The energy threshold in dynamic probing

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Abstract

In dynamic probing tests, penetration is closely related to the potential energy of the hammer (nominal energy). This energy stems from the mass and free fall of the hammer after being released from a certain height. Penetration depends on energy, although only on a portion of that nominal energy that is effectively transferred to the rods (ENTHRU) and, more precisely, the energy that reaches the cone (ENTHRUcone). ENTHRU can be measured by monitoring the upper part of the drive rods. To calculate ENTHRUcone, ENTHRU needs to be corrected in three ways. Firstly, the energy loss in the energy transmission through the rods has to be subtracted, as well as the energy loss due to the skin friction of the rods along the soil around them. It is also necessary to add the energy due to the rod weight penetrating the soil. The main hypothesis assumed and later experimentally proved in this paper is based on the fact that ENTHRUcone has to be greater than a certain value or minimum energy (energy threshold: $T_h$) in order to be able to cause penetration. After analyzing more than one hundred blows with different hammer mass and drop height, a small but consistent $T_h$ and a linear relationship between energy and penetration beyond it were found. The energy that really produces penetration (ENPEN) will be $ENTHRU_{cone}$ minus $T_h$. This allows for improved energy corrections and correlations between results from various kinds of penetration tests.

Keywords Correlation · Penetration · Dynamic · Probing · Energy · Threshold

Introduction

Results from dynamic penetration tests are extensively used in geotechnical studies for calculating and designing purposes (Mishra and Robinson 2019). As there are many diverse types of dynamic penetration tests (Matsumoto et al. 2015), it is necessary to be able to correlate results among these various kinds of tests. The current formulas (Avanzi et al. 2013; Bergdahl 1979; Daniel et al. 2003; Hiley 1925) establish an inverse linear correlation between nominal energy and the number of blows ($N$). There is a market demand for improving these old, but widespread used correlations in order to make more accurate and efficient geotechnical designs. Current standards (ASTM 2016; ISO-AENOR-CEN 2005, 2011) also force geotechnical engineers to make energy corrections when using dynamic penetration test results. This article studies the penetration mechanism related to energy, proposing a new and improved way of correlating results among different kinds of dynamic penetration tests and a new equation for applying energy corrections to adjust blow counts to 60% energy efficiency.

Much effort has been made in the past (Abou-Matar and Goble 1997; Butler et al. 1998; Odebrecht et al. 2005; Sancio and Bray 2005; Schmertmann and Palacios 1979; Sy and Campanella 1991) to measure the parameters (force, acceleration, displacement and energy) involved in the Standard Penetration Tests (SPT), although studies on Dynamic Probing Tests (DP) have received less attention in specialized literature (Matsumoto et al. 2015; Michi et al. 2004; Žaržojus et al. 2013). The aim of the work presented in this paper is to reduce this gap, helping to understand the mechanism of the advance of continuous dynamic penetrometers. Some of the findings made for the SPT can be extended to DP; however, there are many substantial differences that require a specific study.
Penetration is not only governed by soil characteristics, but also by the energy used by the penetration equipment. This penetration is not directly related to the whole potential energy of the hammer (nominal energy), but only to the portion of that energy that is effectively transferred to the rods (ENTHRU), as many authors state (Abou-Matar and Goble 1997; Butler et al. 1998; Daniel et al. 2003; Lee et al. 2014; Sancio and Bray 2005; Sy and Campanella 1991) and, more precisely, the energy that reaches the cone (ENTHRUcone) (Anbazhagan et al. 2022; Lukiantchuki et al. 2017; Odebrecht et al. 2005; Schmertmann and Palacios 1979). In practice, the interpretation of both SPT and DP tests is made by counting the number of normalized blows (N) needed for a given penetration length: 30 cm in SPT, 20 cm in DPSH-B, 10 cm in DPH, etc. (ISO-AENOR-CEN 2005, 2011). The nominal energy of each blow also varies across the different tests, albeit within a relatively narrow range. Typically, these values vary approximately between 300 and 500 J. If the DPM (medium dynamic probing) test is also considered, the range varies approximately between 150 and 500 J (ISO-AENOR-CEN 2005, 2011; Matsumoto et al. 2015).

As a first approximation, the comparison between the results using different penetrometers is based on a linear relation between the penetration per blow and the energy applied (Avanzi et al. 2013; Bergdahl 1979; Daniel et al. 2003). However, there are some non-linearities associated to energy losses at the hammer (Farrar 1998; Kovacs and Salomone 1982), the friction between the driving rods and the surrounding soil (Dahlberg and Bergdahl 1974), etc. Considerable advances (Dahlberg and Bergdahl 1974; Odebrecht et al. 2005; Schmertmann 2007) have been made to take them into account in a simple way while considering the nature of the phenomena involved.

However, there is a basic source of deviation (the existence of an energy threshold Th) associated to the non-linearity of the stress–strain behavior of soils at small stresses. This is part of the assumption considered and further demonstrated throughout this research (Fig. 1). Nevertheless and in practice, a linear relation is assumed in a given soil between energy and penetration in the usual energy range for penetration tests (Avanzi et al. 2013; Bergdahl 1979; Daniel et al. 2003).

In this study, a conventional dynamic penetrometer has been instrumented by installing accelerometers and strain gauges at a point on the driving rods, which, after integration, allows the evolution of the applied energy during each blow of the hammer. The final permanent vertical tip displacement is measured by a caliper. The driving parameters (hammer weight and drop height) vary from blow to blow in order to get a full range of energies, with the tests performed on a site with uniform granular soil.

One of the results of the test campaign is that the relationship between useful energy and tip displacement (penetration) in each blow is approximately linear, but with an apparent energy threshold Th below which no penetration occurs, as shown in Fig. 1.

The threshold Th should be subtracted from the ENTHRUcone to obtain the energy that really produces non-recoverable penetration (plastic displacement). This energy has to be used to correlate results among different dynamic penetration tests if a linear relationship between energy and penetration is assumed to be used, as it is in practice at present (Avanzì et al. 2013; Bergdahl 1979; Daniel et al. 2003). This Th is not a real threshold, as the curve energy-penetration is not perfectly linear, especially for small energy values (Fig. 1); however, for the usual range of energies used in real penetrometers, the soil behaves as if such relationship was linear (see the dotted line in Fig. 1), but with an apparent energy threshold (Th).

In summary, this paper sets out the development of a new adjusted method for correlating results from different kinds of dynamic penetrations tests, as well as offering a proposed new method for the energy correction requirement, taking into account the existence of the aforesaid Th. A method for calculating this Th is also proposed.

**Energy efficiency**

Many authors have been studying and measuring the energy balance when performing the SPT. The first studies covering this issue can be traced back to Palacios (1977) and Schmertmann and Palacios (1979), who measured energy transference by means of the use of strain gauges. Building on these first analysis, several authors followed this line of research during the next decades (Abou-Matar and Goble 1997; Butler et al. 1998; Farrar 1998; Lee et al. 2009; Odebrecht et al. 2005; Sancio and Bray 2005; Sy and Campanella 1991). During the last 10 years, it has still been a main theme in the field of in situ testing, with many other authors introducing very valuable observations (Anbazhagan et al. 2021, 2022; Lee et al. 2014, 2012; Look et al. 2015;
Lukiantchuki et al. 2017; Matsumoto et al. 2015). However, less attention has been paid to the analysis of the dynamic probing (DP) tests (Matsumoto et al. 2015; Michi et al. 2004; Žaržojus et al. 2013). For this reason, the derivation of the dynamic equations for the cone penetrometers (DP) are made following the line of previous analyses for SPT.

