INTRODUCTION

Basic stream flow and rainfall data are not adequately available for planning and designing water management facilities and other hydraulic structures in ungauged watershed. This situation is common in Nigeria due to lack of gauging stations along most of the rivers and streams. However, techniques have been evolved that allow generation of synthetic unit hydrographs. This includes Snyder, Soil Conservation Service (SCS), Gray and Clark’s Instantaneous Unit Hydrograph methods. The peak discharges of stream flow from rainfall can be obtained from the design runoff hydrographs developed from unit hydrographs ordinates determined from established methods. Warren et al. [1972] described hydrograph as a continuous graph showing the properties of stream flow with respect to time, normally obtained by means of a continuous strip recorder that indicates stages versus time and is then transformed to a discharge hydrograph by application of a rating curve. Wilson [1990] observed that with an adjustment and well measured rating curve, the daily gauge readings may be converted directly to runoff volume. He also emphasized that catchment properties influence runoff and each may be present to a large or small...
A vast amount of literature exists treating the various unit hydrograph methods and their development. Jones [2006] reported that Sherman in 1932 was first to explain the procedure for development of the unit hydrograph and recommended that the unit hydrograph method be used for watersheds of 2000 square miles (5000 km²) or less. Chow et al. [1988] discussed the derivation of unit hydrograph and its linear systems theory. Furthermore, Viessman et al. [1989], Wanielista [1990] and Arora [2004] presented the history and procedures for several unit hydrograph methods. Ramirez [2000] reported that the synthetic unit hydrograph of Snyder in 1938 was based on the study of 20 watersheds located in the Appalachian Highlands and varying in size from 10 to 10 000 square miles (25 to 25 000 km²). Ramirez [2000] reported that the dimensionless unit hydrograph was developed by the Soil Conservation Service and obtained from the UH’s for a great number of watersheds of different sizes and for many different locations. It was also stated by Ramirez [2000] that the SCS dimensionless hydrograph is a synthetic UH in which the discharge is expressed as a ratio of discharge, \( Q \), to peak discharge, \( Q_p \), and the time by the ratio of time, \( t \), to time to peak of the UH, \( t_p \). Wilson [1990] also reported that in 1938, McCarthy proposed a method of hydrograph synthesis but in that same year Snyder proposed a better known method by analyzing a larger number of basins in the Appalachian mountain region of the United States. Ogunlela and Kasali [2002] applied four methods of unit hydrographs generation to develop a unit hydrograph for an ungauged watershed. The outcome of the study revealed that both Snyder and SCS methods were not significantly different from each other. Salami [2009] applied three unit hydrograph methods for runoff hydrograph development of lower Niger River basin at downstream of the Jebba Dam. The methods considered were Snyder, SCS and Gray methods. The statistical analysis, conducted at the 5% level of significance, indicated significant differences in the methods except for Snyder and SCS methods which have relatively close values. In this study Snyder and SCS methods were used to determine the ordinate of unit hydrographs and was subsequently used to generate peak runoff hydrographs of rainfall depth of various return intervals through convolution for selected rivers in south west, Nigeria. The outcome of the study will make the selection of peak runoff flows of the desire return period for design of hydraulic structures in the region possible.

MATERIALS AND METHODS

Study Area

The river catchments under consideration are Fawfaw, Oba, Awon, Opeki, Ogunpa, Osun, Otin and Ogun located in the Ogun – Osun River basin, South West Nigeria as presented in Figure 1.

Development of Unit Hydrograph

The methods of unit hydrographs used to determine the peak runoff ordinates are; Snyder’s and Soil Conservation Service (SCS) methods.

Snyder’s method

In adopting Snyder’s method, the following parameters were determined: the peak discharge, \( Q_p \), lag time and the time to peak, rainfall duration, the peak discharge per unit of watershed area, \( q' \), the basin lag \( t' \), the base time, \( t_b \), and the widths, \( w \) (in time units) of the unit hydrograph at 50 and 75 percent of the peak discharge. The parameters were estimated in accordance to Ramirez [2000] and Arora [2004] using equations (1) to (8).

