Evaluating suffusion in dam core soil of glacial till using empirical methods

Abstract

Glacial till, a broadly graded soil formed by the action of glaciation, is used in many parts of the world as dam core material. Statistically these show higher frequency of internal erosion incidents in dams than other types of core soils. One mechanism that may be influential is suffusion as caused by internal instability, which is one initiation mode of internal erosion. A compilation of 23 laboratory tests on suffusion of glacial till core soil has been compiled of which 13 performed unstable. An assessment of the potential for internal instability is made using five empirical methods that evaluates the shape of the particle size distribution. Evaluation of the database indicates on the methods’ applicability to glacial till gradations, and, furthermore, it reveals that the Kenney-Lau approach and its Li-Fannin adaptation provide predictions relatively more accurate in identifying unstable gradations due to suffusion. A stricter assessment is suggested by extending the Li-Fannin adaptation so that it incorporates a transition zone defined by the range 0.68≤H/F<1.0.

Keywords: dams, erosion, glacial soils, gravels, sands, silts

Notation

D  grain size (mm)  
F mass passing at grain size D (%)  
H mass increment between D and 4D (%)  
F_r finer fraction  
C_u coefficient of uniformity, C_u=D_60/D_10  
(H/F)_min stability index, defined by the smallest value of H/F, for 0 <F≤20 % in soil with a widely-graded coarse fraction  
FC#4 percentage fines content of base soil passing the No. 4 sieve (4.75mm)  
F#200 percentage fines content (< #200 sieve, 0.075mm) of full sample of soil  
Δm/m mass loss in seepage test (%)  
GBE global backward erosion

Introduction

Glacial till is used in many parts of the world as impervious core material in embankment dams. This type of soil has been found to be quite susceptible to internal erosion, especially if compared to other types of soils used as core materials in dams. Its susceptibility to suffusion, which stems from the erodibility of the soil itself, is still relatively unexplored. Its vulnerability to backward erosion and filter incompatibility, however, which heretofore perhaps considered its main culprit for internal erosion, has been shown in many laboratory tests. In terms of suffusion, although only a few studies are available, laboratory tests have shown that glacial till may be susceptible. Internal erosion is initiated by the mechanisms of concentrated leak erosion, backward erosion, contact erosion, and suffusion erosion. Backward erosion occurs at a free unfiltered surface or at the interface to an inadequate coarse filter.

It works itself progressively backward towards the source of water by the detachment of soil particles. Assisted by gravity, it can cause near-vertical pipes in the dam body, which is called global backward erosion (i.e., GBE). Suffusion, on the other hand, occurs inside of a fixed bulk volume of a core soil or dam zone, and due to internal instability of the gradation it erodes the finer fraction through the constrictions of its coarser fraction.

In the current study a database consisting of 23 glacial till gradations from laboratory tests are compiled and analysed. Two approaches are used to assess the potential internal instability and thus susceptibility to suffusion, namely the empirical methods of Kenney & Lau and its adaption by Li & Fannin and Burenkova and its adaptations by Wan & Fell. Glacial till soils are strictly speaking outside the extent of the methods, in general sand and gravel gradations were tested, so evaluation of the database herein indicates on the one hand the applicability of these methods. On the other, it will show that an improved assessment can be made by extending the Li-Fannin adaptation by incorporating a transition zone defined by the range 0.68≤H/F<1.0.