**Energy efficiency for SPT**

Even though there are several studies regarding the measurement of energy in the SPT, this paper is consistent with one of the main recent investigations on this matter (Odebrecht et al. 2005). The methodology proposed by Odebrecht et al. (2005) has been used for several reasons, firstly, because this is one of the latest and most complete pieces of research on energy measurement in penetration testing. In the authors’ opinion, this approach is the most appropriate to apply to this research, since it considers the real energy that reaches the bottom part of the rod assembly, i.e., the cone ($E_{\text{sampler}}$), unlike other methods of operation that consider only the energy transmitted to the upper part of the rods (close to the anvil). Thus, by using this methodology, it has been possible to include the additional potential energy due to the mass of the rods and the hammer during penetration displacement, with every energy loss considered during transmission of the energy through the rods.

Housel (1965) was the first to use the term $\text{ENTHRU}$, that is, the energy transmitted to the rod stem. $\text{ENTHRU}$ has been measured by means of integrating signals from some accelerometers and strain gauges located below the anvil, as is usual in such tests.

Some authors (Daniel et al. 2003) have used this $\text{ENTHRU}$ in order to correlate results from different types of tests.

Odebrecht et al. (2005) found that $\text{ENTHRU}$ could be expressed through this equation:

$$\text{ENTHRU} = \eta_1 \cdot m_h \cdot g \cdot (h + p) \tag{1}$$

where $\eta_1$ is the hammer efficiency factor, $m_h$ represents the hammer mass, $g$ is the gravity acceleration, $h$ is the height of fall (free fall of the hammer after being released), and $p$ is the permanent penetration per blow.

After analyzing the data from the tests using Schedule 80 rods, pursuant to the Brazilian Standard (NBR-ABNT 2001), the value of $\eta_1$ yields 0.765.

Odebrecht et al. (2005) also inferred the value of the computed sampler energy ($E_{\text{sampler}}$), the value of the actual energy that reaches the sampler.

$$E_{\text{sampler}} = \eta_3 \cdot \left[ \eta_1 \cdot m_h \cdot g \cdot (h + p) + \eta_2 \cdot m_r \cdot g \cdot p \right] \tag{2}$$

where $\eta_3$ represents the energy efficiency factor, $\eta_2$ is the rod efficiency factor, and $m_r$ is the rod mass.

The adjusted experimental data and the proposed values for the efficiency factors were:

$$\eta_3 = 1 - 0.0042 \cdot l \tag{3}$$

where $l$ is the rod length (m).

The value of $\eta_2$ equals 1.

**Differences between SPT and DP tests**

Both kinds of tests are quite similar although there are some differences that will have to yield to different approaches to the measurement of the energy.

Firstly, the DP tests are performed continuously along the depth, whereas in the case of SPT, the tests are conducted only at certain points inside a borehole that is perforated using different methods (Fig. 2). As this research is fully devoted to Dynamic Probe (DP) testing, the base scenario for this investigation will be the second configuration shown in Fig. 2.

SPT tests are carried out with a sampler of a smaller diameter than the borehole casing, driven into the soil, allowing a disturbed sample to be taken. The bottom of the borehole, inside and around the sampler, is a free boundary. On the contrary, in the DP tests, the hole is created by the cone itself, with no horizontal free boundary at the bottom. On the other hand, there is no casing, meaning that the vertical walls can deform and exert a certain radial pressure and hence friction on the rods.

**Materials and methods**

Due to the differences between SPT and DP tests, a new method for calculating energy efficiency in DP tests shall be proposed. It is important to highlight that there are many studies that calculate energy efficiency in SPT tests; however, there are very few studies on energy efficiency in DP tests.

Such energy efficiency has to be used to correlate results among different DP tests. For this purpose, it is good practice to use the real energy that produces penetration. An improvement will be proposed in the use of such correlations, considering that an energy threshold ($T_h$) may exist which has to be exceeded to produce penetration.

This is the key factor of the present paper and the existence of this $T_h$ will be demonstrated throughout this research.

**Measuring lateral friction**

The drive rods and the cone will be driven vertically in order to avoid lateral friction. This is of particular relevance in dense
sands and cohesive soils at depth (Cearns and McKenzie 1988).

Rods are rotated 1.5 turns or until maximum torque is reached at least at every 1-m penetration. The aim of this rotation is to tighten the rod connections and to reduce skin friction.

Every dynamic probing equipment has its torque measuring system which usually involves a torque wrench or similar device. Rods are rotated by means of this device, meaning that the value of the maximum torque is obtained at the same time.

The frictional force \( F_f \), necessary to rotate the drive rods, is calculated through the skin resistance and the lateral area of the drive rods.

\[
F_f = \tau \cdot 2 \cdot \pi \cdot r \cdot l \quad (4)
\]

where \( \tau \) represents skin friction resistance, \( r \) is the rod radius, and \( l \) is the rod length.

The maximum torque necessary to rotate the drive rods will be the frictional force multiplied by the rod radius.

\[
T = F_f \cdot r = \tau \cdot 2 \cdot \pi \cdot r^2 \cdot l \quad (5)
\]

where \( T \) is the maximum torque.

The value of the skin friction is thus calculated by Eq. (6) below:

\[
\tau = \frac{T}{2 \cdot \pi \cdot r^2 \cdot l} 
\]

(6)

The energy used to overcome the friction between the drive rods and the soil around them during penetration (frictional energy \( E_f \)) can be calculated by means of Eq. (7):

\[
E_f = F_f \cdot p
\]

(7)

Substituting the value of \( F_f \) from Eq. (4) in Eq. (7):

\[
E_f = \tau \cdot 2 \cdot \pi \cdot r \cdot l \cdot p
\]

(8)

In addition, by using the value of the skin friction from Eq. (6), we get the final Eq. (9):

\[
E_f = \frac{T \cdot p}{r}
\]

(9)

This Eq. (9) was obtained by Dahlberg and Bergdahl (1974).
The aforementioned equation only works when assuming that quasi-static skin friction during rotation of the drive rods is the same as the dynamic skin friction during penetration.

Due to this difference between skin friction in static and dynamic processes, Bergdahl (1979) calculated the part of the total number of blows in a penetration test that was “used” to overcome that lateral friction (Nixon 1988; Scarff 1988). Bergdahl (1979) then measured the energy due to lateral friction and found that it was the same energy shown in Eq. (9), but with an empirical correction factor of 3.4. This means that dynamic lateral friction resistance in a vertical direction during driving is much greater than semi-static resistance in a horizontal direction during the turning of the rods. When calculating that factor, Bergdahl (1979) was assuming 100% energy efficiency, meaning that in this research it will be necessary to multiply that energy by the energy efficiency factor (η).

We can thus consider the next equation to arrive at that frictional energy:

\[ E_f = \frac{T \cdot p}{r} \cdot 3.4 \cdot \eta \]  

(10)

where η represents the energy efficiency factor.

This efficiency means the percentage out of the nominal energy that reaches down to the cone.

Nominal energy is defined as the hammer weight multiplied by the height of fall.

\[ E_f = \frac{T \cdot p}{r} \cdot 3.4 \cdot \frac{E_{\text{sampler}}}{m_h \cdot g \cdot h} \]  

(11)

**ENTHRU_{cone} in DP tests**

In the rest of this paper, **ENTHRU_{cone}** is defined as the real amount of energy that effectively reaches the cone in DP tests (Ibanez et al. 2012). It is the equivalent to what Odebrecht et al. (2005) called **E_{sampler}** in a SPT test.

