A comparative analysis of the quality of concrete blocks produced from coconut fibre, oil palm empty fruit bunch, and rice husk as filler material

Mohammad Lutfi1,* , Muh Yamin2, Mujibu Rahman2, and Elisa Ginsel Popang2

1Department of Petroleum Engineering, STT MIGAS, Balikpapan, 76127, Indonesia
2State Agricultural Polytechnic of Samarinda, 75131, Indonesia

Abstract. The accumulation of coconut fibre (CF), oil palm empty fruit bunch (OPEFB), and rice husks (RH) every year can reduce the fertility of soil and water absorption, and causes water acidification. Waste utilization as a filler material of concrete blocks was discussed in this research. Experimental design was used by comparing the quality of concrete blocks based on 36 specimens with varied compositions of waste (1%, 2%, 3%, and 4% by dry weight of total sand) and 3 specimens (0%) as control specimens with 3 replications for each composition. The quality of paving blocks was determined based on the testing of water absorption, porosity, compressive strength, and density. The results revealed that the quality of concrete blocks with the composition of CF (1% and 2%), OPEFB (1% and 2%), and RH (1%) meet the requirements of SNI 03-0691-1996 criteria in category B for parking paver and the maximum composition of each waste materials (3% and 4%) still comply SNI 15-2094-2000 in class 100 and 150 for the block walls. Statistical analysis revealed that the best treatments for compressive strength was RH (1%) and for water absorption it was RH (2%) and CF (4%).

1 Introduction

The waste produced from agricultural and industrial production activities such as empty palm fruit bunch (OPEFB), rice husk (RH), and coconut fibre (CF) are the lignocellulosic materials which causes environmental pollution problems.

OPEFB is the solid waste produced from the palm oil industry. In Indonesia, the volume of crude palm oil (CPO) is approximately 28 million tons per year [1]. The increase of the production is in line with the increase of OPEFB as its waste. It causes environmental hazards from the landfill process [2]. The remainder of the oil palm significantly affected the soil’s physical and chemical properties, and nutrient content [3,4].

* Corresponding author: lutfi_plhld@yahoo.co.id
In the majority of rice producing countries, RH is burnt in open air or is sent to a land fill [5]. However, both methods create enormous CO₂ emission into the atmosphere. This causes the environmental problem of disposal due to its abundance.

CH is the mesocarp of coconut and a coconut consists of 33–35% husk [6]. The natural fibre extracted from CH is CF. It has led to the emergence of the coir industry. The retting process causes extensive damage to the Kayals of Kerala and the aquatic habitats along the southwest coast of India due to oxygen depletion and high concentrations of hydrogen sulphide [7, 8].

One of the efforts to overcome this problem is by using the waste as a filler material for concrete blocks. This effort is in line with the increasing demand of the building materials that had come into the concern of the public and related industries. In Malaysia, the agro-waste is generated from agricultural sources such as RH, jute fibre (JF), and CH [9]. This agro-waste can be remade into sustainable building materials. Reuse of such agro-waste does not only overcome the pollution to the environment and shortage of building materials but also the disposal problem of agro-waste and achieves the objective of sustainable development.

The research related to the durability of concrete has been conducted by many researchers to minimize the negative ecological impact [10-18]. This research proposes to utilize OPEFB, RH, and CF without combustion process to reduce air pollution which is the trigger of global warming [19].

According to the background, a comparative study of the quality of concrete blocks produced from OPEFB, RH, and CF as a filler material based on the Indonesian National Standard (SNI) is discussed in this paper. The results are expected to be a recommendation for areas that have less sand, but have abundant OPEFB, CF, and RH, so it can help the areas toward a sustainable development.

2 Method

2.1 Materials

The waste was collected from different places, namely: OPEFB was obtained from PTPN XIII Semuntai, RH obtained from the rice mill at Loa Janan, and CF was obtained from Samarinda. River sand was obtained from Mahakam river and the water used in this experiment was clean tap water free from impurities capable of undermining the chemical reaction of cement with the water. Portland Pozzolan Cement (Gresik Cement) in a well-protected condition from dampness was used in this research.

The waste material was cleaned and selected, then, mineralization process was conducted before the mixing process to remove lignin, cellulose, and another compound which prevents the stickiness and hardening of cement for concrete mixtures through soaking and drying under the sun light directly.

2.2 Experimental design

2.2.1 Design of concrete mixes

The mold was made with a size of 22 cm in length, 8 cm in width, and 6 cm in thickness (Fig. 1). The number of specimens with composition of waste materials were 36 and 3 specimens without waste materials as specimen control.
The concrete blocks were produced in five different treatments with 3 replications for each treatment (3 specimens for each treatment). Design of concrete mixes are as follows:

| Treatment | Waste (% of sand) | Waste (kg) | Cement (kg) | Sand (kg) | Water (kg) |
|-----------|-------------------|------------|-------------|-----------|------------|
| Treatment 1 (control specimen) | 0 | 0 | 0.555 | 2.775 | 0.37 |
| Treatment 2 | 1 | 0.028 | 0.555 | 2.747 | 0.37 |
| Treatment 3 | 2 | 0.056 | 0.555 | 2.72 | 0.37 |
| Treatment 4 | 3 | 0.083 | 0.555 | 2.692 | 0.37 |
| Treatment 5 | 4 | 0.111 | 0.555 | 2.664 | 0.37 |

2.2.2 Casting, curing, and testing

The casting was conducted by hand mixing for each treatment without vibration process. After the specimens were removed from the molds, curing was performed until it reaches a constant weight at an age of 28 days. During the curing process, loss of water from the concrete blocks was prevented as much as possible.