Experimental database of glacial till gradations

In Rönnqvist & Viklander five laboratory studies on the mechanism of suffusion of glacial till soils were reviewed (i.e., studies by Lafleur & Nguyen, Wan & Moffat et al, Hunter et al & Lilja et al). In these, there were 11 gradations of glacial till in total. In addition, Rönnqvist performed suffusion tests on 4 glacial tills from the core or borrow area of four Swedish dams and eight mixtures with varied gravel content based on these tills. The total compilation yields thus an experimental database that includes 23 gradations (Figure 1). Their grading characteristics are given in Table 1, which reveals an extent of fines content from 4%<F#200<38% and 6<C<600. 13 gradations were reportedly internally unstable according to laboratory tests (i.e., S7, S10, C-20, C-30, CG3, Mr3, Mr19, BE3, BE4, GR2, GR3, ST2 and ST3), exhibiting change in grading characteristics after...
the test or experiencing seepage- or pore water pressure variations indicative of instability during test. However, specimens CG3, GR2 and ST2 ultimately failed by GBE, and nine have potentially a finer fraction that exceeds 35% (i.e., gradations S17, S28, RD, FG1, Mr19, BE1, RA1, GR1, ST2, thus, possibly eliminating these from suffusion susceptibility. However, it is reasonable to suggest that the failure process of CG3 and GR2 were affected by suffusion given their potential for grading instability.

Figure 1 Compilation of 23 glacial till gradations used in laboratory suffusion tests.

Table 1 Grading characteristics of 23 glacial till gradations from laboratory tests

| Source            | Gradation | $D_{60}$ (mm) | Fines content, $F_{max}$ (%) | Finer fraction, $F_s$ (%) | $C = D_{60}/D_{10}$ | Performance     |
|-------------------|-----------|---------------|------------------------------|--------------------------|----------------------|-----------------|
| Lafleur & Nguyen  | S28       | 37.5          | 28 (36)                      | 38                       | 24                   | Stable          |
|                   | S17       | 37.5          | 17 (20)                      | 37                       | 13                   | Stable          |
|                   | S10       | 37.5          | 10 (12)                      | 33                       | 6                    | Unstable        |
|                   | S7        | 37.5          | 7 (8)                        | 25                       | 6                    | Unstable        |
| Wan               | RD        | 9.5           | 22 (23)                      | 45                       | 49                   | Stable          |
| Moffat et al.     | C-20      | 19            | 20 (26)                      | 20                       | 20                   | Unstable        |
|                   | C-30      | 19            | 30 (33)                      | 35                       | 30                   | Unstable        |
| Hunter et al.     | FG1       | 37.5          | 33 (43)                      | 40                       | 425                  | Stable          |
| Lilja et al       | CG3       | 53            | 17 (36)                      | 25                       | 147                  | Unstable (GBE)  |
|                   | Mr3       | 63            | 22 (29)                      | 32                       | 40                   | Unstable        |
|                   | Mr19      | 31.5          | 38 (41)                      | 38                       | 25                   | Unstable        |
### Analysis using empirical methods

In the internally unstable gradation, the finer fraction (i.e., \( F_f \)) under-fills the coarser fraction thus creating fixed coarser particles and mobile finer particles.\(^{14} \) However, unless \( F_f \leq 35\% \) of the total soil it is probably not susceptible to suffusion.\(^{15,16,23} \) An excessive \( F_f \) means the soil is matrix-supported and its coarser particles are floating in the finer fraction, thus eliminating suffusion. It can still be vulnerable to backward erosion. The finer fraction of a gradation is usually defined as the point of inflection of the particle size distribution.\(^{14} \) Results of the empirical analyses are summarised below.

#### The Kenney-Lau method and Li-Fannin adaptation

The potential for internal instability can be evaluated according to Kenney & Lau\(^{15,16} \) from a shape analysis over a designated portion of the finer end of the gradation curve, which for widely-graded materials (i.e., \( C_u \geq 3 \)) is defined by the evaluation range of \( F = 20\% \). Skempton & Brogan\(^{23} \) introduced the stability index (i.e., \( (H/F)_{\text{min}} \)) from the minimum value along the Kenney-Lau H:F shape curve. Li & Fannin\(^{17} \) adapted the method to incorporate the Kezdi\(^{24} \) split-gradation method. They found that the Kenney-Lau method become less conservative over the coarser end of the evaluation range \( (F=15\%-20\%) \) by shifting to the Kezdi criterion. Gradation analyses established the stability index, \( (H/F)_{\text{min}} \) of all 23 gradations (Figure 2) and the results are summarized in Table 2, which shows that the Kenney-Lau method determines:

I. 9 of the 13 unstable gradations (69%) have a potential for internal instability; and,

II. 56% of all gradations deemed potentially unstable performed unstable.

Thus, the Kenney-Lau method exhibits a 69% success rate in identifying the unstable gradations and 56% accuracy. However, if potentially irrelevant gradations are disregarded, i.e., gradations with \( F_f > 35\% \) or that failed by backward erosion (GBE), it leaves 12 gradations; the Kenney-Lau method then identifies 78% of the unstable gradations with 78% accuracy. The Li-Fannin adaptation provides comparably more accurate predictions by 88% accuracy (Table 2) when excluding potentially irrelevant gradations.

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**Table Continued...**

| Source | Gradation | \( D_{50} \) (mm) | Fines content, \( F_{500} \) (%) | Finer fraction, \( F_f \) (%) | \( C_u = D_{60}/D_{10} \) | Performance |
|--------|-----------|------------------|-------------------------------|-----------------------------|---------------------------|-------------|
| BE1    | 30        | 34 (40)          | 50                            | 31                          | Stable                    |
| BE2    | 25        | 22 (42)          | 32                            | 559                         | Stable                    |
| BE3    | 31        | 17 (42)          | 29                            | 600                         | Unstable                  |
| BE4    | 35        | 13 (41)          | 25                            | 363                         | Unstable                  |
| RA1    | 40        | 37 (45)          | 45                            | 85                          | Stable                    |
| GR1    | 43        | 30 (35)          | 40                            | 35                          | Stable                    |
| GR2    | 35        | 19 (36)          | 32                            | 556                         | Unstable (GBE) \(^{2} \) |
| GR3    | 30        | 10 (36)          | 20                            | 196                         | Unstable                  |
| GR4    | 35        | 18 (35)          | 31                            | 440                         | Stable                    |
| ST1    | 33        | 14 (17)          | 31                            | 10                          | Stable                    |
| ST2    | 34        | 10 (17)          | 40                            | 127                         | Unstable (GBE) \(^{2} \) |
| ST3    | 45        | 4 (22)           | 17                            | 73                          | Unstable                  |

I. Mass passing 0.075mm (full sample), brackets indicate FC#4 (regraded to #4 sieve, i.e., 4.75mm)

II. Point of inflection on the gradation curve

III. Specimens that failed by global backward erosion

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Figure 2 Summary of gradation analyses: the Kenney & Lau\textsuperscript{15,16} method and the Li & Fannin\textsuperscript{17} adaptation.

Table 2 Summary of gradation analyses of 23 glacial till gradations from laboratory tests

| Empirical criteria | Identified unstable gradations (%) | Unstable gradations not identified | Potentially unstable with unstable performance (%) |
|--------------------|-----------------------------------|-----------------------------------|--------------------------------------------------|
| Kenney-Lau method\textsuperscript{15,16} | 69 (78\%) | S7, S10, Mr19, ST2 | 56 (78\%) |
| Li-Fannin adaptation\textsuperscript{25} | 69 (78\%) | S7, S10, Mr19, ST2 | 69 (88\%) |
| Burenkova method\textsuperscript{18} (zone I or III) | 62 (67\%) | S7, S10, Mr3, CG3, ST2 | 62 (75\%) |
| Wan-Fell adaptation\textsuperscript{19} (>5\% probability) | 54 (44\%) | S7, S10, Mr3, Mr19, C-20, C-30 | 78 (67\%) |
| Wan-Fell\textsuperscript{20} alternative method (transition or unstable zone) | 42 (44\%) | S7, S10, Mr3, Mr19, C-20, C-30, ST2 | 71 (67\%) |
| Modified Li-Fannin adaptation (unstable zone)\textsuperscript{25} | 62 (67\%) | S7, S10, C-30, Mr19, ST2 | 80 (100\%) |