By using the **E_{sampler}** in a SPT (Eq. (2)) as part of the new **ENTHRU_{cone}**, it is necessary to subtract that part of the **ENTHRU** that is used to overcome that skin friction (\(E_f\)) from \(E_{\text{sampler}}\) (Matsumoto et al. 2015). Equation (12) is thus obtained:

\[ \text{ENTHRU}_{\text{cone}} = \eta_3 \cdot \left[ \eta_1 \cdot m_h \cdot g \cdot (h + p) + \eta_2 \cdot m_r \cdot g \cdot p \right] - E_f \]  

(12)

By substituting the value of \(E_f\) (Eq. (11)) in Eq. (12), and knowing that \(\eta_2\) equals 1:

\[ \text{ENTHRU}_{\text{cone}} = \eta_3 \cdot \left[ \eta_1 \cdot m_h \cdot g \cdot (h + p) + m_r \cdot g \cdot p \right] \cdot \left(1 - \frac{T \cdot p \cdot 3.4}{r \cdot m_h \cdot g \cdot h}\right) \]  

(13)

The value of \(m_r\) is the mass of the rods, although it would be better to also include the anvil mass. Odebrecht et al. (2005) took into consideration the additional potential energy delivered to the cone due to the weight resting on the sampler (cone for DP tests) along the penetration path. In DP testing, not only the mass of the rods and the mass of the hammer acts on the cone, but also the anvil rests on the cone, meaning that the anvil mass has to be included. In this regard, if there were any mass additional to the hammer mass, rod mass, or anvil mass that was resting on the cone during penetration, this should be also included in the anvil mass. Equation (13) can thus be rewritten:

\[ \text{ENTHRU}_{\text{cone}} = \eta_3 \cdot \left[ \eta_1 \cdot m_h \cdot g \cdot (h + p) + m_a \cdot g \cdot p + m_r \cdot g \cdot p \right] \cdot \left(1 - \frac{T \cdot p \cdot 3.4}{r \cdot m_h \cdot g \cdot h}\right) \]  

(14)

where \(m_a\) is the mass of the anvil where the hammer strikes when falling and \(\eta_3\) represents the energy efficiency factor. This different way of measuring **ENTHRU** makes \(\eta_1\) different to \(\eta_4\), as Odebrecht et al. (2005) calculated \(\eta_4\) by means of the **ENTHRU**, disregarding the value of \(m_a\). Therefore, in dynamic probing (DP tests), we cannot assume the value of \(\eta_4\) as 0.765 as Odebrecht et al. (2005) stated.

In compliance with ISO-AENOR-CEN (2005, 2011), as can be seen in Fig. 3, **ENTHRU** (Eq. (15)) is measured just below the anvil but at a distance a little greater than 10 times the rod diameter below the point of hammer impact:

\[ \text{ENTHRU} = \eta_4 \cdot m_h \cdot g \cdot (h + p) + m_a \cdot g \cdot p \]  

(15)

Schmertmann and Palacios (1979) found that the hammer remains in contact with the rods for only about 25% of the penetration (Schmertmann 2007). Equation (15) does not include this correction. This is not a real problem in this research, as **ENTHRU** will actually be measured through monitoring.

Equation (14) can be rewritten as:

\[ \text{ENTHRU}_{\text{cone}} = \eta_3 \cdot \left(\text{ENTHRU} + m_r \cdot g \cdot p\right) \cdot \left(1 - \frac{T \cdot p \cdot 3.4}{r \cdot m_h \cdot g \cdot h}\right) \]  

(16)

The value of \(\eta_3\) obtained from Eq. (3) will be changed. It would be better if this factor were expressed in non-dimensional form.

If the last part of this factor is multiplied and divided by the rod radius:

\[ \eta_3 = 1 - 0.0042 \cdot l \cdot \frac{T}{r} \]  

(17)

The rods used by Odebrecht et al. (2005) had the normal radius for SPT (1.14 × 10^{-2} m).
This way of computing $\eta_3$ is such that it has been assumed that the greater radius of rod, the greater the stiffness of the rod stem, resulting in greater energy efficiency. This research has also been performed by using values taken directly from Eq. (3), with fairly comparable results and conclusions that are identical to this research thus obtained. Hence, it is preferable to use this factor, as shown in Eq. (18).
\[ \text{ENTHRU}_{\text{cone}} \] is therefore finally obtained as:

\[ \text{ENTHRU}_{\text{cone}} = \left( 1 - 4.8 \cdot 10^{-5} \cdot \frac{L}{r} \right) \cdot \left( \text{ENTHRU} + m_r \cdot g \cdot h \right) \cdot \left( 1 - \frac{T \cdot p \cdot 3.4}{r \cdot m_h \cdot g \cdot h} \right) \]  

(19)

**Energy threshold**

The current formulas (see Eq. (20)) used to correlate results among the different dynamic penetration tests establish an inverse linear correlation between nominal energy and the number of blows \((N)\). This equation has been proposed in the ESOPT Symposium in 1974 (originating from the Dutch Formula (Bergdahl 1979) and Hiley’s Formula (Hiley 1925)) and is currently used in geotechnical engineering (Avanzi et al. 2013). This means that it may be possible to compensate a very little amount of energy in each blow by means of a large number of blows in order to achieve the same penetration. However, it is obvious that by using an extremely small amount of energy per blow, it is not possible to cause any penetration at all.

\[ N_1 = \frac{m_2 \cdot g \cdot h_2 \cdot A_1 \cdot e_1}{m_1 \cdot g \cdot h_1 \cdot A_2 \cdot e_2} \]  

(20)

where \(A\) is the cross-sectional area of the cone and \(e\) is the total penetration during the test. This Eq. (20) relates results from two different DP tests: 1 and 2 (used as subscripts).

If possible, this Eq. (20) should be better used in terms of ENTHRU, instead of by using nominal energies (Daniel et al. 2003).

Some studies (Daniel et al. 2005; Danziger et al. 2006) argue that some secondary impact may still be able to contribute to cone penetration, while others saw different results from their research with no further penetration due to secondary impacts (Schmertmann and Palacios 1979). In fact, a number of more recent studies (Lee et al. 2014, 2009) have concluded that some secondary impact and subsequent events, such as low energy impacts, do not produce further penetration.

It is assumed that it is necessary to exceed a certain level of energy to produce penetration (Ibanez 2009), referred to as the “energy threshold” \((T_h)\). This idea was originally conceived by Sanchez-Alciturri, J.M. during his mentoring for a doctorate (Ibanez 2009). Similar results, in terms of force (Soriano 1994), establish that a force (of the compression wave) greater than a certain value (half the dynamic resistance of a soil) to really cause penetration is required.

The Hiley formula (Hiley 1925) for pile capacity also states that there is a resistance to penetration called the ultimate driving resistance of the soil \((R_o)\).

Thus, a new term is defined: \(\text{ENPEN}\), which is the ENergy that actually produces non-recoverable PENetration into the soil:

\[ \text{ENPEN} = \text{ENTHRU}_{\text{cone}} - T_h \]  

(21)

If a single penetration test is performed and all the recorded blows are compared with each other, penetration \(p\) in terms of depth \((p/C_N)\) has to be corrected. In other words, the penetration calculated in this way is the penetration obtained if all the blows had been performed at the same depth (at which the effective vertical stress approximately equals 100 kPa). The overburden correction factor \((C_N)\) used was proposed by Liao and Whitman (1986). We shall therefore use this corrected penetration \((p^*)\):

\[ p^* = \frac{p}{C_N} \]  

(22)

If \(T_h\) existed and there were a linear relationship between \(\text{ENTHRU}_{\text{cone}}\) and corrected penetration \((p^*)\) beyond that \(T_h\) (conclusion that can be directly derived from Eq. (20)), it would be easy to calculate this \(T_h\). If a large amount of data from blows in the same homogeneous soil is plotted in terms of \(\text{ENTHRU}_{\text{cone}}\) vs. \(p^*\), \(T_h\) would be the \(x\)-intercept (see Fig. 1). In other words, \(T_h\) would be the energy that, if it is not surpassed, will mean there is no penetration at all.

In fact, this is just further evidence of the widely accepted non-linearity of soil behavior for very small strains. As will be shown later, behavior in the range of usually applied energies is roughly linear, with this non-linearity only appearing for extremely low values of \(\text{ENTHRU}\).

In the same way as the energy efficiency in dynamic penetration tests depends on the stiffness of the soil, as has been demonstrated in the latest research (Look et al. 2015; Matsumoto et al. 2015; Žaržojus et al. 2013), this \(T_h\) would also be related to this stiffness.

