**Safety aspects of large dragline-operated opencast mines – An overview**

by A. Golder and I. Roy

**Synopsis**

The Jayant opencast operation is one of the largest opencast coal mines in India. Prior to 2008 the mine experienced a number of dragline dump failures, which was a major hindrance in sustaining production. Northern Coalfields Limited (NCL) and the mine management engaged several design, research, and academic institutions to carry out dump slope stability studies, particularly of dragline dumps. Birla Institute of Technology prepared a report on the investigations in May 2009. In this paper we review the findings of the report and the measures taken to tackle the safety aspects of dragline dumps.

**Keywords**

slope stability, circular failure, opencast mining, shear strength, overburden dump.

**Study area**

The Jayant Project of Northern Coalfields Limited (NCL) is situated in the Madhya Pradesh district of Sidhi in the Singrauli Coalfields. The location of the project is latitude 24° 05'45" to 24° 11' 25" N and longitude 82° 38' 21" to 82° 40' 45" E, as per Indian Survey toposheet no. 63 L/12. The Shaktinagar rail station on the Chopan-Katni line of the East Central Railway is approximately 5 km from the project (CMPDI, 2007).

The area of the Jayant Block in the northeastern section of the Singrauli Coalfield is 11.10 km². The Jayant opencast operation of the project is located on a hilly plateau with the RL varying from 390-450 m. The total net geological reserve is 305.50 Mt, while the mineable reserve is 282.71 Mt (as at 31 March 2018) and thus the overall volume of overburden with a common stripping ratio of 2.60 m³/t is 907.20 million m³. There are three different seams present in the Jayant Block, i.e., Turra Seam, Purewa Bottom Seam, and Purewa Top Seam as shown in Figure 1 (Singh et al., 2014; Sharma and Roy, 2015).

In this area, most of the overburden is medium-grained to coarse-grained sandstone, carbonaceous shale, and sandy shale. The the dragline dump is situated on shale and sandy shale that provides a competent foundation. The floor of the dump is covered with a thin layer comprising a wet mixture of coal dust, carbonaceous shale, and sandstone (Singh et al., 2012), and fragments of waste rock, which is referred to as interface material. Two types of circular failure surfaces are envisaged as shown in Figure 2.  

- Failure within the material of dump  
- Failure within both the dump material and interface material.

**Hydrogeological factors**

The hydrogeological parameters that control the stability of the dump are determined as follows.

- An attempt was made earlier to delineate or establish the water table/phreatic surface within the dump by installing piezometers. However, the piezometers could not be installed due to difficulty experienced in drilling through the loose, fragmented, and heterogeneous dump material.  
- In the absence of sufficient hydrogeological data, the position and curvature of the phreatic surface inside the dump, as well as the seepage height above the dump toe as shown in Figure 3, was observed visually and reported by mine officials during rainy seasons.  
- It is not feasible to evaluate the phreatic surface in the dragline dump using piezometers because the soil is heterogeneous. The water table height ($H_w$) is estimated by Casagrande’s equation (Murthy, 2002; Sengupta and Roy, 2015; Moosavi, Shirinabadi, and Gholinejad, 2016).
Safety aspects of large dragline-operated opencast mines – An overview

The upward thrust of the water can be defined as the product of the unit weight of the water and the volume of the dragline dump submerged under the water table within the failure mass (Roy, 2016).

The seepage force is calculated as the product of the upward thrust and the sine of the gradient of the horizontal phreatic surface (Murthy, 2002; Sengupta and Roy, 2015) (Figure 4).

**Seismicity and blast vibrations**

Seismic forces are regarded according to the Indian Standard criteria for earthquake-resistant structural design (5th edn) IS 1893:2002 (IS-1893 (part 1), 2002). The horizontal seismic coefficient ($A_h$) design for the Jayant dragline dump is determined by the following expression (Sengupta and Roy, 2015):

$$ A_h = ZI S_a / 2 R g $$  \[3\]

where

$Z$ = Zone factor (study area is located in zone III according to IS 1893:2000)

$I$ = Importance factor

$R$ = Response reduction factor

$S_a / g$ = Average response acceleration coefficient of dump mass.

