Research Article

Stability Analysis of a Car-Following Model on Two Lanes

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Considering lateral influence from adjacent lane, an improved car-following model is developed in this paper. Then linear and nonlinear stability analyses are carried out. The modified Korteweg-de Vries (MKdV) equation is derived with the kink-antikink soliton solution. Numerical simulations are implemented and the result shows good consistency with theoretical study.

1. Introduction

Traffic flow is a system of consecutive vehicles with interaction [1]. Recently various models have been developed, including general models, safety distance models, action point models, optimal velocity models (OVM), cellular automaton models, and fuzzy logic models [2–4]. Among those models, OVM developed by Bando et al. [5, 6] is well known for its accuracy and rationality. Afterwards Helbing and Tilch [7] calibrated the OV model by experimental data and developed a generalized force model (GFM) to overcome the deficiencies. But both OVM and GFM cannot describe the phenomenon that the following vehicle may not decelerate when the leading vehicle is much faster even if the headway distance is smaller than safety distance. Then the inconsistencies of previous models were overcome by a continuous microscopic single-lane model, the intelligent driver model, developed by Treiber et al. [8] by the analysis of German freeways data. After this, a full velocity difference model (FVDM) was developed by Jiang et al. [9, 10] to solve the disadvantage. However, there are still some problems in previous models, which are discussed in detail and improved by Treiber and Kesting [11]. Also the gas-kinetic-based model was investigated by observed data and simulation experiments, which showed good agreement with phenomena in reality [12]. In this century, many new models have been established by considering decentralized delayed-feedback control [13], delay time due to driver’s reaction [14], extended OV function for acceleration difference [15], multiple velocity difference [16], and optimal velocity difference [17].

To study traffic jam waves in OVM, Komatsu and Sasa [18] firstly derived the modified Korteweg-de Vries (MKdV) equation to describe kink waves. Then Muramatsu and Nagatani [19] derived Korteweg-de Vries (KdV) equation from OVM to describe soliton waves in traffic jam, and Nagatani also found triangular shock wave solved Burgers equation [20]. From then many models have been analyzed by nonlinear stability theory aforementioned. Nagatani [21] derived MKdV equation near critical point in two continuum models: partial differential and discrete lattice model. Yu [22] presented a simplified OVM considering relative velocity and derived KdV and MKdV equations. Ge et al. developed several intelligent transportation system (ITS) based models with KdV and MKdV analysis [23] and also did similar research in three OVM based models [24]. Yu et al. [25] recently build a two-delay model with MKdV investigation and implemented numerical simulations. More studies show that the triangular wave, soliton wave, and kink wave occur in stable region, metastable region, and unstable region, respectively [20, 26, 27].

However, only a few researches focused on car-following with lateral impact, in which case the lateral influence from adjacent lane should be considered. Nagatani [28] presented two lattice models to simulate traffic flow wave on a two-lane highway with lane changing. Jin et al. [29] considered the lane-width influence and developed a non-lane-based
2. Improved OVM

The typical OV model is presented as [5, 6]  
\[
\frac{d^2 x_n(t)}{dt^2} = \alpha \left[ V^{\text{op}}(\Delta x_n(t)) - v_n(t) \right],
\]
where \( x_n(t) \) and \( v_n(t) \) are the position and velocity of the \( n \)th vehicle, \( \Delta x_n(t) \) is the headway distance between the \( n \)th and its leading vehicle, \( \alpha \) is the sensitivity parameter of the driver, and \( V^{\text{op}}(\cdot) \) is the optimal velocity function described as [5]  
\[
V^{\text{op}}(\Delta x_n(t)) = \frac{v_{\text{max}}}{2} \left[ \tanh(\Delta x_n(t) - h_c) + \tanh(h_c) \right],
\]
where \( v_{\text{max}} \) is the maximum velocity on a particular roadway and \( h_c \) means the safety headway distance.  

However, as noticed in the study on roadway, a driver usually focuses not only on the leading vehicle on the present lane, but also on the vehicle on adjacent lane, especially when the neighboring vehicle decelerates. This phenomenon occurs because of the potential action of lane changing or the avoidance of collision when the lane width is small [29]. Hence, the lateral influence should be considered in car-following model even if lane changing does not occur.

It is assumed that the driver makes his decision upon the combination impact of leading vehicle and neighbor vehicle by introducing a second OV function, which can be defined as  
\[
V^{\text{op}}(l_n(t)) = \begin{cases}  
V^{\text{op}}(\Delta x_{n+1}(t)), & l_v \leq \Delta x_{n+1}(t) < d, \\
0, & \text{others,}
\end{cases}
\]
where \( \Delta x_{n+1}(t) \) is the headway distance between the \( n \)th vehicle and its leading vehicle on the adjacent lane, \( l_v \) is the length of a normal vehicle, and \( d \) is a preset constant.