**Physical analysis of the energy threshold**

At every blow, there is a force \((F)\) that acts on the cone against the soil below. This \(F\) produces a penetration, or displacement of the cone \((s)\), which consists of an elastic displacement and a plastic displacement. This latter displacement is the vertical displacement measured by a caliper \((p)\). The elastic displacement just before failure shall be called \(s_p\). Figure 4 shows the behavior of a perfectly plastic model (Briaud 2013).

\[ \text{ENTHRU}_{\text{cone}} = \int F \cdot ds \]  

(23)
Considering the elastic behavior of the soil, and its stiffness \((k)\):

\[ F = k \cdot s \]  

(24)

If \( F < R_u \):

\[ \text{ENTHRU}_{cone} = \int k \cdot s \cdot ds = \frac{1}{2} \cdot k \cdot s^2 = \frac{1}{2} \cdot F^2 \]  

(25)

However, if \( F = R_u \), then there is a plastic displacement \((p)\):

\[ \text{ENTHRU}_{cone} = \frac{1}{2} \cdot \frac{R_u^2}{k} + R_u \cdot p \]  

(26)

The meaning of Eq. (26) is such that there is an energy threshold \((T_h)\), the value of which is:

\[ T_h = \frac{1}{2} \cdot \frac{R_u^2}{k} \]  

(27)

When the \( T_h \) is reached, \( \text{ENTHRU}_{cone} \) increases linearly with a slope \( R_u \). As we are going to use \( p^* \) instead of \( p \), the slope will be \( R_u \cdot C_N \), as shown in Eq. (22). As the result of this physical analysis, the \( \text{ENTHRU}_{cone}-p^* \) relationship is depicted in Fig. 5. In this case, the slope is the reciprocal of \( R_u \cdot C_N \). \( C_N \) is going to be calculated as the medium value at the considered depth under study.

**Field testing**

All the foregoing considerations show the need to validate those hypotheses by means of a broad field testing with real energy measurements.

More than one hundred blows were given using an instrumented rod in order to calculate the value of \( T_h \), according to Eq. (21) (the \( x \)-intercept in the graph \( \text{ENTHRU}_{cone} \) vs. \( p^* \), as can be seen in Figs. 1 and 5).

For each blow, the energy (\( \text{ENTHRU} \)), the depth, and the real penetration were measured. \( \text{ENTHRU}_{cone} \) was calculated by means of Eq. (19).

**Field site**

The field work was conducted at Arija, in the north of the province of Burgos, Spain. Figure 6 shows the location of this site and a detailed orthophoto.

The geotechnical profile at the site includes a 40-m-thick sedimentary layer of Holocene deposits, containing rock fragments, siliceous minerals with a sizeable quantity of feldspars, micas, some heavy minerals, iron oxides, and carbonates. The bedrock is formed by sandstone and siltstone clay from the Cretaceous period.
The upper 6.5 m of soil is a uniform clean silica sand (classified as SP, USCS), with a particle size distribution curve as shown in Fig. 7 and a bulk unit weight of 19.8 kN/m³. Below this level, the clay content increases.

These sands are of a great economical interest and they are dragged out from the river for construction and industrial purposes.

The soil surface was relatively horizontal and it was easy to access with all the equipment (a penetrometer on wheels and vehicle).

Geotechnical survey

The blows, the results of which will be used in this research, were performed at the location $P$ in Fig. 8, with a geotechnical survey designed to geotechnically characterize the site. A series of geotechnical in situ and laboratory tests were performed around $P$.

Around $P$, three penetration tests (DPSH-B) were also performed, forming an equilateral triangle whose sides are 3 m long (as can be seen in Fig. 8). The results of these DPSH-B tests ($N_{DPSH-B}$) can be also seen in Fig. 13.

Electrical resistivity tests were also performed because, in the authors’ opinion, it is important to get more information regarding the stratigraphic profile of the site (it was especially important to know where the clean and clayey sands were located). This research was originally designed to perform tests at different depths, meaning that it was considered indispensable to get more information on the layer profile to better choose at what depth the DP tests were going to be carried out. By means of these electrical profiles, it was decided to perform one of the sets of tests at a depth of below 6.5 m, where the clayey sands were located. Finally, in combination with the results from DPSH tests, it was decided to conduct one of the sets of tests at a depth of between 6.7 and 7.0 m, as will be explained later.

As a two-dimensional soil profile was required, the ERT (electrical resistivity tomography) method was chosen. An electrical resistivity test was therefore conducted passing through $P$, using a Schlumberger-Wenner configuration. The geometry of the electrodes defines the array, of which there are several kinds, with the most common being the dipole–dipole and Schlumberger-Wenner configurations. For the expected soil profile (approximately horizontal layers),
Fig. 8 Geotechnical survey (Aerial photos from SIGPAC, Fondo Español de Garantía Agraria O.A.—FEBA)
the Schlumberger-Wenner array is the one which offers the best results (Pazdirek and Blaha 1996).

The electrical resistivity survey was undertaken using a SYSCAL R1 PLUS Switch72 device, property of the University of Burgos. The electrical tests were designed with the processing software ELECTRE (a total of 1,313 electrical measurements were conducted along the cross section). After rejecting invalid quadripole data, the apparent electrical resistivity cross section was plotted using PROSYS II processing software. RES2DINV interpretation software was used for inversion of the 2D ERT data for calculating real electrical resistivity cross sections (Fig. 9).

As a high density of data was preferred to going very deep, electrode spacing used was small (1 m) (Fig. 9 Profile a). The test was repeated at the central part, closer to \( P \), but with a smaller electrode spacing (0.5 m) (Fig. 9 Profile b) to get even a higher resolution near \( P \).

The results from the electrical resistivity tests are shown in Fig. 9. The geological interpretation is such that the shallower part of the soil consists of a layer of clean sands. Below a depth of approximately 5.5–6.5 m, the electrical resistivity of the sands becomes smaller. It is interpreted as the sand increases its clay content.

By trying to better characterize these shallower clean sands, two direct shear tests were also performed, with the sand compacted to the real in situ unit weight. The values of the critical and peak shear strength are shown in Fig. 10. The failure envelope for either critical (\( \phi = 29^\circ \)) and peak (\( \phi = 35^\circ \)) values is also depicted.

Direct shear tests were carried out on these dilatant sands until the ultimate condition was reached at large strains.

After the peak, deformations occurred, with a quite appreciable softening. When there was not any further change of volume, the specimen reached the critical condition. These values of shear stresses at large deformations are the critical values, which define the critical state line (Lancellota 1995).

The depth of the ground water level varied from 0.4 to 0.9 m during the period of the field campaign.

**Dynamic penetration apparatus and adjustments**

The field campaign was carried out using DPSH-B equipment (a Sunda Menhir 100 kN-DPSH penetrometer), with a standard \( r \) value (1.14 cm). The cross-sectional area of the cone (\( A \)) was 20.03 cm\(^2\).

This wheeled DP apparatus has to be transported by vehicle. In this case, an off-road vehicle was used (Fig. 11).

The Sunda penetrometer is prepared to be used for DPSH-B and DPSH-A (in such a case, just by taking down the hammer height magnetic controlling sensor a total of 25 cm). DPM tests can also be undertaken. In this case, not only the sensor has to be taken down 25 cm, but the hammer has to be replaced by a 50-kg hammer (also provided with the original penetrometer).

Pursuant to ISO-AENOR-CEN (2005, 2011), the instrumented section was located at a distance of 32.6 m from the top of the anvil (greater than ten times the diameter of the bars). A schematic figure of penetrometer can be seen in Fig. 11.

The aim of this research is to perform different DP tests and plot their results in figures such as Figs. 1 and 5 in order to find out if \( T_h \) really exists or not. There is therefore...
a need to perform tests in which different $ENTHRU_{cone}$ are obtained. Due to the latter, the penetrometer was adapted for being able to perform tests in which the nominal energies could be easily changed.