Lag time, \( t' \)

\[ t' = C_t(L \times L_c)^{0.3} \tag{1} \]

Where \( C_t \) is a coefficient representing variations of watershed slope and storage. Values of \( C_t \) range from 1.0 to 2.2 [Arora, 2004]. An average value of 1.60 is assumed for this catchment. Equation (1) gives the lag time for the watershed.

Unit-hydrograph duration, \( t_r \) (storm duration)

\[ t_r = \frac{t'}{5.5} \tag{2} \]

From equation (2) the duration of the storm was obtained. However, if other storm durations are intended to be generated for the watershed,
the new unit hydrograph storm duration ($t'_p$), the corresponding basin lag time ($t'_l$) can be obtained from equation (3).

$$t'_l = t_l + \left( \frac{t_p - t_l}{4} \right)$$  \hspace{1cm} (3)

The peak discharge ($Q'_p$) was obtained from equation:

$$Q'_p = \frac{2.78 \times C_p \times A}{t_l}$$  \hspace{1cm} (4)

Where $C_p$ is the coefficient accounting for flood wave and storage conditions. Values of $C_p$ range from 0.3 to 0.93 [Arora 2004] with an average of 0.62 is assumed for this catchment.

The base time was obtained from equation (5)

$$t_b = 3 + 3 \left( \frac{t_l}{24} \right)$$  \hspace{1cm} (5)

The time width $W_{50}$ and $W_{75}$ of the hydrograph at 50% and 75% of the height of the peak flow ordinate were obtained based on equations (6) and (7) respectively in accordance with U.S Army Corps of Engineer [Arora, 2004]. The unit of the time width is hr. Also the peak discharge per area (cumec/km$^2$) is given by equation (8).

$$W_{50} = \frac{5.9}{q'_p}$$  \hspace{1cm} (6)

$$W_{75} = \frac{3.4}{q'_p}$$  \hspace{1cm} (7)

$$q'_p = \frac{Q'_p}{A}$$  \hspace{1cm} (8)

The output from the equations and the measured physical parameters of each of the basins are presented in Table 1.

**Soil Conservation Service (SCS)**

In adopting the method of US Soil Conservation Service (SCS) for constructing synthetic unit hydrographs was based on a dimensionless hydrograph, which relates ratios of time to ratios of flow [Viessman et al. 1989] and Ramirez [2000]. The peak discharge and the time to peak were determined in accordance with [Viessman et al. 1989, Wanielista 1990, Ramirez 2000, SCS 2002, Ogunlela and Kasali 2002, Raghunath 2006] by adopting equations (9) to (12).

**Peak discharge**

The peak discharge is obtained through the equation [Ramirez 2000]:

$$Q'_p = \frac{0.208 \times A \times Q_d}{t_p}.$$  \hspace{1cm} (9)

where: $Q'_p$ = peak discharge (m$^3$/s)

$A$ = watershed area (km$^2$)

$Q_d$ = quantity of run off (mm)

$t_p$ = time to peak (hr)

Time to peak ($t_p$) and lag time ($t_l$):

$$t_p = \frac{t_c}{2} + t_l \quad t_p = \frac{t_c + 0.133t_c}{1.7}$$  \hspace{1cm} (10)

$$t_l = 0.6t_c$$  \hspace{1cm} (11)

where: $t_c$ = time of concentration (min)
where: \( L \) – length of channel (m); 
\( S \) – slope of channel.

The estimated values of both the peak discharge and time to peak were applied to the dimensionless hydrograph ratios in accordance to SCS and the points for the unit hydrograph were obtained [Raghunath 2006] and used to develop the unit hydrograph curve. The calculated values for parameters \( t_p \) and \( q_p \) were applied to the SCS dimensionless unit hydrograph to obtain the corresponding unit hydrograph ordinates. The estimated unit hydrograph ordinates is presented in Table 2 based on the values of time to peak discharge \( (t_p) \) and peak discharge \( (q_p) \) for each river catchment.