\textit{a)} Results when excluding gradations with finer fraction >35\% (i.e., S17, S28, RD, FG1, Mr19, BE1, RA1, GR1, and ST2) and gradations that failed by backward erosion (GBE) (i.e., GR2, ST2 and CG3)

\textit{b)} Gradations RD, FG and CG3 are not included because of insufficient gradation data

\textit{c)} Unstable = H/F<0.68, within evaluation range 0<F≤20 \%

The Burenkova method and Wan-Fell adaptation

Burenkova\textsuperscript{18} proposed that the potential for internal instability be evaluated from “conditional factors of uniformity” determined for the slope of the coarser fraction by $D_{90}/D_{60}$ and for the overall slope of the gradation curve by $D_{90}/D_{15}$. Subsequently Wan & Fell\textsuperscript{19} adapted the method for widely graded soils with limited clay and plasticity to incorporate a probabilistic approach. Gradation analyses of all 23 gradations (Figure 3) and the results are summarized in Table 2, which shows that the Burenkova method determines:

I. 8 of the 13 unstable gradations (62\%) locate in Zones I or III and have a potential for internal instability; and,

II. 62\% of all gradations deemed potentially unstable performed unstable.

Thus, the Burenkova method exhibits a 62\% success rate in identifying the unstable gradations and 62\% accuracy. By disregarding potentially irrelevant gradations the accuracy of the Burenkova method increases to 75\% (Table 2). Whereas the Wan-
Fell adaptation has an accuracy of 67% but lower success rate (44%), when excluding potentially irrelevant gradations (Figure 4). Potential for internal instability is for the Wan-Fell adaptation set at greater than 5% probability of internal instability.

**The wan-fell alternative method**

Wan & Fell\(^1\) discovered that potential for internal instability in soils with a steeply sloped coarse fraction and flat finer fraction are better evaluated by a value of \(D_{20}/D_4\) for the finer fraction and the same characteristic \(D_{90}/D_{60}\) value for the coarser fraction. Gradation analyses of all 23 gradations (Figure 5) and the results are summarized in Table 2, which shows that the Wan-Fell alternative method determines:

- a) 5 of the 12 unstable gradations (unstable CG3 not included due to insufficient gradation data) (42 %) locate in the unstable or transition region and have a potential for internal instability; and,
- b) 71% of all gradations deemed potentially unstable performed unstable.

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**Figure 3** Summary of gradation analyses: the Burenkova\(^5\) method.

**Figure 4** Summary of gradation analyses: the Wan & Fell\(^1\) (2004) adaptation.
Extending the Li-Fannin adaptation

The Kenney & Lau\textsuperscript{15,16} method has been tested in several studies in the literature\textsuperscript{10,20} and although, strictly, widely graded soil with fines is outside of the method’s extent of data, there are studies suggesting validity.\textsuperscript{12,23,26} Consider the laboratory tests of Rönqvist\textsuperscript{13} addressed in this study (Figure 8): specimens BE1, BE2, RA1, GR1 and GR4 are potentially internally unstable obtaining (H/F)\textsubscript{min} < 1, but performing without instability in the tests. This suggests that the Kenney-Lau boundary of H/F=1 over-predicts instability in glacial till gradations. Indeed, in reviewing the Kenney-Lau method, Rönqvist & Viklander\textsuperscript{27} found H/F=1 slightly conservative since the boundary does not encompass all stable gradations included in the Kenney-Lau data. By plotting stability index against erosion loss (Δm/m) of the Rönqvist\textsuperscript{13} tests, decreasing internal stability yields increasing erosion loss as indicated by the best-fit power trendline of the specimens with F\textsubscript{f}<35% (those potentially susceptible to suffusion) (Figure 6). The unstable specimens exhibits (H/F)\textsubscript{min} <0.68 and (Δm/m)>5 %, whereas the stable (H/F)\textsubscript{min} ≥0.68. ST2 deviates due to GBE. Also, by plotting the stability index before the test (i.e., pre-test) against after (i.e., post-test) a gradually more severe shift in grading characteristics can be seen at lower stability indices (Figure 7) (as indicated by the polynomial trendline). The post-test gradation data on d < 0.063 mm of ST1, ST2 and RA1 has been assumed equal to their initial, pre-test, gradation.