According to the Indian seismic map, the project is located in zone III, with the horizontal seismic coefficient of 0.02 m/s$^2$, as per the IS code considered here. The blast vibration coefficient on the dump mass due to ongoing blasting was estimated such that the horizontal coefficient of 0.04 m/s$^2$ will include both seismicity and blasting (Mosinets and Shemyakin, 1974).
Safety aspects of large dragline-operated opencast mines – An overview

Dump floor inclination
The mine floor inclination varies from 2° to 4° (CMPDI, 2018). For stability calculations a dump floor inclination of 3° is considered here (Sengupta and Roy, 2015).

Dragline dump height
The dragline dump height, which varies between 60 to 100 m, and the surcharge load of the shovel dump on the dragline dump are also considered in the stability analysis (Mosinets and Shemyakin, 1974; Zaitseva and Zaitsev, 2009; Sengupta, Sharma, and Roy, 2014).

Coal rib
According to existing practice in this mine, a coal rib of 7 m base width and 3 m top width with average Turra seam thickness of 19 m, as shown in Figure 3, is considered as a resisting force against dump failure (Roy, 2003). The coal rib left at the toe of the dump acts as a retaining wall and reduces dragline dump re-handling to some extent (Colwell and Mark, 2003; Besimbaeva et al., 2018).

Laboratory tests for the generation of geotechnical information
Samples of the dump material as well as the interface material were collected and transported to BIT Mesra for determination of the strength parameters (Ranjan et al., 2017) (Table II).

Recommendations
Considering the above parameters and by applying both Fellenius and Bishop’s simplified method (Moosavi, Shirinabadi, and Gholinejad, 2016), the slope angles for the dragline dump are calculated (Table III) and recommended for a minimum factor of safety of 1.20 for different heights of the seepage face (Besimbaeva et al., 2018).

| Parameter                  | Dump material at natural moisture condition | Interface material in submerged condition at the base of the dragline dump | Interface layer separating the coal rib/barrier |
|----------------------------|--------------------------------------------|--------------------------------------------------------------------------|-----------------------------------------------|
| Cohesion (kN/m²)           | 75                                         | 40                                                                       | 155                                           |
| Angle of internal friction (°) | 25                                         | 21                                                                       | 35                                            |
| Bulk density (kN/m³)       | 20.6                                       | Not required in calculation                                              | 16                                            |
The results are documented for different dragline dump geometries in Table III.

The angle of repose of the dragline dump is the overall angle with respect to the horizontal plane over which it is standing. The dragline dump is considered to be cohesionless for the purpose of determining the angle of repose, although in reality the dump has an emerging water table, an inclined floor, pore pressure within the dump material, and is affected by blast vibrations and pore pressure within the dump material. Also, cohesion is generated due to the compaction of the dump material under its own weight. Hence the angle of repose of the dragline dump is 37° in the ideal case; but in actual site conditions owing to the above prevailing geo-engineering considerations the overall slope of the dump will differ.

The above recommended slopes of the dragline dump are maintained by optimizing the following parameters of the dump profile as shown in Figure 3.

- At the mining level of the dragline – berm width
- Berm width present at coal rib/barrier
- Angle of slope below mining level of the dragline.

It is recommended that truck and shovel dumps overlying the dragline dump are formed 120 to 150 m away from the toe of the dragline dump (Sharma and Roy, 2015), i.e. the interval between the toe of the dump formed by the shovel dump and dragline dump should be at least two cut widths 120–150 m. In this case, the dragline dump will act as a foundation for the shovel dump. Hence, the geotechnical properties are considered to be the same for both the shovel dump and its foundation. Accordingly, the following combinations of shovel dump are calculated and recommended (Government of India, 2017; Directorate General of Mine Safety, 2008) in Table IV and shown in Figure 3.

The recommended overall slope angle of the shovel dump can be maintained by adjusting the berm width at the coal rib roof level.

Precautionary measures

In addition to managing the slope, several proposals are suggested and implemented to ensure the stability of internal dumps as well as dragline dumps (Sharma and Roy, 2015):

