flocculation may lead to enhanced siltation rates through faster settling of material. The settling velocity of the flocs depends on the size, density and shape, which in turn are governed by inter-particle collision frequency (Mehta and McAnally 2008). Krone (1962) suggested that since particle collision frequency depends on the concentration of particles in suspension, suspended particle concentration may be used as a surrogate to estimate the floc settling velocity. Wolanski et al. (1989) proposed dividing the settling process into four zones; empirical relations have been developed to express settling velocity in each zone as a function of suspension concentration. Table 3 shows the different zones; an empirical relation proposed by Hwang (1989) to estimate settling velocities in each zone is given by Eq. 1.

**Fig. 4.** Grain size distributions of harbor bed samples.
\[
w_s = \begin{cases} 
  w_{sf} & C < C_1 \\
  a_w \frac{C^{n_w}}{(C^2 + b_w^2)^{m_w}} & C_1 < C < C_2 \\
  \sim\text{negligible} & C_2 < C < C_3 \\
  \end{cases} \tag{Eq. 1}
\]

where

\[w_{sf} = \text{free settling velocity;}
\]

\[C = \text{volume suspension concentration;}
\]

\[a_w = \text{velocity scaling coefficient;}
\]

\[n_w = \text{flocculation settling exponent;}
\]

\[b_w = \text{hindered settling coefficient;}
\]

\[m_w = \text{hindered settling exponent;}
\]

\[C_1 - C_3 = \text{zone concentration limits as defined in Table 3;}
\]

**Table 3.** Settling process zones

| Zone 1 | Zone 2          | Zone 3         | Zone 4     |
|--------|----------------|---------------|------------|
| Free settling | Flocculation settling | Hindered Settling | Consolidation |
| $C < C_1$ | $C_1 < C < C_2$ | $C_2 < C < C_3$ | $C_3 < C$ |
| $w_s = w_{sf}$ | $w_s = w_s(C)$ | $w_s = w_s(C)$ | $w_s \to 0$ |

When the suspension concentration is below a value $C_1$, free settling occurs and the settling velocity corresponds with the one each particle would have in the absence of other particles. Particles are far away from each other and no flocculation occurs. As concentrations increase, flocculation begins to occur and therefore the original grains begin to form flocs which have higher settling velocities. This process of flocculation settling continues up to a concentration $C_2$, which corresponds with the maximum settling velocity ($w_{sm}$). Above $C_2$, the concentration becomes so high that the flocs have trouble settling and begin to collide with each other. Settling becomes hindered and could be thought of as a condition where water is trying to escape the pore space as sediment settles down. If concentration continues to increase and reaches a value $C_3$, the process turns into a consolidation process rather than a settling one. Zone concentration limits.
and coefficients are not universal and depend on the sediment type and grain size distribution, as well as the environmental conditions in which the settling process takes place, such as salinity and turbulence or the lack thereof.

The settling velocity of the material found in the harbor was determined by conducting the settling column experiments first described by McLaughlin (1959) and later improved by Ross (1988). Fig. 5 shows the settling column used. It is 0.10 m in diameter and 1.9 m high, and has 5 mm sampling tubes located at the following elevations above the bed: 0.06 m, 0.16 m, 0.31 m, 0.51 m, 0.72 m, 0.93 m, 1.13 m, 1.33 m and 1.54 m. The design of the column is based on the one developed by Lott (1987), and the experimental procedure followed the one described by Ross (1988). The following five different initial concentration conditions: $C_0 = 1$ g/L, 5 g/L, 10 g/L, 15 g/L, and 25 g/L were used. Results from the experiments are shown in Fig. 6 along with a curve fit with Eq. 1; the resulting coefficients are shown in Table 4.

Table 4. Coefficients used in Eq. 1 to fit the measured harbor sediment settling velocities.

| Coefficient | $a_w$ | $n_w$ | $b_w$ | $m_w$ | $C_2$ [g/L] | $w_{sm}$ [m/s] |
|-------------|-------|-------|-------|-------|-------------|---------------|
| Value       | 0.1   | 2.1   | 10    | 2.08  | 10.1        | 2.1e-4         |

Fig. 5. Picture taken during a settling column test. The left image shows the full column for a test with a high initial concentration. Note how the concentration varied between the top and the bottom of the settling column as shown in the right-most panes.
Fig. 6. Harbor bed sediment settling velocities.