### Development of Peak Runoff Hydrographs

The established unit hydrographs ordinates were used to develop the runoff hydrographs due to actual rainfall event over the catchment. Peak runoff hydrographs for selected return periods (20yr, 50yr, 100yr, 200yr and 500yr) were developed through convolution. The maximum 24-hr rainfall depths of the different recurrence interval for the catchment under consideration are 174.2 mm, 205.0 mm, 232.3 mm, 262.73 mm and 309.0 mm respectively [Olofinyoye et al. 2009]. The runoff hydrograph was derived from a multiperiod of rainfall excess called hydrograph convolution. It involves multiplying the unit hydrograph ordinates \( (U_n) \) by incremental rainfall excess \( (P_n) \), adding and lagging in a sequence to produce a resulting runoff hydrograph. The SCS type II curve was used to divide the different rainfall data into successive equal short time events and the SCS Curve Number method was used to estimate the cumulative rainfall for storm depth of 20yr, 50yr, 100yr, 200yr and 500yr return period. The incremental rainfall excess was obtained by subtracting sequentially, the rainfall excess from the previous time events. The equations that apply to the SCS Curve Number method are given below [SCS 2002].

### Table 1. Watershed characteristics for generating unit hydrograph (Snyder’s method)

| River watershed | \( L \) (km) | \( L_e \) (km) | \( t_c \) (hr) | \( t_e \) (hr) | \( T_e \) (hr) | \( A \) (km\(^2\)) | \( S \) (%) |
|-----------------|-------------|-------------|-------------|-------------|-------------|-------------|----------|
| Faw-Faw         | 11.80       | 6.40        | 5.66        | 1.07        | 13.54       | 89.57       | 46.00    | 0.59     |
| Oba             | 23.50       | 10.00       | 8.23        | 1.50        | 78.53       | 96.70       | 375.00   | 0.39     |
| Awun            | 35.60       | 20.00       | 11.48       | 2.09        | 60.52       | 106.44      | 403.00   | 0.34     |
| Ogunpa          | 22.87       | 13.20       | 8.87        | 1.61        | 21.15       | 98.62       | 108.85   | 0.46     |
| Opeki           | 43.50       | 20.00       | 12.19       | 2.22        | 81.31       | 108.57      | 575.00   | 0.21     |
| Otun            | 36.00       | 16.00       | 10.77       | 1.96        | 76.01       | 104.31      | 475.00   | 0.36     |
| Osun            | 47.50       | 15.00       | 11.48       | 2.09        | 175.66      | 106.44      | 1170.00  | 0.21     |
| Ogun            | 600.00      | 315.00      | 61.25       | 11.14       | 574.13      | 255.73      | 20400.00 | 0.07     |