Referring to previous study, \( \Delta v_{l_n}(t) \) and \( \Delta v_{n}(t) \) are introduced [7], where \( \Delta v_{l_n}(t) = v_{n+1}(t) - v_n(t) \) and \( \Delta v_{n}(t) \) can be given as  
\[
\Delta v_{l_n}(t) = \begin{cases}  
v_n(t) - v_{n+1}(t), & l_v \leq \Delta x_{n+1} < d, \\
0, & \text{others,}
\end{cases}
\]
in which \( v_n(t) \) is the velocity of the leading vehicle on an adjacent lane.

The new model can be expressed as  
\[
\frac{d^2 x_n(t)}{dt^2} = \alpha \left[ pV^{\text{op}}(\Delta x_n(t)) + qV^{\text{op}}(\Delta x_{l_n}(t)) - v_n(t) \right] 
+ \lambda_1 \Delta v_n(t) + \lambda_2 \Delta v_{l_n}(t),
\]
where \( p \) and \( q \) are the weights of the two OV functions and \( \lambda_1 \) and \( \lambda_2 \) are the weights of velocity difference.

3. Linear Stability Analysis

According to linear stability analysis method [6], stable condition of the uniform traffic flow is given by  
\[
\begin{align*}
V_1^{(0)}(t) &= nh_c + V^{\text{op}}(h_c) t, \\
V_2^{(0)}(t) &= nh_c + V^{\text{op}}(h_c) t.
\end{align*}
\]

Let \( y_n(t) \) and \( y_{l_n}(t) \) be small deviations from \( x_n^{(0)}(t) \) and \( x_{l_n}^{(0)}(t) \) as \( x_n(t) = x_n^{(0)}(t) + y_n(t) \) and \( x_{l_n}(t) = x_{l_n}^{(0)}(t) + y_{l_n}(t) \). The linearized equation can be obtained:  
\[
\frac{d^2 y_n(t)}{dt^2} = \alpha \left[ pV'(\Delta x_n(t)) \Delta y(t) 
+ qV'(\Delta x_{l_n}(t)) \Delta y(t) - v_n(t) \right] 
+ \lambda_1 \Delta v_n(t) + \lambda_2 \Delta v_{l_n}(t),
\]
where \( V' \) and \( V' \) are the derivatives of OV functions \( V^{\text{op}}(\Delta x_n(t)) \) and \( V^{\text{op}}(l_n(t)) \). By expanding \( y_n(t) \) \( \propto \exp(inz + zt) \) and \( y_{l_n}(t) \) \( \propto \exp(inz + zt) \), (7) can be rewritten as  
\[
z^2 + \left[ \alpha - (\lambda_1 + \lambda_2) \left( e^{ik} - 1 \right) \right] z 
- \alpha \left( pV' + qV' \right) (e^{ik} - 1) = 0.
\]

Then expand \( z \) by the order of \( ik \) at the point of \( ik \approx 0 \) as \( z = z_1 \) \( ik + z_2(ik)^2 + \cdots \) and insert it into (8). The following terms can be obtained:  
\[
z_1 = pV' + qV',
\]
\[
z_2 = \left( \frac{1}{2} + \frac{\lambda_1 + \lambda_2}{\alpha} \right) \left( pV' + qV' \right) - \frac{(pV' + qV')^2}{\alpha}.
\]

According to previous study, the vehicle system is stable when \( z_2 > 0 \), which is  
\[
\alpha > 2 \left( pV' + qV' \right) - 2(\lambda_1 + \lambda_2),
\]
and the neutral stability condition has the following form:  
\[
\alpha_c = 2 \left( pV' + qV' \right) - 2(\lambda_1 + \lambda_2).
\]

The stability surface is described in Figure 1. Parameters are set as \( v_{\text{max}} = 4, h_c = 7, p = 1, q = 0, \lambda_1 = 0.2, \lambda_2 = 0 \) in Figure 1(a), while \( p = 0.8, q = 0.2, \lambda_1 = 0.16, \lambda_2 = 0.04 \) in Figure 1(c). As can be seen, the unstable region is smaller considering influence from adjacent lane [25].
4. Nonlinear Stability Analysis