Nominal energies can be transformed either by changing the height of fall or the mass of the hammer.

In such a way, the dynamic penetrometer was first modified in order to be able to change the height of fall. This was fairly easy, as it just consisted of changing the location of a magnetic sensor, by drilling the support bar where the sensor is attached (see Fig. 12). In this manner, the height of fall of the hammer could be chosen from 40 to 75 cm (every 5 cm).

The original hammer was then replaced by another smaller hammer and a series of additional steel plates that could be bolted to the hammer or not, as can be seen in Fig. 12. This new hammer and steel plates should fulfill some geometrical requirements of the penetrometer architecture.

By combining and assembling different number of these steel plates, we found hammer mass ranging from approximately 40 to 70 kg. It is important to state that, due to the geometrical circumstances of the penetrometer and because of manufacturing issues when making the steel plates, no round figures regarding hammer mass were obtained (hammer mass varied from a minimum value of 42.85 kg to a maximum value of 69.16 kg). The only criteria to be satisfied was to get hammer mass into this approximate range and mainly to know exactly the hammer mass and the height of fall in order to accurately calculate the nominal energy and then the $ENTHRU_{cone}$. It was not at all important to get round figures.
These possible hammer mass and height of fall values led us to nominal energies varying from approximately 150 to 500 J. As explained before, this is of the order of magnitude of usual DP tests in geotechnical practice (ISO-AENOR-CEN 2005, 2011; Matsumoto et al. 2015).

The penetrometer had also to be modified by adding a pair of nylon pieces in order to avoid the instrumented section being placed inside the hole that the penetrometer has in its frame (see Fig. 12).

**Instrumentation and monitoring**

The upper part of the rods was instrumented using four strain gauges and two accelerometers as can be seen in Figs. 13 and 14.

The instrumentation was located $32.6 \times 10^{-2}$ m below the point of contact between the hammer and the anvil. This length was chosen in compliance with ISO-AENOR-CEN (2005, 2011). It is necessary that the instrumented section of rod be positioned at a distance greater than ten times the rod diameter below the point of hammer impact on the anvil.

Each of the four strain gauges was fixed and attached to the rod and was independent from the rest of the strain gauges. They were assembled as four different quarter Wheatstone bridges.

The two ICP piezoelectric accelerometers were mounted diametrically opposite on small steel pieces that were bolted to the rod. The accelerometers were suitable for an acceleration of up to 10,000 g.

The signal conditioner/amplifier used in this research was the SCADAS III SC 316 front-end system signal acquisition equipment (from the LMS Difa Instruments Company).

**Blows performed**

The blows were performed in the shallowest layer of the soil where there were clean sands which were properly characterized. The blows were conducted at a very homogeneous part of the soil at two different depths: Zone 1 (from 2.5 to 3.2 m) and Zone 2 (from 4.6 to 5.2 m). Even though the clayey sands below 5.5–6.5 m (see soil resistivity profiles from Fig. 9) were not so well-identified and characterized, it was also decided to perform blows at these clayey sands in order to find out if the calculation of $T_h$ depends on the type of soil, or not. The blows in these clayey sands were also conducted at a homogeneous part of this layer, i.e., Zone
3 (from 6.7 to 7.0 m). These three different depths were chosen after the geotechnical and geophysical survey, with the three zones highlighted in the DPSH profiles shown in Fig. 15.

Since energy and penetration were needed to be obtained for every impact, a single blow (impact event) is defined as one complete test in this research.

At every impact event, acceleration and force were measured directly and independently, with the energy effectively transferred to the rods calculated as will be explained later.

In order to achieve $T_p$, as explained before, it is necessary to compare blows with different $ENTHRU_{cone}$. To this end, the nominal energy was changed from blow to blow. The mass of the hammer varied from a minimum value of 42.85 kg to a maximum value of 69.16 kg. The height of fall changed from 40 to 75 cm.

The values of the maximum torque for rod friction control $T$, measured at the three depths, were 5 N m, 13 N m, and 28 N m respectively. As expected, the greater the depth, the bigger the values of the maximum torque measured, due to the friction between the rods and the soil around them. These $T$ values had to be used when calculating the $ENTHRU_{cone}$ as part of the input of Eq. (19).

**Results and discussion**

The importance of the processing of the signals from strain gages and accelerometers to get satisfactory results should be mentioned. Therefore, prior to the final results, we should explain and discuss some issues related to monitoring signals and their processing.

**Interpretation and discussion of strain and acceleration signals**

The first uncertainty, related to the signals from sensors, was to know if the signals registered were correct or not. That is why, besides the fact that the strain gauges and the accelerometers came ready-calibrated from the factory, before the tests, the whole of the monitoring assembly, including the sensors and the signal acquisition equipment, was fully calibrated by the authors in order to know if the measurements were correct. The measurements from the accelerometers

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**Fig. 14** Instrumented portion of the rod string with the strain gauges and the accelerometers

**Fig. 15** DPSH-B penetration profiles
were calibrated by using a Brüel & Kjaer 4294/WH2606 calibration exciter, while the measurements from the strain gauges were calibrated by testing the instrumented rod in a loading frame via a compression test.

The signal conditioner/amplifier used in this research was SCADAS III signal acquisition equipment, model SC 316 front-end system (LMS Difa Instruments Company).

It was deemed important to connect the acquisition equipment and sensors in a way that minimized the noise signal. To ensure this, oxygen-free shielded cables were used, along with DB9 and LEMO connectors.

An anti-aliasing filter (low pass filter) was also used to minimize noise signal.

LMS Test.Lab 7B data acquisition software was used with a Signature Acquisition-RT interface.

Design oversampling digital data conversion at a rate of 25,600 Hz (sampling frequency) was employed with the final representation of the data at a rate of 10,000 Hz (frequency span). This was the goal for the rate, as sensors showed linear behavior up to this rate.

Data acquisition was designed to start 0.1 s before the impact where measured acceleration was greater than 50 g (noise signal could be up to 30 g). For acquisitions 0.1 s before the first impact, a pre-trigger had to be defined by the software.

One example of the records of these data is shown in Fig. 16 (blow 11: 2.64 m deep). The first impact is clearly visible in the direct records of force and acceleration, at a time of about 0.1 s, with a clear increase of about 300 J of the calculated transferred energy. At a time of about 0.15 s, a secondary (rebound) impact of much lower intensity is noticed, particularly in the acceleration.

The force was calculated using the elastic model (Eq. (28)), by averaging the value of the strain from the four strain gauges.

\[ F = E \cdot \epsilon \cdot A \]  

where \( E \) represents the steel bar elastic modulus and \( \epsilon \) is the strain.

The velocity was calculated by integrating (Eq. (29)) the acceleration signal from accelerometers in the time domain (an average value from the two accelerometer signals).

\[ v = \int_0^t a \cdot dt \]  

where \( a \) is the acceleration and \( t \) is the time.

**Measuring energy and penetration**

The method used in this research for measuring the \( ENTHRU \) was the FV, which measures the \( EFV \) (energy transmitted to the drill rod from the hammer during the impact event):

\[ ENTHRU = \max \left[ \int F(t) \cdot v(t) \cdot dt \right] \]  

This is the method proposed in current standards (ASTM 2016; ISO-AENOR-CEN 2005, 2011).

The FV method is recommended instead of other methods that were used in the past, as many authors have proposed (Abou-Matar and Goble 1997; Butler et al. 1998; Farrar 1998; Sancio and Bray 2005; Schmertmann 2007; Sy and Campanella 1991).

It is important to note that \( ENTHRU \) is the maximum value of Eq. (30). \( ENTHRU \) is really calculated by integrating twice: firstly to get velocity from acceleration and secondly to calculate \( ENTHRU \) by applying Eq. (30). This mathematical issue can cause significant errors, mostly at the latter part of the records if there is noise signal. It is therefore essential to check every \( ENTHRU \) calculation. If \( ENTHRU \) does not remain as a constant value after the first and secondary impacts, the maximum value has to be computed after, while remaining close to the secondary impact.