Upper Mississippi River Characteristics in the Near-Harbor Area

Available Data

Data available at USGS gaging stations 07010000 at St. Louis, MO and 07020500 at Chester, IL were used to characterize the Mississippi River in the vicinity of the harbor. A summary of the data is shown in Table 5. Given that no significant tributaries flow into the Mississippi River between St. Louis, MO and Chester, IL a preliminary analysis showed that for the matching period of record July 1942 – November 2011 the flow conditions, on average, differ by less than 1% (Fernández et al. 2012). Therefore, all analyses related to river data presented hereafter only used the information recorded at St. Louis, MO.

Flow Discharge and Suspended Sediment Concentrations

Flow discharge and sediment concentrations in the Mississippi River at St. Louis, MO for the period October 1st, 1980 to September 30th, 2011 are shown in Fig. 7. Historic mean flows at St.
Louis, MO computed for different periods are shown in Table 6. Values reported therein indicate that the mean flow in the Mississippi River for the period beginning when the harbor started operation (2007) and ending in 2011 has been approximately 56% larger than what it had been over the period beginning in 1861 and ending in 2011 and 33% larger than what it had been over the period beginning in 1980 and ending in 2011.

A suspended sediment concentration rating curve determined from the 11,285 measurements available for the 30 year period is shown in Fig. 8. A power law curve shown in Eq. 2 and on the lower right of Fig. 8 was fit to the data (solid line) and envelopes indicating concentration values equal to 0.2, 0.5, 2.0 and 5.0 times the values estimated with the power curve fit to the data are indicated with dashed lines. Although the data shows scatter, 80.4% (99.3%) of the data lie inside the envelopes for 0.5-2.0 (0.2-5.0) times the value obtained with the power law relation.

\[ C = 1.022e^{-0.05Q^{1.1641}} \]  
_Eq. 2_

where

\[ C = \text{suspended sediment concentration [g/L]; and} \]

\[ Q = \text{flow discharge [m}^3/\text{s].} \]

**Table 5.** Summary of available data from USGS gaging stations at St. Louis, MO and Chester, IL that were used in the study.

| USGS 07010000 Mississippi River at St. Louis, MO | Begin Date | End Date |
|-----------------------------------------------|------------|----------|
| Daily Data                                    |            |          |
| Discharge                                     | 01/01/1861 | 09/30/2011|
| Suspended sediment                             | 10/01/1980 | 09/30/2011|
| Field/lab water-quality samples                | 01/31/1953 | 09/30/2011|

| USGS 07020500 Mississippi River at Chester, IL | Begin Date | End Date |
|-----------------------------------------------|------------|----------|
| Daily Data                                    |            |          |
| Discharge                                     | 07/01/1942 | 09/30/2011|
| Suspended sediment concentration               | 10/01/1982 | 09/30/2011|
| Field/lab water-quality samples                | 10/14/1970 | 09/30/2011|
Table 6. Historic mean flows for Mississippi River at St. Louis, MO. Periods indicated correspond to hydrologic years.

| Period       | [years] | Mean Flow [m³/s] |
|--------------|---------|-----------------|
| 1861-2011    |         | 5,265           |
| 1941-2011    | b       | 5,665           |
| 1981-2011    | c       | 6,175           |
| 2007-2011    | d       | 8,210           |

a Complete period of record for Mississippi River discharge at St. Louis, MO.
b Period of record matching the discharge data available at Chester, IL.
c Period of record matching the suspended sediment concentration measurements at St. Louis, MO
d Period of record beginning in the year when the harbor started operating (2007).

Fig. 7. Flow discharge and suspended sediment concentrations in the Mississippi River at St. Louis, MO for the period Oct. 01, 1980 to Sep. 30, 2011.