For the convenience of nonlinear analysis, (5) is rewritten as

$$ \frac{d^2 \Delta x_n(t)}{dt^2} = \alpha \left[ \frac{pV(\Delta x_{n+1}(t)) - pV(\Delta x_n(t))}{\Delta t} \right. $$

$$ + qV(\Delta x_{l,n+1}(t)) - qV(\Delta x_{l,n}(t)) $$

$$ \left. + \lambda_1 \frac{d\Delta x_{n+1}(t)}{dt} - \lambda_1 \frac{d\Delta x_n(t)}{dt} \right] + \lambda_2 \frac{d\Delta x_{l,n+1}(t)}{dt} - \lambda_2 \frac{d\Delta x_{l,n}(t)}{dt}. $$

MKdV equation is obtained in unstable region around the critical point \((h_c, \alpha_c)\), where \(V'' = 0\). By the analysis method in [18], the long wave expansion is applied in this section.
\[
\begin{align*}
&+ \varepsilon^4 \left[ \alpha \partial_x R - \frac{\alpha (pV'' + qV''')}{6} \partial_x R^3 \right] \\
&+ \varepsilon^5 \left[ (2b - \lambda_1 - \lambda_2) \partial_x \partial_t R \right. \\
&\left. - \frac{\alpha (pV' + qV')}{24} \partial^4_x R \right] \\
&+ \varepsilon^6 \left[ \frac{\alpha (pV''' + qV''')}{12} \partial_x R^3 \right] = 0. \\
\end{align*}
\]

(15)

It is noticed that the \(\partial_x \partial_t R\) in the sixth order term of (15) can be eliminated by taking the derivative of \(X\) in the fifth order term. Then insert \(b = pV' + qV'\) and \(\alpha/\alpha - 1 = \varepsilon^2\) into (15), that is,

\[
\begin{align*}
&[\partial_t R - m_1 \partial_x^3 R + m_2 \partial_x R^3] \\
&+ \varepsilon [m_3 \partial_x^2 R + m_4 \partial_x^4 R + m_5 \partial_x^2 R^3] = 0,
\end{align*}
\]

where

\[
\begin{align*}
m_1 &= \frac{(\alpha + 3 \lambda_1 + 3 \lambda_2) (pV' + qV')}{6 \alpha}, \\
m_2 &= -\frac{(pV''' + qV''')}{6}, \\
m_3 &= \frac{(pV' + qV')}{2}, \\
m_4 &= \left[ \frac{4 (2pV' + 2qV' - \lambda_1 - \lambda_2)}{24 \alpha^2} \right] (\alpha + 3 \lambda_1 + 3 \lambda_2) \\
&\quad - \frac{\alpha + 4 (\lambda_1 + \lambda_2)}{24 \alpha} (pV' + qV'), \\
m_5 &= \frac{2 (2pV' + 2qV' - \lambda_1 - \lambda_2) - \alpha (pV''' + qV''')}{12 \alpha}.
\end{align*}
\]

(17)

In order to have standard MKdV equation, the following transformations are made in (16):

\[
T = \frac{1}{m_1} T_m', \quad R = \sqrt{\frac{m_1}{m_2}} R_m.
\]

(18)

Then (16) can be rewritten as

\[
\begin{align*}
&[\partial_{T_m} R_m - \partial_x^3 R_m + \partial_x R_m^3] \\
&+ \frac{\varepsilon}{m_1} [m_3 \partial_x R_m + m_4 \partial_x^2 R_m + \frac{m_1 m_5}{m_2} \partial_x^2 R_m^3] = 0. \\
\end{align*}
\]

(19)

Ignoring the \(O(\varepsilon)\) term, we have MKdV equation with a kink-antikink soliton solution expressed as

\[
R_m (X, T_m) = \sqrt{B} \tanh \left[ \sqrt{\frac{B}{2}} (X - BT_m) \right].
\]

(20)

To determine the value of amplitude \(B\), the solvable condition is considered:

\[
(R_m, M[R_m]) \equiv \int_{-\infty}^{\infty} R_m M[R_m] dX = 0,
\]

(21)

where \(M[R_m]\) means the \(O(\varepsilon)\) term in (19). By performing the integration of (21), the value of amplitude \(B\) can be obtained:

\[
B = \frac{5m_2 m_5}{2m_1 m_4 - 3m_1 m_5}.
\]

(22)

The kink-antikink soliton solution of headway can be written as follows:

\[
\Delta x_n = h_c + \sqrt{\frac{\alpha_c}{\alpha}} \left( \frac{5m_1 m_5}{2m_1 m_4 - 3m_1 m_5} \right)
\times \tanh \left[ \sqrt{\frac{\alpha_c}{\alpha}} \left( \frac{5m_2 m_5}{4m_2 m_4 - 6m_1 m_5} \right) \right] \\
\times \left[ n + \left( \frac{pV' + qV'}{2} \right) \right]
\times \left( \frac{\alpha_c - 1}{\alpha} \right) \left( \frac{5m_2 m_5}{2m_2 m_4 - 3m_2 m_5} \right).
\]

(23)

With the amplitude of (23), we have the coexisting surface in Figures 1(b) and 1(d) based on Figures 1(a) and 1(c), respectively. The space is divided into three regions: stable region above the coexisting surface, metastable region between coexisting surface and stability surface, and unstable region below stability surface. However, the limitation should be noted that this nonlinear analysis is only valid at the critical point when \(h = h_c\).

