Capacity assessment of urban roads according to Slovak standard does not sufficiently take into account the influence of vehicle maneuvers on road section performance between the two intersections. This research analyzed turning relations on selected urban road intersections and the influence of right turn and left turn on major stream speed changes. For the purpose of determining the influence of the right turn on decrease in capacity, measurements were carried out at 7 uncontrolled intersections with curve radius ranging from 6 to 30 m. To determine the influence of the left turn on the capacity of urban road a critical gap was evaluated, using the Rauff’s method, based on the analysis of accepted and rejected gaps in the major traffic stream, which was one of the main inputs for simulations. The findings were used to determine the influence of selected maneuvers on the road capacity, which led to a proposal for reduction coefficients based on computational analysis performed using simulations in PTV VISSIM.

Keywords: traffic, urban roads, capacity, traffic flow, simulation

1 Introduction

Influence of the turning maneuvers at simple intersections and parking spaces on the reduction of the capacity of urban roads has not received much attention. Focus has predominantly been directed at signal intersections, e.g. Kim [1].

Several models and analyses were created for unsignalized intersections, which consider turning relations [2-6]. Authors of [7] dealt with an analysis and modelling of turning relations and their influence on the intersection capacity in urban intersections in China. They generally focused on two types of crossing behaviors. The first type is called the single-vehicle-crossing (SVC) and usually appears when the traffic flow is not saturated. The second type is known as vehicle-stream crossing (VSC) and refers to situations when one or more vehicles have to seek for the right gap to move across another vehicle stream. More attention from researchers is devoted to describing this type of crossing behavior.

Research has also focused on the capacity analysis of turning maneuvers on the U-turn intersections and their influence on the capacity of the intersections. This topic was addressed in [8] where analysis applied the values of the base critical gaps $t_{c,base}$, as well as the base follow-up time $t_{f,base}$.

The critical gap was determined to be about 6.4 s, the follow-up time 2.5 s. In the case of U-turns, accommodated at median openings with narrow median nose width of 6.4 m, the critical gap was about 6.9 s and the follow-up time 3.1 s. Difference in capacity was up to 286 pc/h for making the U turns at median openings with narrow medians (median nose width 6.4 m).

The turning relations are often simulated by software tools. In recent years, VISSIM of PTV VISION packet has been used most frequently.

Researchers in [9] simulated the turning relations using the VISSIM. Although their analysis of turning was predominantly based on the Harder’s model, they also determined the base critical gaps for different types of movement on unsignalized intersections (Table 1). Apart from presenting important data, the model confirms the suitability of the VISSIM program for similar purposes.

Delay during the turning relations on intersections is also analyzed in the Highway Capacity Manual [10]. The HCM analyzes the problem of calculation of delay during the turning on intersections separately for the left and right turns. The delay during the right turn is solely a result of deceleration of turning vehicles and the adaptation to the speed of previous vehicles. The delay during the left turn is a result of waiting for the previous vehicle to complete their turning maneuver. Generally, the typical values of delay given in Table 2 may be used.

The values are only valid for 10% of turning vehicles and must be reduced in the case of the remaining vehicles. Predictions of speed changes on the right turning lanes on urban streets are offered in [11].

The parking manoeuvers significantly contribute to reduction of the capacity of urban streets. Results of [12], which describe the number of lanes and the number of parking manoeuvers per hour as the variables for the capacity reduction, were adopted in the HCM 2000 [10]. Likewise, Valleley in [13] focuses on relation between
The influence of turning vehicles on the capacity of urban roads

2.1 Right turns

Measuring spots were selected based on previous research findings related to the characteristics of traffic and road network, in a way that would represent maximum range of radius sizes for turns from the main road, a sufficient number of turns with respect to traffic intensity and the angle of intersection of approx. 90°. Altogether, 7 uncontrolled level intersections with curve radius ranging from 6 to 30 m were tested. Measurements were carried out at individual spots in duration of 4 hours in order to record the traffic flow during the peak hour, as well as in lower volumes.

The methodology of collection, selection and analysis of data regarding the characteristics of traffic flow and observations of actual intersections in the city of Zilina were performed as a part of research activities conducted at the author’s institution. The aim of this part was to determine the influence of selected maneuvers on urban road capacity. The analysis included the effect of turning vehicles to the capacity of the intersection without the influence of next effects, e.g. impact of pedestrians, in the sense of Slovak Standard for the simple intersections. Only the analysis of regular intersections was selected for presentation in the paper.