The quality and characteristics of concrete blocks was determined by observing the physical appearance of specimens and performing water absorption and compressive strength tests at the laboratory of civil engineering (State Polytechnic of Samarinda) based on the national standardization agency of Indonesia on the SNI 03-0691-1996 [20] and SNI 15-2094-2000 [21] criteria.

2.2.3 Analysis

The quality of concrete blocks among the specimens with waste materials and the control specimen were analyzed based on the average values (compressive strength, density, water absorption, and porosity) of each treatment to have a quick overview.

Analysis of variance was conducted using completely randomized designs to investigate the effect of treatments on the quality of concrete blocks. Furthermore, in cases where there is a significant difference between means, then LSD analysis was used to determine which treatments are the best among them based on [20] and [21] criteria and the concept of waste management approach. LSD must not be applied unless the F-test indicates that significant differences between means were present.
The correlation between the composition of waste materials on the compressive strength and water absorption were described using linear regression analysis. The relationship level is stated in numbers, wherein a larger number shows a stronger relationship. The guidance of the correlation coefficient interpretation are as follows: 0.000 – 0.199 (very weak), 0.200 – 0.399 (weak), 0.400 – 0.599 (inconclusive), 0.600 – 0.799 (strong), and 0.800 – 1.000 (very strong) [22].

3 Results and discussions

3.1 Visual appearance

The whole specimens that were produced revealed that the visual appearance meet the requirement of [20, 21]. Fig. 2 shows that the concrete blocks produced from CF, OPEFB, and RH respectively are not cracked and defective, also, the corners are not easily damaged by finger strength.

![Fig. 2. Visual appearance of concrete blocks produced from CF, OPEFB, and RH.](image)

3.2 Water absorption and porosity

According to Fig. 3(a), the average values of water absorption level of the concrete blocks with filler materials from OPEFB, CF, and RH can be described as follows:

a. Treatment 3 and treatment 4 with the composition of CF (30% and 40%) meet the requirements of [20] criteria in category A and [21]. The treatment 1 and treatment 2 with the composition of OPEFB (10% and 20%) comply to [20] criteria in category B and SNI [21].

b. Treatment 2 and treatment 4 with the composition of RH (20% and 40%) meet the requirements of [20] criteria in category A, treatment 1 with the composition of RH (10%) in category B, and treatment 3 with composition of RH (30%) in category C. All treatments comply with [21].

c. Treatment 1 with the composition of OPEFB (10%) meet the requirements of [20] criteria in category B and SNI 15-2094-2000. The treatment 2 to treatment 4 with the composition of OPEFB (20%, 30%, and 40%) comply with SNI 03-0691-1996 criteria in category A and [21].

The base value (3.60) is the average value of water absorption of control specimens with no waste materials (treatment 1). In general, water absorption of specimens produced from waste are better than control specimens.

According to Fig. 3(b), the decrease in porosity is proportional to the addition of waste materials, except for RH. Further research needs to be conducted at composition of 5%, 6%, 7% and so on. Probably, at the particular composition, the porosity and water absorption will increase gradually in line with the addition of waste materials.
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c. Treatment 1 with the composition of OPEFB (10%) meet the requirements of [20] criteria in category B and SNI 15-2094-2000. The treatment 2 to treatment 4 with the composition of OPEFB (20%, 30%, and 40%) comply with SNI 03-0691-1996 criteria in category A and [21].

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One of the causes of the variation of porosity for RH is that the husks used was not crushed first, but mixed into the mixture directly, it will cause the increasing of water absorption and porosity for treatment 1 and treatment 3. Whereas in treatment 2 and treatment 4, the husks was crushed before mixing, so that the water absorption is low.

3.3 Compressive strength and density

According to Fig. 4(a), the average values of the compressive strength of concrete blocks with filler material from OPEFB, CF, and RH can be described as follows:

a. Treatment 1 and treatment 2 with the composition of OPEFB (10% and 20%) meet the requirements of [20] criteria in category B and [21] in class 150. Treatment 3 and treatment 4 with the composition of OPEFB (20% and 30%) comply [20] criteria in category C and [21] in class 150.

b. Treatment 1 and treatment 2 with the composition of CF (10% and 20%) meet the requirements of [20] criteria in category B and [20] in class 150. Treatment 3 and treatment 4 with the composition of CF (30% and 40%) comply with [20] criteria in category C and [21] in class 100 for treatment 3 and class 150 for treatment 4.

c. Treatment 1 and treatment 3 with the composition of RH (10% and 30%) meet the requirements of [20] criteria in category B, treatment 2 with composition of 20% in category C, and treatment 4 with composition of 40% in category D. The whole treatments comply with [21] in class 150 except treatment 4 which is included in class 100.