Penetration was measured by means of a caliper, measured from a fixed part of the equipment (the method of integrating twice from acceleration values was discarded because of the increasing mathematical error).

The blows were performed with special care, as there are many variables that can adversely affect measured \( ENTHRU \), such as lack of verticality, poor contact at joints, etc. (Reading et al. 2010), and the frequency of hammer blows (Seed et al. 1985).

Every integral was calculated using MATLAB software, aggregating the integrand in the time domain. By considering a discretization, the integral was transformed into an aggregation of finite addends (obtained from the values of the instrumented signals).

**Results**

The results from the blows are shown in Tables 1, 2, and 3.
The value of $T_h$ was calculated at these three different depths, in line with the previously explained. A statistical analysis is also required in order to verify the main hypothesis in this research, i.e., the existence of an apparent $T_h$ if a linear correlation is used to compare results from various kinds of penetration tests.

| Blow | Depth (m) | $m_h$ (kg) | $h$ (cm) | $p$ (mm) | $E_{nh}$ (J) | $ENTHRU$ (J) | $ENTHRU_{cone}$ (J) | $ENTHRU_{cone}/E_{nh}$ | $p^*$ (mm) |
|------|-----------|------------|----------|----------|--------------|--------------|----------------------|------------------------|-------------|
| 1    | 2.48      | 69.2       | 75       | 29.0     | 509          | 341          | 324                  | 63.7%                  | 15.9        |
| 2    | 2.51      | 63.9       | 75       | 20.0     | 470          | 340          | 326                  | 69.3%                  | 11.0        |
| 3    | 2.53      | 63.9       | 70       | 17.6     | 439          | 310          | 298                  | 67.9%                  | 9.7         |
| 4    | 2.54      | 63.9       | 65       | 16.4     | 408          | 297          | 285                  | 69.9%                  | 9.1         |
| 5    | 2.56      | 63.9       | 60       | 15.0     | 376          | 273          | 262                  | 69.7%                  | 8.3         |
| 6    | 2.58      | 63.9       | 55       | 14.2     | 345          | 239          | 229                  | 66.4%                  | 7.9         |
| 7    | 2.59      | 63.9       | 50       | 12.4     | 314          | 219          | 210                  | 67.0%                  | 6.9         |
| 8    | 2.60      | 63.9       | 45       | 11.3     | 282          | 196          | 194                  | 68.8%                  | 6.3         |
| 9    | 2.61      | 63.9       | 40       | 10.2     | 251          | 175          | 168                  | 67.0%                  | 5.7         |
| 10   | 2.63      | 63.9       | 35       | 9.9      | 239          | 168          | 161                  | 67.9%                  | 5.2         |
| 11   | 2.64      | 63.9       | 30       | 9.2      | 230          | 157          | 150                  | 67.8%                  | 4.7         |
| 12   | 2.66      | 63.9       | 25       | 8.5      | 222          | 147          | 140                  | 67.7%                  | 4.3         |
| 13   | 2.67      | 63.9       | 20       | 7.7      | 214          | 138          | 130                  | 67.6%                  | 4.1         |
| 14   | 2.69      | 63.9       | 15       | 7.0      | 206          | 128          | 120                  | 67.5%                  | 3.9         |
| 15   | 2.70      | 63.9       | 10       | 6.3      | 198          | 120          | 120                  | 67.4%                  | 3.7         |
| 16   | 2.71      | 63.9       | 5       | 5.6      | 184          | 108          | 108                  | 67.3%                  | 3.5         |
| 17   | 2.72      | 63.9       | 0       | 5.0      | 182          | 96           | 96                   | 67.2%                  | 3.3         |
| 18   | 2.73      | 63.9       | -       | 4.5      | 180          | 90           | 90                   | 67.1%                  | 3.1         |

Table 1: Experimental results at depths from 2.5 m to 3.2 m (Zone 1)
Values of energy threshold

At every aforementioned zone, the results were plotted in a separate graph, as shown in Figs. 17, 18, and 19. First blows were performed using many different nominal energy values, which is why, in Fig. 17, there are tests with many different \( ENTHRU_{\text{cone}} \) values. For operative reasons, the next blows were mostly conducted by using only four different nominal energies, which explains why, at deeper locations and especially in Fig. 19, most of the points are concentrated closer together at certain ranges.

\( T_h \) is the \( x \)-intercept of these correlation lines of best fit in Figs. 17, 18, and 19. The results can be seen in Table 4.

It is important to remark that there is not a single, universal value for this threshold: \( T_h \) depends on the nature of the soil and the type of DP test (regarding the cross-sectional area of the cone).

Analysis of the elastic stiffness of the soil

The value of \( k \) was calculated for the three different zones under study, according to Eq. (27) and Fig. 5. These results are summarized in Table 5.

By knowing the value of \( C_N \), the ultimate driving resistance \( (R_u) \) was calculated (see Table 5). Figure 20 shows the values of \( R_u \) for the three studied zones. As expected, there is an apparent tendency to increase with the soil penetration resistance \( (NDPSH-B) \), regardless of the depth (the influence of the depth has been eliminated by using the factor \( C_N \) (Eq. (22)).

The value of \( k \) is similar for the clean sand (Zones 1 and 2) and higher for the clayey sand (Zone 3).

Statistical analysis

The theoretical equation for the linear model in the regression analysis that has been done is as follows (see Figs. 1 and 5):

\[
p = m_E \cdot ENTHRU_{\text{cone}} - m_E \cdot T_h
\]

(31)

where \( m_E \) is the slope and \( -m_E \cdot T_h \) is the y-intercept in this linear model.

This factor \( m_E \), which is the incremental ratio between penetration and effective energy, is a measure of the reciprocal of \( R_u \cdot C_N \) (see Fig. 5).