The results were compared to the currently effective methodology stated in Slovak standard, which only takes into consideration the influence of maneuvers that is based on their minimum numbers. To verify the theoretical calculations, measurements were used as input into a simulation model which was used to further specify obtained results.

| Vehicle movement                          | Base critical, \( t_{c,base} \) (s) | Base follow-up time, \( t_{f,base} \) (s) |
|------------------------------------------|-------------------------------------|------------------------------------------|
| Left turn from major (four-lane)         | 4.1                                 | 2.2                                      |
| U-turn (six-lane)                        | 5.6                                 | 2.3                                      |
| U-turn (four-lane, wide median)          | 6.4                                 | 2.5                                      |
| U-turn (four-lane, narrow median)        | 6.9                                 | 3.1                                      |
| Through traffic on minor (four-lane)     | 6.5                                 | 4.0                                      |
| Left turn from minor (four-lane)         | 7.5                                 | 3.5                                      |

Table 2 Typical values of delay by HCM [10]

| Midsegment Volume (veh/h/ln) | Through Vehicle Delay (s/veh/pt) by Number of Through Lanes |
|-----------------------------|-------------------------------------------------------------|
|                             | 1 Lane | 2 Lanes | 3 Lanes |
| 200                         | 0.04    | 0.04    | 0.05    |
| 300                         | 0.08    | 0.08    | 0.09    |
| 400                         | 0.12    | 0.15    | 0.15    |
| 500                         | 0.18    | 0.25    | 0.15    |
| 600                         | 0.27    | 0.41    | 0.15    |
| 700                         | 0.39    | 0.72    | 0.15    |
The data reflect changes of the traffic flow characteristics depending on design, [16]. Six simulation series were carried out using the model, each one corresponding to a particular radius size and a corresponding speed on a turn (of maximum and minimum size). Thus calibrated, the model was assigned a lane with varying numbers of vehicles going straight and turning right. The alteration of values was carried out 8 times for each series, with the following ratios: 1775:25, 1750:50, 1700:100 vehicles per hour and with the following ratios expressed in percentage: 90:10, 80:20, 70:30, 60:40 and 50:50. The purpose of this variation in ratios of the direction of traffic was to create various situations to determine the influence of the right turn maneuver on the decrease in the capacity of a collector urban road with respect to the number of turning vehicles. The first three ratios corresponded to

follows that increasing the turn radius size by 1 m increases the speed by 0.9 km/h.

This confirms the assumption that regulations should not quantify all the right turns by one common value because their influence on the vehicle speed might be entirely different.

To determine the influence of selected vehicle maneuvers on decreasing the capacity of collector urban road, a simulation model was developed using the simulation software PTV VISSIM. From geometrical perspective, this was a simple model. To calibrate the model, the final speed values and other characteristics of vehicle movement in traffic flow were used in order to determine the influence of the radius size of the right turns on the capacity of urban road. The microsimulation allows evaluate the traffic flow characteristics second by second based on the new design elements. Software PTV VISSIM could log several indicators (e.g. speed, density and volumes) from the lane segments. The data reflect changes of the traffic flow characteristics depending on design, [16].

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Table 3 Through vehicle speed (km/h) with respect to the right turns for $a = 3.4 \text{ m/s}^2$

| R (m) | Number of right turns | Percentage of right turns |
|-------|-----------------------|--------------------------|
| 6     | 49.033 48.729 48.066 | 47.318 44.459 41.754 38.245 37.128 |
| 9     | 49.069 48.992 48.3 47.717 46.644 44.456 42.405 38.565 |
| 12    | 49.106 49.091 48.64 48.143 47.168 46.427 45.236 43.392 |
| 18    | 49.135 49.176 48.003 48.37 48.145 47.594 47.073 45.856 |
| 24    | 49.184 49.261 49.159 49.093 49.175 48.92 48.708 48.402 |
| 30    | 49.194 49.281 49.235 49.291 49.402 49.425 49.246 48.985 |

Table 4 Reduction coefficient of capacity with respect to the right turns for $a = 3.4 \text{ m/s}^2$

| R (m) | Number of right turns | Percentage of right turns |
|-------|-----------------------|--------------------------|
| 6     | 1.00 1.00 1.00 1.00 0.99 0.98 0.96 0.94 |
| 9     | 1.00 1.00 1.00 1.00 1.00 0.99 0.97 0.96 |
| 12    | 1.00 1.00 1.00 1.00 1.00 0.99 0.98 0.98 |
| 18    | 1.00 1.00 1.00 1.00 1.00 1.00 0.99 0.99 |
| 24    | 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 |
| 30    | 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 |
the fact that the initial required speed of vehicles in a simulation model oscillates within the range 50±2.0 km/h, the decrease in the speed of the through traffic flow due to the right turn is very low. Maximum decrease in speed is 13 km/h, which corresponds to the minimum radius size and represents 50% of the right turns.