Suspended and Bed Material Sediment Characteristics

Grain size distributions for the sediments in the Mississippi River at St. Louis, MO were available as part of U S Geological Survey field/lab water quality samples (Table 5). Fig. 9 shows a total of 108 grain size distributions of the material traveling as suspended load and Fig. 10 shows a total of 114 grain size distributions for the material found on the bed of the Mississippi River at St. Louis, MO. The solid black line represents the median grain size distribution curve, and the dashed lines correspond to the 75th and 25th percentiles. The sediment size for which 50% of the grains are smaller is 0.008 mm for the material traveling as suspended load and 0.44mm for the material found on the bed of the river.
**Fig. 8.** Suspended sediment concentration rating curve for the Mississippi River at St. Louis, MO. The dashed lines indicate envelopes for values equal to 0.2, 0.5, 2.0, and 5.0 times the concentration values estimated with the power relation fit to the data shown in the lower right of the figure.

**Fig. 9.** Grain size distributions for the material in suspension in the Mississippi River at St. Louis, MO. The solid gray lines correspond to the 108 available measurements; the solid black line corresponds to the median grain size distribution and the dashed lines represent the 75th and 25th percentiles. The bulk $D_{50}$ for the material is 0.008 mm.
Fig. 10. Grain size distributions for the bed material in the Mississippi River at St. Louis, MO.

The solid gray lines correspond to the 114 available measurements; the solid black line corresponds to the median grain size distribution and the dashed lines represent the 75th and 25th percentiles. The bulk $D_{50}$ for the material is 0.44 mm.

**Mean Annual Hydrograph, and Suspended Sediment Concentrations and Duration Curves**

The mean annual flow hydrograph and mean annual sediment concentrations are shown in Fig. 11. A black dashed line spike can be seen in the sediment concentration hydrograph during late February. That line corresponds to the 30-year daily average concentrations but it is significantly biased by an extreme event that occurred in February of 1985, as shown in Table 7. If the values for those days are not included in the averaging process, the curve takes the shape of the solid line, which was taken as the representative mean annual sediment concentration curve herein. Fig. 12 shows the flow duration curve and the suspended sediment concentration duration curve based on the mean annual data in Fig. 11. Flows (concentrations) are lower than 5,000 m$^3$/s (0.27 g/L) for half of the year and higher than 8,000 m$^3$/s (0.43 g/L) for 30% of the year. The remaining 20% of the time covers the periods in which flows and suspended sediment concentrations increase (decrease) rapidly between mid-February and mid-March (mid-July and mid-August).
Fig. 11. Mean annual hydrograph and suspended sediment concentrations for Mississippi River at St. Louis, MO. The dashed black line indicates the mean annual concentration values when including the values observed in the period Feb. 22<sup>nd</sup> - 26<sup>th</sup>, 1985.

Table 7. Suspended sediment concentration values measured in 1985 and associated flow discharges.

| Date       | Flow Discharge Q [m<sup>3</sup>/s] | Suspended Sediment Concentration C [g/L]<sup>a</sup> |
|------------|-----------------------------------|------------------------------------------------------|
| Feb. 22 1985 | 6,343                             | 2.75                                                  |
| Feb. 23 1985 | 11,836                            | 5.74                                                  |
| Feb. 24 1985 | 15,348                            | 6.72                                                  |
| Feb. 25 1985 | 17,302                            | 5.69                                                  |
| Feb. 26 1985 | 18,632                            | 3.09                                                  |

<sup>a</sup> The 99<sup>th</sup> percentile for concentrations measured in the period Oct. 1<sup>st</sup> 1980 to Sep. 30<sup>th</sup> 2011 is 1.78 g/L. Within that time period, only 3 (12, 30) values exceeded 4g/L (3 g/L, 2.5 g/L) corresponding to 0.03% (0.11%, 0.27%) of the data.
Fig. 12. Mean annual flow (Q) duration curve and mean annual suspended sediment concentration (C) duration curve for the Mississippi River at St. Louis, MO.