\begin{figure}[h]
\centering
\includegraphics[width=\textwidth]{figure5}
\caption{Summary of gradation analyses: the Wan & Fell\textsuperscript{21} alternative method.}
\end{figure}

\begin{figure}[h]
\centering
\includegraphics[width=\textwidth]{figure6}
\caption{Stability index (H/F) min in relation mass loss (power trendline is established for samples with F\textsubscript{f}<35 %).}
\end{figure}
In the range 0.68≤(H/F)_{min}<1.0 there is some intermixing around the line of unity (i.e., (H/F)_{pre}=(H/F)_{post}) suggesting a transition zone. More data is needed for a further analysis, nevertheless, the plotting positions of the stability indices (i.e., (H/F)_{min}) of the 23 gradations in this study (Figure 8) corroborates the possibility of an improved delineation between stable and unstable gradations in the Kenney-Lau H:F space. Indeed, by introducing the transition zone 0.68≤H/F<1.0 the stable gradations, previously deemed potentially unstable, locate in the transition zone (i.e., gradations GR4 and RA1) (Figure 8). Furthermore, by analysing stability indices (H/F)_{min} only, there is a risk of overlooking that a gradation exhibits potential instability along the H:F-shape curve. Consider gradation BE2: the stability index analysis for BE2 suggests stability according to the Li-Fannin adaptation, which agrees with the test performance, however, the shape of the BE2 gradation preceding the (H/F)_{min}=0.78 at F=20% is in the Kenney & Lau unstable zone H/F<1.0 beyond F≈11% (Figure 7). For BE2 this indicates instability over this range which contradicts its performance, suggesting there is a transition zone. Relevant also for specimen RA1 (although this specimen may not have been susceptible to suffusion due to excessive finer fraction) and specimen GR4 (Figure 8). Figure 8 shows the stability indices (H/F)_{min} plotted in the H:F-space, against the modified Li-Fannin adaptation with transition zone 0.68≤H/F<1.0. Although it fails to identify the unstable gradation C-30 the modified method’s success in identifying unstable gradations is slightly reduced to 67 % compared to the Kenney-Lau method, but the accuracy improves to 100 % (when excluding the potentially irrelevant gradations) (Table 2).
Conclusion

23 gradations of glacial till have been evaluated in this paper of which 13 have performed unstable. Five empirical methods for the evaluation of potential for internal instability have been applied on the gradations. Evaluation suggests that the approach by Kenney-Lau for shape analysis of the gradation curve, and its Li-Fannin adaptation, provide predictions relatively more accurate in identifying unstable gradations due to suffusion. A stricter assessment is suggested for this type of soil by modifying the Li-Fannin method so that it incorporates a transition zone defined by the range 0.68 < H/F < 1.0, indeed, 80 % of the gradations deemed potentially unstable with the modified method did perform unstable. However, in the design or specification of gradations using the Kenney & Lau method, a relaxation from the original H/F = 1 boundary is not recommended.

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Conflicts of interest

The authors declare that there is no conflict of interest.