- i. Topsoil is dumped separately away from the existing internal overburden dump.
- ii. To form a foundation for the dragline dump, no surface-soil may be dumped at the level from where coal has been extracted.
- iii. By ensuring normal gravitational seepage of water in the direction of the sump area, nominal collection of water takes place where coal has been extracted.
- iv. The dragline dump receives sufficient time to settle, followed by supplementary truck dumping, therefore the distance between the toe of shovel dump and the dragline dump is between 120 and 150 m, i.e. two cuts beyond the toe of the dragline dump (Sharma and Roy, 2015).
- v. The voids in the dragline dumps are consolidated with the help of dozers.
- vi. Some coal is left at the toe of the dump to act as a barrier (coal rib). It is designed in such a way that the overburden dump should cover up the coal rib/barrier as much as possible, and that the coal rib/barrier is likely to burn naturally.
- vii. Efforts are made to extract coal from the coal rib without blasting, at regular intervals of 200 to 250 m along the strike length of the pit, so that there is no accumulation of water against the coal rib.
- viii. Before dumping by dragline, the interface layer is cleaned from where coal has been extracted to as great an extent as possible (Singh et al., 2012). If possible, fragmented overburden rock is dumped to cover the slushy floor at the base of the dragline dump to increase the coefficient of friction at the dump floor.
- ix. As per the recommendations of BIT Mesra, Ranchi (Roy, 2016):
  a) If possible, the mine floor (foundation of internal dump) may be ripped or blasted at intervals to a depth of 1 to 2 m, thereby increasing the coefficient of friction prior to dumping by dragline (Government of India, 2019). It is also recommended that minor blasts to promote the flow of water to the sandstone beds below the open pit floor should be carried out to limiting water retention at the base of the dump.
  b) Regular monitoring of the dumps through a non-contact survey using a laser profiler or laser scanner is proposed to detect any movement of overburden dumps or dump faces that will indicate a potential dump failure. As the dragline dumps are inaccessible, a reflectorless instrument based on laser technology is recommended for surveying the

| Table III  
Prediction of dragline dump geometry |
|-----------------|-----------------|-----------------|
| Height of dragline dump (H) (m) | Overall slope angle of the dragline dump (°) as shown in Figure 3 |
| Seepage height of water (Pw) = 3 m, and height of water table (Hw) = 25 m (Figure 3) | Seepage height of water (Pw) = 6 m, and height of water table (Hw) = 36 m (Figure 3) |
| 60 | 33 | 31 |
| 70 | 32 | 30 |
| 80 | 31 | 29 |
| 90 | 30 | 28 |
| 100 | 28 | 26 |

| Table IV  
Prediction of shovel dump geometry |
|-----------------|-----------------|
| Shovel dump height (m) | Angle of overall slope (°) |
| 60 | 35 |
| 70 | 33 |
| 80 | 32 |
displacement of the dump face between the crest and the toe of the dump.

c) It is proposed that monitoring of the dump should be carried out and recorded at seven-day intervals during the monsoon and post-monsoon seasons (July to November), whereas in the dry season (December to June) monitoring should be done at 15-day intervals. In the case of any movement of the overburden dump, the de-coaled floor near the toe of the dump is declared as a hazard zone with removal of men and machinery from the hazard zone.

d) The seepage of water from the face of the dragline dump is to be monitored when the coal rib has been breached at 7-day intervals from July to November and at 15-day intervals for the rest of the period.

Conclusion

Based on the recommendation of the Birla Institute of Technology Mesra, the Jayant opencast project has maintained the dump profiles by adjusting the following parameters as shown in Figure 3:

- a) Berm width at the coal rib roof level
- b) Angle between the coal rib roof and the dragline mining level
- c) Berm width at the dragline mining level
- d) Slope angle above the dragline mining level.

The above-mentioned measures have successfully prevented any major failure of backfilled dumping in spite of the huge volumes of waste rock (around 40 million m$^3$ per year loose volume) handled and dumped within the de-coaled area.

Acknowledgement

The authors are grateful to the administration of Birla Institute of Technology Mesra, Ranchi (India) for their permission to present this paper. This research work was financially supported by Coal India Limited (CIL), Kolkata - 700156, India (Project Code No.: CIL/R&D/01/68/2018). The opinions expressed in this paper are those of the authors and not to the institution to which they belong.

Funding Agency: Coal India Limited, Kolkata, India (P. Code No.: CIL/R&D/01/68/2018).

The authors declare that they have no conflict of interest.

References

BESIMBAeva, O.G., KhIMbROva, E.N., NIZametDINov, F.K., and OLEINIKova, E.A. 2018. Assessment and prediction of slope stability in the Kentobe open pit mine. Journal of Mining Science, vol. 54. pp. 988–994, https://doi.org/10.1007/s10913-018-0651-4

BUREAU OF INDIAN STANDARDS. 2002. IS-1893 (part 1). Criteria for earthquake resistant design of structures. New Delhi. pp. 14–23.