**Characteristic Flow Discharges**

Different definitions of a constant characteristic discharge that would be capable of producing the same channel morphologies observed in a river under varying flow conditions have been proposed in the literature. Some of these definitions are related to channel equilibrium conditions (e.g. Inglis 1947), meander wavelengths (e.g. Ackers and Charlton 1970), stream bankfull geometry (e.g. Wilkerson and Parker 2011; Nixon 1959), exceedance probability (e.g. Blench 1956; Leopold and Maddock 1953) or sediment transport capabilities (e.g. Terrell and Borland 1958). These concepts are typically applied in relation to geomorphic processes and their effect on channel geometry.

In this study, the concept of characteristic discharges is adapted to assess the flows responsible for the sediment loads in the Upper Mississippi River. Specifically, the concepts of dominant and effective discharge are used due to their relation with sediment loads in the river without
consideration for morphological implications. The dominant discharge is defined here as the flow that, if sustained throughout a period of time, would produce the same mean sediment discharge observed during that period under varying flow conditions. The effective discharge is defined here as the one carrying the largest volume of sediment in the river. This definition is based on the bed-generative discharge concept first proposed by Schaffernak (1916, 1922), and its computation follows the approach described by Biedenharn et al. (2000). The method has been used and described by different authors (e.g. Garde and Ranga Raju 1977; Gandolfo 1940) but other authors refer to it as the dominant discharge (e.g. Thomas and Benson 1966). It is not the objective of this study to provide clarification and comparison between available definitions; the reader is referred to Soar and Thorne (2011) for a recent review on the subject.

Using the data available for the 1981-2011 period, the mean annual suspended sediment concentration was determined and the dominant discharge was back calculated with the sediment-rating curve shown in Fig. 8 and Eq. 2. The values obtained are 0.337 g/L for the mean concentration and 7,608 m³/s for the dominant discharge.

The effective discharge computation is shown in Fig. 13. The resulting value is 9,582 m³/s, which corresponds to the maximum value of the curve of weighted contributions (right panel) obtained from the product of the flow frequency curve (left panel) and the sediment rating curve (middle panel). Other local maxima may be seen in the curve. These represent the discharges responsible for carrying large sediment volumes. As is often the case, the result obtained has two distinctive peaks, indicating that a frequent discharge carrying a relatively small sediment load for a long time is almost as effective as an infrequent discharge carrying a large amount of sediment over a shorter period of time. Using the rating curve in Fig. 8, the suspended sediment concentration associated with the effective discharge was obtained. The resulting value was 0.441 g/L.
Fig. 13. Effective discharge analysis plots and results. The left panel shows the flow frequency curve; the middle panel shows the sediment rating curve; and the right panel shows the weighted contributions and effective discharge.

Key Findings and Discussion

What is the source of the sediment responsible for siltation inside the harbor?

Origin based on grain size distributions and sedimentation patterns

The sediment size analyses from the river and the harbor are summarized in Fig. 14; median $D_{50}$ values are shown in Table 8. Harbor bed sediments are slightly coarser than the material that is carried in suspension by the Upper Mississippi River at St. Louis, MO but are significantly finer than the material in the bed of the river, suggesting that the sediment source is likely to be the suspended sediment in the river. The sedimentation patterns inside the harbor also shed light on the origin of the sediment. As shown in Fig. 2, siltation blankets the entire bed of the harbor. The relatively uniform thickness of the deposited sediment observed in the March and June bathymetries is due to a combination of two factors: the fine-grained nature of the deposited sediment, and barge traffic (approximately 20 barges per day), which can under some conditions cause resuspension and redistribution due to propeller wash (Garcia et al. 1999). Although
coarser materials were found close to the entrance, all sediments were significantly finer than the Upper Mississippi River bed material.

**Fig. 14.** Comparison of Harbor and Upper Mississippi River grain size distributions.

**Table 8.** Median D$_{50}$ values for Harbor and Upper Mississippi River sediment grain size distributions.

| Sediment source | Median D$_{50}$ [mm] |
|-----------------|----------------------|
| Drum            | 0.008                |
| S1-9            | 0.017                |
| A-C             | 0.015                |
| River           |                      |
| Suspended       | 0.008                |
| Bed             | 0.440                |

**Suspended sediment dynamics within the harbor**

The sediment that enters the harbor in suspension is deposited first on the perimeter of the harbor where the flow velocities and shear stresses, even in the presence of barge traffic, approach zero. Sediment deposits preferentially along these zones and then builds up uniformly from the edges.
towards the middle of the harbor. The siltation patterns shown in Fig. 2 show some zones that are lower in elevation in the south section close to the entrance. These areas have likely been scoured due to barge traffic going in and out of the harbor.