5. Numerical Simulation

Consider a two-lane system with 100 vehicles running on each lane under a periodic boundary condition without overtaking or lane changing. The initial values are \(v_{\text{max}} = 4\), \(h_c = 7\), and \(d = 10\) on both lanes. In this section, only the situation of \(p + q = 1\) is simulated, because we suppose that the sum of OV functions’ weights should remain as 1, which
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means the driver has no extra attention when considering lateral influence.

First add a disturbance on one lane as follows:

\[
\begin{align*}
\text{for lane 1, } & \Delta x_n(0) = 7 - 0.3 \quad 45 < n < 50 \\
& \Delta x_n(0) = 7 \quad \text{others;}
\end{align*}
\]

\[\text{for lane 2, } \Delta x_n(0) = 7 \quad 0 < n \leq 100.\] (24)

Figure 2: Space-time evolution of headway for lane 1 in (a) and lane 2 in (b) when \( p = 0.8, q = 0.2, \lambda_1 = 0.16, \) and \( \lambda_2 = 0.04 \) and for lane 1 in (c) and lane 2 in (d) when \( p = 0.6, q = 0.4, \lambda_1 = 0.12, \) and \( \lambda_2 = 0.08.\)

the proportion of lateral influence increases from 0.2 to 0.4, the density wave on lane 2 becomes heavier, while on lane 1 the perturbation decays.

Second, we consider both lanes have small disturbances, which are defined as follows:

\[
\begin{align*}
\text{for lane 1, } & \Delta x_n(0) = 7 \quad 0 < n \leq 60 \\
& \Delta x_n(0) = 7 \quad \text{others;}
\end{align*}
\]

(25)

Let \( \alpha = 2.85, p = 1, q = 0, \lambda_1 = 0.2, \) and \( \lambda_2 = 0, \) which cannot satisfy the stable condition in (10). Figure 3...
shows the space-time evolution of headway for lane 1 in (a) and lane 2 in (b) from 900 s to 1000 s. The headway profiles of traffic wave at $t = 950$ s are in (c) for lane 1 and (d) for lane 2. It is observed that the small perturbation propagates into traffic jam on both lanes. Furthermore, traffic jam is more serious in lane 2 due to the larger initial headway perturbation.

Then suppose $\alpha = 2.85$, $p = 0.8$, $q = 0$, $\lambda_1 = 0.2$, and $\lambda_2 = 0$. Figure 4 describes the space-time evolution of headway and the headway profiles of traffic wave at $t = 950$ s corresponding to Figure 3. The initial perturbation decays after sufficient time on both lanes. Thus the consideration of lateral impact from adjacent lane can suppress traffic jam.

The amplitude of traffic wave in lane 2 is larger just like in Figure 3.

Finally when $\alpha = 2.2$, $p = 0.8$, $q = 0.2$, $\lambda_1 = 0.16$, and $\lambda_2 = 0.04$, the system is more unstable and serious kink-antikink waves are observed in Figure 5. Unlike Figures 3 or 4, there is no significant difference between the two lanes, because all the vehicles get influenced heavily by lateral impact.

Then we exchange the disturbances of the two lanes, and simulations show similar results. In other words, the situation on lane 1 after exchange is like lane 2 before exchange, the same for lane 2. By simulation, the theoretical analysis of MKdV solution can be described.
6. Conclusions

In this paper, a new car-following model is proposed considering lateral influence from adjacent lane. Linear and nonlinear stability analyses are carried out, from which MKdV equation is obtained. Numerical simulations show that new model has good consistency with theoretical study and can suppress traffic jam wave. However, when $\alpha$ satisfied the unstable condition, both lanes will have serious traffic jam of same level despite different initial headway perturbation. In conclusion, even though lane changing does not happen, considering lateral impact has influence on car-following behavior indeed and can keep the vehicle system more stable.

Conflict of Interests

The authors declare that there is no conflict of interests regarding the publication of this paper.

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Figure 5: Space-time evolution of headway for lane 1 in (a) and lane 2 in (b) and headway profile of traffic wave at $t = 950$ s for lane 1 in (c) and lane 2 in (d). ($\alpha = 2.2$, $p = 0.8$, $q = 0.2$, $\lambda_1 = 0.16$, and $\lambda_2 = 0.04$).

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