CMPDI. 2018. Revised mine closure plan of Jayant opencast project.

CMPDI. 2007. Environmental impact assessment (EIA).

COLWELL, M. and Mark, C. 2003. Analysis and design of rib support (ADRS) A rib support design methodology for Australian coalfields. Proceedings of the 24th International Conference on Ground Control in Mining, West Virginia University, Morgantown, WV. https://www.cdc.gov/niosh/mining/UserFiles/works/pdfs/aador.pdf

DASH, A.K. 2019. Analysis of accidents due to slope failure in Indian opencast coal mines. Current Science, vol. 117. pp. 304–308.

DIRECTORATE GENERAL OF MINE SAFETY (DGMS). 2008. Report of high powered committee on accident in the West Coal Section of Jayant, NCL. Dhanbad, Jharkhand.

GOVERNMENT OF INDOIA. 2010. Ministry of Coal. https://coal.nic.in/

GOVERNMENT OF INDIA. 2017. Coal mines regulations. New Delhi.

MOOSAVI, E., SHIRINABADI, R., and CHOLINEJAD, M. 2016. Prediction of seepage water pressure for slope stability at the Gol-E-Gohar open pit mine. Journal of Mining Science, vol. 52. pp. 1069–1079. https://doi.org/10.1134/S1062739116060561

Mosinets, V.N. and Shemyakin, E.I. 1974. Research on the seismic effects of blasting in the mining industry. Journal of Mining Science, vol. 10. pp. 442–447. https://doi.org/https://doi.org/10.1007/BF02504743

MUT¥TH, V.N.S. 2002. Geotechnical Engineering: Principles and Practices of Soil Mechanics and Foundation Engineering. Jurnal Ekonomi Malaysia. https://doi.org/10.1017/CBO9780511523240.004

SINGH, P.K., SINGH, M.P., VOLKMANN, N., NaIK, A.S., and BÖRNER, K. 2014. Petrological characteristics of lower Gondwana coal from Singrauli coalfield, Madhya Pradesh, India. International Journal of Oil, Gas and Coal Technology, vol. 8. pp. 194–220. https://doi.org/10.1504/IJOCT.2014.064849

RANJAN, V., SEN, P., KUMAR, D., and SARKAR, A. 2017. Enhancement of mechanical stability of waste dump slope through establishing vegetation in a surface iron ore mine. Journal of Mining Science, vol. 53. pp. 377–388. https://doi.org/10.1134/S106273911702228X

Roy, I. 2016. A report on CIL R&D Project: Development of guidelines to predict distance between the toe of the shovel dumps and that of the dragline dumps with consideration of safety and economical design of both shovel dumps and dragline dumps of Coal India. Ranchi, Jharkhand.

Roy, I. 2003. The role of interface material at the base of internal dumps and effectiveness of coal rib in the safe working of opencast coal mines. Journal of Rock Mechanics and Tunnelling Technology, vol. 9. pp. 155–172.

Sengupta, S. and Roy, I. 2015. Study of internal dump stability of Dudhichha open cast project, Northern Coalfields Limited, India. Journal of The Institution of Engineers (India): Series D, vol. 96. pp. 67–75. https://doi.org/10.1007/s40033-014-0061-5

Sengupta, S., Sharma, S., and Roy, I. 2014. Investigation of shear strength parameters of highwall rock slopes and overburden dump mass in opencast coal mines. International Journal of Engineering, Management, Humanities and Social Sciences Paradigms (IJEMHS), vol. 7. pp. 1–6.

Sharma, S. and Roy, I. 2015. Slope failure of waste rock dump at Jayant opencast mine, India: A case study. International Journal of Applied Engineering Research, vol. 10. pp. 33036–33042.

SINGH, P.K., ROY, M.P., PASWAN, R.K., SINGH, V.K., SINHA, A., SHASTRI, L.B., SINGH, C.P., and Roy, M. 2012. Effect of production blasts on waste dump stability. Rock Fragmentation by Blasting. Singh, P.K. and Sinha, A. CRC Press, London.

ZAITSEVA, A.A. and ZAITSEV, G.D. 2009. Influence of geological and technological factors on the internal dump capacity in flat deposits. Journal of Mining Science, vol. 45. pp. 382–389. https://doi.org/10.1007/s10913-009-0048-2