### Table 2: Experimental results at depths from 4.6 to 5.2 m (Zone 2)

| Blow | Depth (m) | \( m_h \) (kg) | \( h \) (cm) | \( p \) (mm) | \( E_p \) (J) | \( ENTHRU \) (J) | \( ENTHRU_{\text{cone}} \) (J) | \( ENTHRU_{\text{cone}}/E_p \) p* (mm) |
|------|-----------|----------------|-------------|-------------|-------------|----------------|-----------------------------|---------------------------------|
| 1    | 4.59      | 63.9           | 75          | 19.2        | 470         | 338            | 301                         | 64.1%                           | 14.3                           |
| 2    | 4.60      | 42.9           | 40          | 6.6         | 168         | 143            | 128                         | 76.1%                           | 4.9                            |
| 3    | 4.79      | 63.9           | 75          | 17.8        | 470         | 306            | 275                         | 58.5%                           | 13.5                           |
| 4    | 4.80      | 42.9           | 75          | 11.5        | 315         | 237            | 214                         | 67.7%                           | 8.7                            |
| 5    | 4.82      | 63.9           | 75          | 18.3        | 470         | 313            | 281                         | 59.6%                           | 13.9                           |
| 6    | 4.83      | 63.9           | 40          | 10.8        | 251         | 187            | 165                         | 65.9%                           | 8.2                            |
| 7    | 4.85      | 63.9           | 75          | 19.1        | 470         | 350            | 312                         | 66.2%                           | 14.6                           |
| 8    | 4.86      | 42.9           | 40          | 7.2         | 168         | 133            | 118                         | 70.2%                           | 5.5                            |
| 9    | 4.87      | 63.9           | 75          | 19.7        | 470         | 352            | 312                         | 66.4%                           | 15.1                           |
| 10   | 4.89      | 42.9           | 75          | 12.4        | 315         | 246            | 219                         | 69.6%                           | 9.5                            |
| 11   | 4.91      | 63.9           | 75          | 19.4        | 470         | 342            | 304                         | 64.7%                           | 14.9                           |
| 12   | 4.92      | 63.9           | 40          | 11.4        | 251         | 199            | 175                         | 69.6%                           | 8.8                            |
| 13   | 4.94      | 63.9           | 75          | 19.4        | 470         | 338            | 300                         | 63.9%                           | 14.9                           |
| 14   | 4.95      | 42.9           | 40          | 7.0         | 168         | 135            | 120                         | 71.3%                           | 5.4                            |
| 15   | 4.96      | 63.9           | 75          | 18.6        | 470         | 337            | 301                         | 64.0%                           | 14.4                           |
| 16   | 4.98      | 42.9           | 75          | 12.0        | 315         | 243            | 218                         | 69.1%                           | 9.3                            |
| 17   | 4.99      | 63.9           | 75          | 18.5        | 470         | 325            | 290                         | 61.8%                           | 14.3                           |
| 18   | 5.01      | 63.9           | 40          | 11.0        | 251         | 188            | 166                         | 66.3%                           | 8.5                            |
| 19   | 5.02      | 63.9           | 40          | 10.6        | 251         | 188            | 167                         | 66.6%                           | 8.2                            |
| 20   | 5.03      | 63.9           | 75          | 18.3        | 464         | 353            | 315                         | 67.9%                           | 14.2                           |
| 21   | 5.05      | 63.9           | 75          | 18.3        | 470         | 347            | 310                         | 66.0%                           | 14.2                           |
| 22   | 5.07      | 63.9           | 70          | 17.5        | 439         | 321            | 287                         | 65.3%                           | 13.6                           |
| 23   | 5.09      | 63.9           | 65          | 16.9        | 408         | 297            | 264                         | 64.8%                           | 13.2                           |
| 24   | 5.10      | 63.9           | 60          | 16.2        | 376         | 291            | 257                         | 68.3%                           | 12.7                           |
| 25   | 5.15      | 63.9           | 55          | 12.0        | 345         | 261            | 236                         | 68.5%                           | 9.4                            |
| 26   | 5.17      | 63.9           | 50          | 9.5         | 314         | 230            | 210                         | 67.0%                           | 7.5                            |
| 27   | 5.19      | 63.9           | 45          | 7.8         | 282         | 205            | 189                         | 66.9%                           | 6.1                            |
$T_h$ is the $x$-intercept (see Figs. 1 and 5).

When using a linear trending line to interpolate, as in this case, it is easy to calculate the statistical parameters of the slope and the $y$-intercept. In every line, the $x$-intercept ($T_h$), the slope ($m_E$), and the $y$-intercept ($-m_E \cdot T_h$) fulfil the next relationship:

$$x - \text{intercept} = -\frac{y - \text{intercept}}{\text{slope}}$$

(32)

The sampling standard deviation of the $x$-intercept, i.e., the standard deviation of $T_h$ ($S_{T_h}$), can therefore be expressed in terms of the sampling variances and the mean values of the slope ($m_E$) and the $y$-intercept ($-m_E \cdot T_h$):

$$S_{T_h} = \frac{1}{T_h} \sqrt{\frac{S^2_{-m_E \cdot T_h}}{-m_E \cdot T_h^2} + \frac{S^2_m}{m_E^2}}$$

(33)

Table 3. Experimental results at depths from 6.7 to 7.0 m (Zone 3)

| Blow | Depth (m) | $m_h$ (kg) | $h$ (cm) | $p$ (mm) | $E_s$ (J) | $ENTHRU$ (J) | $ENTHRU_{cone}$ (J) | $ENTHRU_{cone}/E_n$ | $p^*$ (mm) |
|------|-----------|------------|----------|---------|-----------|-------------|---------------------|--------------------|-----------|
| 1    | 6.73      | 63.9       | 75       | 9.6     | 470       | 366         | 318                 | 67.6%              | 8.6       |
| 2    | 6.74      | 42.9       | 40       | 3.6     | 168       | 142         | 123                 | 73.1%              | 3.2       |
| 3    | 6.74      | 63.9       | 75       | 9.1     | 470       | 355         | 310                 | 66.0%              | 8.1       |
| 4    | 6.75      | 42.9       | 75       | 5.5     | 315       | 253         | 224                 | 71.1%              | 4.9       |
| 5    | 6.76      | 63.9       | 75       | 8.6     | 470       | 351         | 309                 | 65.7%              | 7.7       |
| 6    | 6.76      | 63.9       | 40       | 4.9     | 251       | 202         | 176                 | 70.2%              | 4.4       |
| 7    | 6.77      | 63.9       | 75       | 8.4     | 470       | 361         | 318                 | 67.7%              | 7.5       |
| 8    | 6.78      | 42.9       | 40       | 2.9     | 168       | 144         | 127                 | 75.7%              | 2.6       |
| 9    | 6.78      | 63.9       | 75       | 8.1     | 470       | 348         | 309                 | 65.6%              | 7.2       |
| 10   | 6.79      | 42.9       | 75       | 5.0     | 315       | 254         | 227                 | 71.9%              | 4.5       |
| 11   | 6.80      | 63.9       | 75       | 8.5     | 470       | 353         | 311                 | 66.2%              | 7.6       |
| 12   | 6.80      | 63.9       | 40       | 4.3     | 251       | 209         | 185                 | 73.7%              | 3.9       |
| 13   | 6.81      | 63.9       | 75       | 8.0     | 470       | 351         | 312                 | 66.2%              | 7.2       |
| 14   | 6.81      | 42.9       | 40       | 2.7     | 168       | 143         | 127                 | 75.7%              | 2.4       |
| 15   | 6.82      | 63.9       | 75       | 7.7     | 470       | 355         | 316                 | 67.2%              | 6.9       |
| 16   | 6.83      | 42.9       | 75       | 5.0     | 315       | 255         | 228                 | 72.2%              | 4.5       |
| 17   | 6.83      | 63.9       | 75       | 7.6     | 470       | 353         | 315                 | 67.0%              | 6.8       |
| 18   | 6.84      | 63.9       | 40       | 4.6     | 251       | 204         | 180                 | 71.6%              | 4.1       |
| 19   | 6.84      | 63.9       | 75       | 7.4     | 470       | 358         | 320                 | 68.1%              | 6.6       |
| 20   | 6.85      | 42.9       | 40       | 2.6     | 168       | 146         | 130                 | 77.6%              | 2.3       |
| 21   | 6.85      | 63.9       | 75       | 7.4     | 470       | 354         | 316                 | 67.3%              | 6.7       |
| 22   | 6.86      | 42.9       | 75       | 4.9     | 315       | 262         | 235                 | 74.4%              | 4.4       |
| 23   | 6.87      | 63.9       | 75       | 7.4     | 470       | 355         | 317                 | 67.5%              | 6.7       |
| 24   | 6.87      | 63.9       | 40       | 4.3     | 251       | 206         | 182                 | 72.7%              | 3.9       |
| 25   | 6.88      | 63.9       | 75       | 7.4     | 470       | 351         | 314                 | 66.8%              | 6.7       |
| 26   | 6.88      | 42.9       | 40       | 2.7     | 168       | 145         | 130                 | 77.1%              | 2.4       |
| 27   | 6.89      | 63.9       | 75       | 7.3     | 470       | 352         | 315                 | 66.9%              | 6.6       |
| 28   | 6.89      | 42.9       | 75       | 4.9     | 315       | 253         | 226                 | 71.7%              | 4.4       |
| 29   | 6.90      | 63.9       | 75       | 7.2     | 470       | 347         | 311                 | 66.2%              | 6.5       |
| 30   | 6.91      | 63.9       | 40       | 4.0     | 251       | 205         | 183                 | 73.0%              | 3.6       |
| 31   | 6.91      | 63.9       | 75       | 7.2     | 470       | 354         | 317                 | 67.4%              | 6.5       |
| 32   | 6.92      | 42.9       | 40       | 2.7     | 168       | 142         | 127                 | 75.6%              | 2.4       |
| 33   | 6.92      | 63.9       | 75       | 7.2     | 470       | 346         | 310                 | 66.0%              | 6.5       |
| 34   | 6.93      | 42.9       | 75       | 4.7     | 315       | 236         | 212                 | 67.2%              | 4.2       |
| 35   | 6.93      | 63.9       | 75       | 7.2     | 470       | 349         | 312                 | 66.4%              | 6.5       |
| 36   | 6.94      | 63.9       | 40       | 4.0     | 251       | 201         | 180                 | 71.6%              | 3.6       |
| 37   | 6.95      | 63.9       | 75       | 7.2     | 470       | 353         | 317                 | 67.3%              | 6.5       |
where $S^2$ is the sampling variance (COV).