The results show that the deceleration of vehicles prior to turning (i.e. the distance from the edge of the intersection at the moment when deceleration began) does not have a significant influence on the traffic flow as a whole and the corresponding average decrease in speed and urban road capacity.

From these findings follows that influence of the right turn on the decrease in the capacity of urban road is not significant. Decrease in capacity takes place when the ratio of turning vehicles is high (30% and more), provided that the radius size is lower than 12 m. Table 4 illustrate that maximum decrease of capacity according to the reduction coefficient determined from the ratio of vehicles released by the simulation model to the initial number of vehicles, is approximately 6%, at minimum radius size and 50% of right turns.

![Figure 2](image1.png)  
**Figure 2** Dependency of decrease of capacity on the number (percentage) of turning vehicles and curve radius size (a = 3.4 m/s²)

![Figure 3](image2.png)  
**Figure 3** Decrease in capacity with respect to number of the right turns according to the research and according to Slovak standard

The number of turns from the continuous straight lane in direction to the right as they are considered by STN 73 6110 [17] i.e. 25, 50, 100 maneuvers per hour, respectively. The series were simulated twice, for the value of deceleration of a vehicle of a = 2.0 m/s² as considered by the simulation software and for the value of deceleration of a vehicle of a = 3.4 m/s² by AASHTO Green Book [18]. Overall, to determine the influence of the right maneuver on the capacity of the collector urban road, nearly 100 simulations were carried out and evaluated, the required output data being the average speed of the continuous traffic flow and the number of vehicles passing through the model in 1 hour, which was used to determine the decrease of the capacity as a ratio of these vehicles to the number of vehicles equal to the basic capacity, i.e. the basic volume of the collector urban road (reduction coefficient). Examples of results of the simulations are shown in Tables 3 and 4 and Figure 2.

Results show that the decrease in the speed of vehicles in the traffic flow is lower than what was predicted from the absolute values of the speed of vehicles on turn with respect to the radius size. The average decrease in speed of the traffic flow from maximum speed limit was 2.89 km/h at a = 2.0 m/s² and 2.74 km/h at a = 3.4 m/s². Considering the fact that the initial required speed of vehicles in a simulation model oscillates within the range 50±2.0 km/h, the decrease in the speed of the through traffic flow due to the right turn is very low. Maximum decrease in speed is 13 km/h, which corresponds to the minimum radius size and represents 50% of the right turns.

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The outcome of the measurements, processed videos from individual intersections and subsequently obtained accepted and maximum rejected gaps, was an estimation of critical time gaps. They were determined for individual intersections, which were observed by four selected methods of assessment:

1. estimate of $t_g$ based on the gaps that were accepted and rejected by drivers turning left, and which occurred between the two vehicles driving only straight in a superior traffic flow during the peak-hour intensity - label (PHI without RT)

2. estimate of $t_g$ based on all the gaps that were accepted and created by drivers turning left, which occurred in a superior traffic flow during the peak-hour intensity - label (PHI)

3. estimate of $t_g$ based on the gaps that were accepted and rejected by drivers turning left, and occurred between the two vehicles driving only straight in a superior traffic flow during the time of measurements - label (all measurements, without RT)

4. estimate of $t_g$ based on the gaps that were accepted and rejected by drivers turning left, and which occurred in a superior traffic flow during the time of measurements - label (all measurements).

The results of these evaluations are presented in Table 5.