The settling velocities determined in the experiments (Fig. 5) and shown in Fig. 6 range between 1e-6 to 1e-3 m/s, with the largest values associated with larger suspended sediment concentrations at which flocculation occurs. Although the concentrations in the Mississippi River rarely exceed 2g/L (Fig. 8), it is possible that concentrations may exceed this value inside the harbor as the sediment settles to the bottom. This is most likely to prevail during periods when the harbor is not operating at full capacity. The presence of a bar-like feature on the east side of the harbor on the July 29th bathymetry is also thought to be related to barge traffic redistribution of sediments, since most of the barge traffic occurs through the southern part of the harbor and towards the west and north west sections.

**When are the sediments most likely to be deposited in the harbor?**

Harbor siltation volumes and rates are shown in Fig. 15. The black solid line corresponds with the volumes of sediment above the design conditions of the harbor. The values are divided by 20 so as to plot this variable using the same axis limits as the flow discharge, and to clearly present the salient trends. In the three cases where the volume of sediment in the harbor increases, the period corresponds to late February or early March to late July or early August. (Decreases are caused almost solely by dredging.) This timeframe corresponds to the spring and early summer months; siltation rates within this period can be as high as 1.2 cm/m²/day, as indicated by the red dashed line.

**Applicability of the dominant and effective discharge concepts**

Typically, the dominant and effective discharge concepts are not meant to be used in rivers where the majority of the material transported corresponds to silt and clay sizes (i.e. wash load). The main reason for this is that wash load does not correlate with flow discharge and therefore, as long as the sediment is available, the river will transport it regardless of the flow magnitude. Fig.
14 shows that more than 80% of the material traveling in suspension in the Upper Mississippi River corresponds to wash load. However, Fig. 8 and Fig. 11 show that wash load in the Mississippi River, as defined using e.g. the 62.5 μm cutoff criterion (River Research Council, 2007), does indeed positively correlate with discharge to a surprising degree. The trends shown by both variables in Fig. 11 are remarkably similar, and more than 80% of the suspended sediment concentration data shown in Fig. 8 lies within envelopes of 0.5-2.0 times the value estimated with the sediment rating curve. A possible explanation for this behavior is given below.

**Fig. 15.** Flow and suspended sediment concentration conditions in the Upper Mississippi River at St. Louis, MO for hydrologic years 2008-2010, as well as harbor siltation volumes and rates.

During late February and early March, snowmelt takes place and river flows increase. At the same time, fine sediment from bare agricultural land is carried by runoff into the river and transported as wash load. This phenomenon is sustained throughout the growing season, and is enhanced
by rainfall in the spring and early summer. Once the crops are established and precipitation
diminishes (late summer), fine sediment availability is reduced and both the flows and suspended
sediment concentrations in the river return to their base flow patterns. The mean annual
hydrograph shown in Fig. 11 reflects these processes.

Snowmelt followed by spring and early summer precipitation contribute to the flow magnitude and
the availability of sediment due to bare agricultural land in the Upper Mississippi River basin, thus
creating conditions in which fine sediment availability matches the period of high flows. High flows
do not necessarily cause larger sediment transport, but are correlated due to the characteristics
of the river basin. The dominant and effective discharge concepts may be applied in this and other
river basins where sediment availability matches the period of high flows even though the relation
between the two variables is not strictly causal.

How does siltation relate to the hydraulic conditions in the river?

Table 9 summarizes the results obtained for the characteristic discharges, the number of days for
which they are exceeded and the associated suspended sediment concentrations. Comparison
of the characteristic discharges with the mean annual hydrograph and mean annual suspended
sediment concentrations shown in Fig. 11 suggest that the Mississippi River carries larger
sediment volumes between the end of February and early August than otherwise.