Base value (31.67) is the average value of the compressive strength of the control specimens without waste materials. The compressive strength of the whole treatments have average values below the control specimen. The adhesion process under the influence of the gravitational field [23] between cement and aggregate increases the compressive strength, whereas the cohesion process between aggregate decreases the compressive strength.
The variation of compressive strength values for each replication at the same composition is caused by the compacting process that was conducted by hand without the vibration process which has led to the nonhomogeneous problem during the casting process.

The addition of waste materials is decreasing the density of specimens, it causes the compressive strength to also decrease due to the density of the waste materials to be less than the density of sand. Fig. 4(b) depicts the decrease in density due to the addition of waste materials.

### 3.4 Completely randomized designs

Analysis of variance by means of completely randomized designs revealed that the effect of treatments on the water absorption and compressive strength of concrete blocks can be described as follows respectively:

- a. There are no significant differences of OPEFB composition variation on the water absorption with calculated $F (2.386) < \text{table } F (4.066)$ and the compressive strength with calculated $F (2.355) < \text{table } F (4.066)$ for a significance level of 5% ($\alpha = 5\%$).
- b. There are significant differences of RH composition variation on the water absorption with calculated $F (32.023) > \text{table } F (7.591)$ for $\alpha = 1\%$ and also for RH composition variation on the compressive strength with calculated $F (4.148) > \text{table } F (4.066)$ for $\alpha = 5\%$.
- c. There is a significant difference of CF composition variation on the water absorption with calculated $F (33.644) > \text{table } F (7.591)$ for $\alpha = 1\%$. Whereas there is no significant difference of CF composition variation on the compressive strength with calculated $F (0.937) < \text{table } F (4.066)$ for $\alpha = 5\%$.

### 3.5 Least significance different (LSD) analysis

LSD calculations are used based on the results of completely randomized designs, if the results revealed that there is a significant difference between the means of waste composition on the water absorption and compressive strength, then LSD can be used, and vice versa.

According to the compressive strength on [20], LSD analysis revealed that treatments of RH for composition of waste 1%, 2%, and 3% are good treatments. Treatment for compositions of 1% are included in category B, while for compositions of 2% and 3% are included in category C.

The best treatment for RH with composition of waste of 2% and treatments of CF with composition of 3% and 4% are included in quality A based on water absorption level on
In context of waste management, the more waste that can be recycled, the more amount of waste can be reduced, therefore, the best treatment for CF is with composition of 4%.

### 3.6 Correlation

A very strong relationship was indicated by a high correlation coefficient between the composition of OPEFB and CF on water absorption, \( R^2 = 0.9337 \) with regression equation of \( y = -0.949x + 5.1125 \) for OPEFB and \( R^2 = 0.9399 \) with regression equation of \( y = -1.5604x + 7.2445 \) for CF respectively. Whereas a very weak relationship can be seen between composition of RH and water absorption (\( R^2 = 0.0198 \) with regression equation is \( y = 0.023x + 2.8713 \)).

An inconclusive relationship was indicated between the composition of OPEFB and CF on the compressive strength, \( R^2 = 0.4027 \) with regression equation of \( y = -3.5x + 29.167 \) for OPEFB and \( R^2 = 0.5729 \) with regression equation of \( y = -2.1667x + 23.333 \) for CF respectively. Whereas the very weak relationship can be seen between the composition of RH and the compressive strength (\( R^2 = 0.0143 \) with regression equation of \( y = -0.0667x + 20 \)).

### 4 Conclusions

The results revealed that the quality of concrete blocks with the composition of CF (1% and 2%), OPEFB (1% and 2%), and RH (1%) meet the requirements of [20] criteria in category B for parking paver and the maximum composition of each waste (3% and 4%) still comply with [21] criteria in class 100 and 150 for the block walls.

According to the completely randomized designs and least significance difference calculations, the best treatment for the compressive strength test is RH with composition of waste 1% included in category B on [20] criteria and the best treatment based on the water absorption test were RH with composition of waste 2% and CF with composition of waste 4% included in category A.
Linear regression analysis revealed a very strong and inconclusive relationship indicated between the composition of OPEFB and CF on water absorption and compressive strength tests respectively. Whereas the very weak relationship of RH to both tests revealed that treatments 1 and 3 need to be repeated by crushing the husk before the mixing process.

The results of this research need to be compared to the other experimental designs at the same composition of each waste material (especially for RH treatments) with a better method using machines and the technology to reduce the content of lignin and cellulose and other substances without burning waste materials to obtain the silica contained in the charcoal. The abrasion and resistance to sodium sulphate tests also need to be conducted to describe comprehensively the quality of concrete blocks based on [20] criteria.

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