The results show a relatively broad span in $t_g$, which is dependent on the type and quantity of data it was estimated from. The $t_g$ based on measurements taken during the peak hour has a larger span than $t_g$ based on all the measurements, taken during the entire observation. The maximum span of $t_g$ reaching 2.0 seconds resulted from evaluation of gaps between the two vehicles driving straight in the superior traffic, which drivers turning left accepted or rejected during the peak hour. The dependency of the critical time gaps $t_g$ on the intensity of superior streams is illustrated by Figure 5.
Since it is expected that the capacity of a section of a collector urban road in the case of the left turns is largely influenced by the density of the superior traffic flow, the entry intensity values for the superior stream in the simulation model varied from 100 to 1800. Overall, to determine the influence of the left maneuver on decrease in the capacity of the collector urban road, 576 simulations were carried out and evaluated leading to 576 reduction coefficients. An example of the simulation results is presented by Figures 7 and 8.

From the findings follows that with increasing critical time gap $t_g$ increases the influence of left turn, which causes a significant decrease in the capacity of a section. With the maximum number of turns (100 = 33 left turns), which is considered by Slovak Standard [17], the decrease in capacity may be as much as 39 % (entry volume 1800 pc/h with the number of vehicles in the superior traffic flow 1800 pc/h), or 43 % (entry volume 1233 pc/h with the number of vehicles in the superior traffic flow 1800 pc/h).

Increase in the number of turning vehicles causes increase in the influence of the left turn, which results in a significant decrease in the capacity of a section. At the ratio of the left turning vehicles of 10 %, the capacity decreases as soon as the volume of the superior traffic stream is 700 pc/h with $t_g = 4$ s leading to a decrease by 6 %, and with $t_g = 6$ s up to 14 % (at the intensity upon entry at 1800 pc/h) (Figure 9).
For 33 left turns, the standard considers flat decrease in the capacity by 20% with no regard to the intensity of the superior traffic flow and the critical time gap. The research findings show that decrease in the capacity spans between 0-28% for $t_g = 4.0$ s; 0-53% for $t_g = 5.3$ s; 0-71% for $t_g = 5.5$ s.

Given the same volume upon entry and the ratio of turning vehicles being 50%, the capacity decreases as soon as the intensity of the superior traffic stream is at 400 pc/h, at $t_g = 4$ or 200 pc/h for other $t_g$.

For 33 left turns, the standard considers flat decrease in the capacity by 20% with no regard to the intensity of the superior traffic flow and the critical time gap. The research findings show that decrease in the capacity spans between 0-28% for $t_g = 4.0$ s; 0-53% for $t_g = 5.3$ s; 0-71% for $t_g = 5.5$ s.

**Figure 7** Dependency of average speed of the traffic stream on the number (percentage) of turning vehicles and the intensity of counter-stream DP

**Figure 8** Dependency of number of realized maneuvers on the number (percentage) of turning vehicles and the opposing flow rate

**Figure 9** Dependency of capacity decreasing on the $t_g$ value
s and 6.0 s. From that follows that the capacity of a section should not be determined solely by the number of the left turns because of the broad span of the rate of influence of this maneuver in relation to the intensity of the superior traffic flow. In the extreme case, a higher ratio of the left turns may decrease the capacity by as much as 90%.

3 Conclusion

The analysis of results shows that the right turn per se does not significantly influence the decrease in the capacity of urban road as it is considered by the Slovak standard [17]. The decrease in the capacity only occurs at a high ratio of turning vehicles (min. 30%) and at the radius size not being larger than 12 m. From that follows that in order to assess the capacity of urban road, it would be more objective to use a combination of the two methods - to determine Level of Service (LOS) based on the speed of the continuous traffic flow and decrease in the capacity based on the reduction coefficient.

In the case of the left turn, the results show clear dependency of the capacity of urban road and the speed of traffic flow on the combination of the intensity, i.e. the ratio of turning vehicles and volume of the superior traffic flow.

All the obtained dependencies could provide guidelines for the assessment of collector roads not based on the decreasing their capacity but based on determination of functional levels with respect to the average speed of vehicles in traffic flow, as well. It has clearly been shown that besides the number of turning vehicles, it is necessary to take into account other characteristics of the traffic flow, such as the intensity of the superior traffic stream and the critical time gap. The presented results objectify and expand the scale of values required for the determination of the maneuvering coefficient.

Furthermore, in order to determine the decrease in the capacity of uncontrolled intersections in the urban areas [24], it is necessary to pay attention to impact of pedestrians on the turning maneuvers and also the effect of intersections with the unconventional arrangement of superior traffic flows. In this respect, research activities at the university are already being implemented and are directed towards adjusting the Slovak standards and technical regulations.

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