The dominant discharge is exceeded for 120 days between mid-March and mid-July, and the
effective discharge is exceeded only for a few days in April and all of May. Siltation volumes and
siltation rates are shown in Table 1 and Fig. 15; they are greatest in periods including these
months. Although bathymetric survey dates allow assessment of the silting process over the
period between February and August, lack of data for the months of April and May impede
determining if harbor siltation occurs mostly during early or late spring, summer or both.

Nonetheless, the process of siltation is clearly related to flow conditions in the river. The data
show that whenever suspended sediment concentrations at St. Louis, MO are above 0.44 g/L,
large siltation volumes inside the harbor are possible. According to Fig. 12, these concentrations are met during 30% of the year.

Table 9. Upper Mississippi River at St. Louis, MO characteristic discharges, exceedance and associated suspended sediment concentrations for hydrologic years 1981-2011.

| Discharge Type | Discharge Value Q [m³/s] | Exceedance [days - %] | Associated Suspended Sediment Concentration C [g/L] |
|----------------|--------------------------|-----------------------|---------------------------------------------------|
| Mean           | 6,170                    | 162 – 44%             | 0.264                                             |
| Dominant       | 7,608                    | 120 – 33%             | 0.337                                             |
| Effective      | 9,582                    | 36 – 10%              | 0.441                                             |

Potential Effect of Barge Traffic and Towboat Operations on Harbor Siltation

Studies on the effect of towboat navigation and barge tows under typical conditions of Upper Mississippi River traffic have shown that bed shear stresses under such conditions deviate from those expected under steady-uniform flow. More specifically, higher shear stresses are associated with the passage of the tow and the stern of the barge tow (Rodriguez et al. 2002; Garcia et al. 1999, 1998). Barge traffic in and out of the harbor plays an important role in sediment resuspension. The harbor is directly open to the Mississippi River, but has no through-flow discharge and thus acts as a sediment trap. Towboats and barges that enter for loading and unloading operations resuspend the sediment in the harbor, but even with the small settling velocities measured in the laboratory and reported in Fig. 6, such resuspension does not seem to contribute substantially toward keeping sediment from settling inside the harbor. As shown in Table 1 and Fig. 15, between the months of July and December of 2009, the excess volume of sediment in the harbor decreased and no dredging efforts took place. This suggests that in those months in which Upper Mississippi River flow discharge and suspended sediment concentrations return to base levels, sediment resuspended by towboats and barges may leave the harbor. This observed decrease, however, corresponds to only an insignificant amount of sediment compared to the amount that comes into the harbor during the spring and summer months.
Conclusions

Flow and sediments in the Upper Mississippi River were characterized with information available at the USGS gaging station in St. Louis, MO. The most relevant results of our analysis are as follows.

1. The correlation between wash load and flow discharge in the Upper Mississippi River is due to the characteristics of the basin, namely, snowmelt followed by spring and early summer precipitation over bare agricultural land that create conditions in which fine sediment availability matches the period of high flows.

2. The dominant and effective discharge concepts may be applied to the Upper Mississippi River and similar basins where these conditions are met.

3. The $D_{50}$ for the material carried in suspension by the Mississippi River at St. Louis, MO is 0.008 mm and for the material found on the bed it is 0.44 mm.

4. Settling velocities for the material carried in suspension by the Mississippi River in St. Louis, MO are between $1e^{-6}$ to $5e^{-4}$ m/s with the largest values associated with larger suspended sediment concentrations where flocculation is possible.

Comparison of the Upper Mississippi River data with laboratory results of harbor bed samples and bathymetric survey data leads to the following findings:

5. Sediment deposited in the harbor is wash load from the Upper Mississippi River that enters the harbor in suspension and deposits due to the lack of flow-through inside;

6. Towboat and barge operations resuspend sediment, but their effect on preventing siltation is negligible in spite of the small settling velocities;

7. Flow conditions in the Upper Mississippi River in the period between Mid-March and Mid-July correlate with high siltation rates inside the harbor; the analysis suggest (but in the absence of specific bathymetric data does not prove) that large siltation rates are possible in the month of May when the effective discharge in the Mississippi River is exceeded.
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