References

1. Ravaska O. Piping susceptibility of glacial till. Florence Proc 19th ICOLD Congress; 1997:455–471.
2. Sherard JL. Sinkholes in dams of coarse, broadly graded soils. Proc 13th ICOLD Congress; 1979:25–35.
3. Foster MA. The probability of failure of embankment dams by internal erosion and piping. The University of New South Wales; 1999.
4. Foster M, Fell R, Spannagle M. The statistics of embankment dam failures and accidents. Canadian Geotechnical Journal. 2000;37:1000–1024.
5. Sherard JL. Filters for glacial till and other coarse impervious soils. Memorandum 1G, Unpublished reports of filter research carried out in the Lincoln, Nebraska Soil Conservation Service Soil Mechanics Laboratory. 1984.
6. Sherard JL, Dunnigan LP. Critical filters for impervious soils. Journal of Geotechnical Engineering. 1989;115(7):927–947.
7. Lilija HM, Rother M, Ravaska OT. Filter material study for an earth dam project. In: Dam Safety, editor. Balkema. 1988:825–832.
8. Hunter G, Fell R, Topham C. Backward erosion piping: what are the chances of that? Proc Australian National Committee on Large Dams 2012; Perth. 2012.
9. Lafleur J, Mlynarek J, Rollin AL. Filtration of broadly graded cohesionless soils. Journal of Geotechnical Engineering. 1989;15(12):1747–1768.
10. Wan CF. Experimental investigations of piping erosion and suffusion of soils in embankment dams and their foundations. Sydney, Australia. The University of New South Wales; 2006.
11. Lafleur J, Nguyen PH. Internal stability of particles in dam cores made of cohesionless broadly graded moraines. Internal Erosion of Dams and their Foundations. 2007:151–158.
12. Moffat R, Fannin RJ. A hydromechanical relation governing internal stability of cohesionless soil. Canadian Geotechnical Journal. 2011;48:413–424.
13. Rönnqvist H. On the assessment of internal erosion of dam cores of glacial till. Luleå, Sweden. Luleå University of Technology; 2015.
14. ICOLD. Internal erosion of existing dams, levees and dykes, and their foundations. Bulletin 164, Paris France. 2017.
15. Kenney TC, Lau D. Internal stability of granular filters. Canadian Geotechnical Journal. 1985;22(2):215–225.
16. Kenney TC, Lau D. Internal stability of granular filters: Reply. Canadian Geotechnical Journal. 1986;23:420–423.
17. Li M, Fannin RJ. Comparison of two criteria for internal stability of granular soil. Canadian Geotechnical Journal. 2008;45:1303–1309.
18. Burenkova VV. Assessment of suffusion in non-cohesive and graded soils. In: Brauns Heiburn, Schuler editors. Filters in geotechnical engineering. Proc. First International Conference “Geo-Filters, Rotterdam, Balkema. 1993:357–360.
19. Wan CF, Fell R. Experimental investigation of internal instability of soils in embankment dams and their foundation. UNICIV report No 429. The University of New South Wales; 2004.
20. Wan CF, Fell R. Assessing the potential of internal instability and suffusion in embankment dams and their foundations. Journal of Geotechnical and Geoenvironmental Engineering. 2008;134(3):401–407.
21. Rönnqvist H, Viklander P. Laboratory testing of internal stability of glacial till, a review. Electronic Journal of Geotechnical Engineering. 2014a;19:6315–6336.
22. Moffat R, Fannin RJ, Garner S. Spatial and temporal progression of internal erosion in cohesionless soil. Canadian Geotechnical Journal. 2011;48:399–412.
23. Skempton AW, Brogan JM. Experiments on piping in sandy gravels. Géotechnique. 1994;44:449–460.
24. Kezdi A. Soil physics – selected topics. Amsterdam. Elsevier Scientific Publishing; 1979.
25. Li M, Fannin RJ, Garner SJ. Application of a New Criterion for Assessing the Susceptibility to Internal Erosion. Canadian Dam Association; 2009.
26. Rönnqvist H, Viklander P. Extending the Kenney-Lau method to dam core soils of glacial till. Geotechnical Research. 2014b;1:73–87.
27. Rönnqvist H, Viklander P. On the Kenney-Lau approach to internal stability evaluation of soils. Geomaterials. 2014c;4:129–140.

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