The correlation coefficient ($R$) of the linear model in this simple regression calculations and the summary of the results of the previous equations is shown in Table 6. Statgraphics Plus 4.0 software was used.

The $P$-value in the ANOVA analysis is 0.0000 for the three cases studied at the three depths considered.

**Discussion**

Since the $P$-value in the ANOVA analysis is less than 0.01, there is a statistically significant relationship between $ENTHRU_{cone}$ and $p/C_N$ at the 99% confidence level.

Similarly, the correlation coefficients ($R$), as seen in Table 6, indicate a strong relationship between the variables. Values of $R$ and $R^2$ are high enough, even though not any outlier has been discarded from this research, unlike some authors (Viviescas et al. 2019) that have proven to be reasonable to exclude some SPT outliers for researching. Obviously, discarding outliers would have even increased the values of $R$ and $R^2$.

The sampling standard deviation of $T_h$ ($S_{Th}$), which is not higher than the value of $T_h$, indicates that $T_h$ probably exists statistically, with a positive value.

The results from the tests are clear; the soil behaves as if it were needed to subtract $T_h$ from $ENTHRU_{cone}$ to properly correlate the real penetrations and the blow count between different penetration tests.

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**Table 4** Values of $T_h$

| Zone  | $T_h$ (J) |
|-------|-----------|
| Zone 1 | 37        |
| Zone 2 | 23        |
| Zone 3 | 22        |
where \( q \) is the applied pressure.

If we want to calculate the force:

\[ F = K \cdot s \cdot A \]  

Comparing to Eq. (24), we obtain the equation for calculating the soil elastic stiffness \((k)\):

\[ k = K \cdot A \]  

The value of \( K \) can be calculated by using (Muzás Labad 2002; Terzaghi 1955) the next equation (for submerged sands):

\[ K_{30} = 0.6 \cdot 10^{\frac{N_{SPT}+2}{3}} \]  

where \( K_{30} \) is the subgrade reaction coefficient corresponding to a 30 cm diameter and \( N_{SPT} \) is the number of blows obtained when conducted a SPT test.

In order to correlate \( N_{SPT} \) and \( N_{DPSh-B} \), we can apply this equation (Ibanez 2009):

\[ N_{SPT} = 2 \cdot N_{DPSh-B} \]  

\( K \) can then be calculated through the current formula for sands (Muzás Labad 2002; Terzaghi 1955):

\[ K = K_{30} \cdot \left( \frac{d_{c}(m) + 0.3}{2 \cdot d_{c}(m)} \right)^2 \]  

where \( d_{c} \) is the diameter of the cone in meters.

If we apply these last equations for the results obtained through this investigation, we obtain the values of \( R_{u}, k \), and then \( T_{h} \) (Eq. (27)). These results are shown in Table 7.

The results of \( T_{h} \) are enough similar to those of Table 5, so we can assume this manner of calculating \( T_{h} \) can be appropriate.

As was demonstrated (Figs. 17, 18, and 19), the soil actually behaves as if there were a linear relationship between energy and penetration beyond that \( T_{h} \), with the penetration closely related to such \( ENPEN \) (Eq. (21)). It would clearly be interesting to propose a new method for calculating the energy correction required to adjust the blow counts to 60% energy efficiency \((N_{60})\) by using a correction who includes this \( T_{h} \):

\[ N_{60} = \eta \cdot m_{h} \cdot g \cdot h - T_{h} \cdot N \]  

Table 5 Elastic stiffness \((k)\)

| Zone | \( N_{DPSh-B} \) | \( I/A \cdot R_{u} \cdot C_{N} \) | \( C_{N} \) | \( R_{u} \) (kN) | \( T_{h} \) (J) | \( k \) (kN/m) |
|------|-----------------|-------------------------------|-----------|--------------|--------------|------------|
| Zone 1 | 10.5 | 0.0364 | 1.727 | 16 | 37 | 3420 |
| Zone 2 | 8 | 0.0512 | 1.299 | 15 | 23 | 4915 |
| Zone 3 | 20.5 | 0.0239 | 1.113 | 38 | 22 | 32119 |

Fig. 20 Ultimate driving resistance \((R_{u})\) vs. Penetration resistance \((N_{DPSh-B})\)

Table 6 Statistical results

| Zone | \( T_{h} \) (J) | \( R \) | \( m_{E} \) | \( -m_{E} \cdot T_{h} \) | \( S_{nE} \) | \( S_{nE} \cdot T_{h} \) | \( S_{Th} \) (J) |
|------|----------------|--------|------------|-------------------------|------------|----------------------|----------|
| Zone 1 | 37 | 0.8992 | 0.0364 | -1.3311 | 0.0027 | 0.6718 | 19 |
| Zone 2 | 23 | 0.9629 | 0.0512 | -1.1684 | 0.0029 | 0.7089 | 14 |
| Zone 3 | 22 | 0.9595 | 0.0239 | -0.5321 | 0.0012 | 0.3060 | 13 |

Table 7 \( R_{u}, k \), and \( T_{h} \) calculation

| Zone | \( R_{u} \) (kN) | \( k \) (kN/m) | \( T_{h} \) (J) |
|------|----------------|--------------|----------|
| Zone 1 | 17 | 2925 | 49 |
| Zone 2 | 10 | 1090 | 48 |
| Zone 3 | 19 | 8403 | 22 |
Conclusions

In order to calculate penetrations in DP tests (as well as in SPT tests), it is necessary to use $ENTHRU_{cone}$ instead of $ENTHRU$, as the energy that really produces penetration is the fraction of the total energy that effectively reaches the tip of the cone below the rod string.

The way of measuring energy in dynamic probing (DP tests) is different from that used in SPT tests, which is mainly due to the existence of skin friction between the drive tip of the cone below the rod string.

In order to calculate $ENTHRU_{cone}$ in DP tests, a new equation (Eq. (19)) is proposed while to calculate $ENTHRU_{cone}$ correctly, it is necessary to measure $ENTHRU$ using an instrumented rod. In DP tests, it is not enough accurate to calculate $ENTHRU$ by means of equations similar to Eq. (15), because of the additional difficulty of calculating the possible values of energy efficiency factors.

It has been proved that soil actually behaves as if there were a certain amount of energy ($T_a$) to be exceeded by $ENTHRU_{cone}$ to produce penetration. For the usual energy values used in DP tests (the range of real interest), the soil behaves as if there were a linear relationship between energy and penetration, but with a virtual energy threshold ($T_a$). The values of these $T_a$ are not negligible (greater than 20 J in this case).

In order to correlate results from different DP tests (e.g., by using Eq. (20)), it is desirable the use of $ENPEN$ (Eq. (21)), instead of $ENTHRU$ or $ENTHRU_{cone}$, and definitely the use of $ENPEN$ (Eq. (34)) is considerably more appropriate than using nominal energies, as it is usual practice when applying Eq. (20). This new proposed and improved way of correlating results among various kinds of dynamic penetration tests requires the calculation of $T_a$, which can be obtained by means of Eq. (27).

As the soil actually behaves as if there were a linear relationship between energy and penetration beyond that $T_a$, a new method for calculating the energy correction required to adjust the blow counts to $60\%$ energy efficiency (via Eq. (42)) is also proposed.

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