### Table 2. Unit hydrograph ordinates for US Soil Conservation Service (SCS) method

| Faw-Faw River | Ob River | Awon River | Ogunpa River | Opeki River | Otun River | Osun River | Ogun River |
|---------------|----------|------------|--------------|-------------|------------|------------|-----------|
| \( t \) (hr)  | \( Q \) (m\(^3\)/s) | \( t \) (hr) | \( Q \) (m\(^3\)/s) | \( t \) (hr) | \( Q \) (m\(^3\)/s) | \( t \) (hr) | \( Q \) (m\(^3\)/s) | \( t \) (hr) | \( Q \) (m\(^3\)/s) |
| 0.0           | 0.0      | 0.0        | 0.0          | 0.0         | 0.0        | 0.0        | 0.0       | 0.0        | 0.0        | 0.0       |
| 1.1           | 19.7     | 2.1        | 80.2         | 3.0         | 59.6       | 1.9        | 25.3      | 4.3        | 60.4       | 3.0        |
| 2.1           | 45.8     | 4.2        | 186.5        | 6.1         | 138.7      | 3.9        | 58.9      | 8.5        | 140.4      | 6.0        |
| 3.2           | 30.2     | 6.3        | 123.1        | 9.1         | 91.6       | 5.8        | 38.9      | 12.8       | 92.7       | 9.0        |
| 4.2           | 14.7     | 8.4        | 59.7         | 12.1        | 44.4       | 7.7        | 18.9      | 17.1       | 44.9       | 12.0       |
| 5.2           | 7.1      | 10.5       | 28.9         | 15.1        | 21.5       | 9.6        | 9.2       | 21.3       | 21.8       | 15.0       |
| 6.3           | 3.4      | 12.6       | 14.0         | 18.1        | 10.4       | 11.6       | 4.4       | 25.6       | 10.5       | 18.0       |
| 7.3           | 1.7      | 14.7       | 6.7          | 21.2        | 5.0        | 13.5       | 2.1       | 29.8       | 5.1        | 21.0       |
| 8.4           | 0.8      | 16.7       | 3.4          | 24.2        | 2.5        | 15.4       | 1.1       | 34.1       | 2.5        | 24.1       |
| 9.4           | 0.4      | 18.8       | 1.7          | 27.2        | 1.3        | 17.3       | 0.6       | 38.4       | 1.3        | 27.1       |
| 10.5          | 0.2      | 20.9       | 0.8          | 30.2        | 0.6        | 19.2       | 0.3       | 42.6       | 0.6        | 30.1       |
| 11.5          | 0.0      | 23.0       | 0.0          | 33.3        | 0.0        | 21.2       | 0.0       | 46.9       | 0.0        | 33.1       |
\[ Q_d = \left( \frac{P^* - I_a}{P^* + 0.8S}\right)^2 \] for \( P^* > 0.2S \)

\[ Q_d = 0 \] for \( P^* \leq 0.2S \)

where: \( P^* \) – accumulated precipitation (mm)

\( Q_d \) – cumulative rainfall excess, runoff (mm)

\( I_a \) – initial abstraction, \( I_a = 0.2S \).

\[ S = \frac{25400}{CN} - 254 \] 

Table 3. Peak runoff hydrograph (m³/s)

| Storm return periods | 20yr, 24hr | 50yr, 24hr | 100yr, 24hr | 200yr, 24hr | 500yr, 24hr |
|----------------------|------------|------------|-------------|-------------|-------------|
| **Faw-Faw River catchment** | | | | | |
| Snyder               | 112.63     | 143.70     | 171.28      | 203.15      | 352.34      |
| SCS                  | 304.43     | 388.06     | 464.59      | 556.52      | 699.89      |
| **Oba River catchment** | | | | | |
| Snyder               | 678.80     | 866.23     | 1030.03     | 1218.99     | 1510.35     |
| SCS                  | 1240.54    | 1581.35    | 1893.19     | 2267.81     | 2852.03     |
| **Awon River catchment** | | | | | |
| Snyder               | 555.52     | 707.11     | 839.52      | 992.08      | 1227.17     |
| SCS                  | 922.46     | 1175.88    | 1407.77     | 1686.34     | 2120.76     |
| **Ogumpa River catchment** | | | | | |
| Snyder               | 180.44     | 230.26     | 273.80      | 324.02      | 401.46      |
| SCS                  | 391.89     | 499.55     | 598.06      | 716.40      | 900.96      |
| **Opeki River catchment** | | | | | |
| Snyder               | 724.84     | 925.16     | 1100.29     | 1302.34     | 1613.93     |
| SCS                  | 933.81     | 1190.34    | 1425.08     | 1707.08     | 2146.84     |
| **Otin River catchment** | | | | | |
| Snyder               | 672.80     | 858.83     | 1021.46     | 1209.10     | 1498.46     |
| SCS                  | 1093.50    | 1393.91    | 1668.80     | 1999.02     | 2513.93     |
| **Osun River catchment** | | | | | |
| Snyder               | 1558.63    | 1989.43    | 2366.10     | 2800.66     | 3470.81     |
| SCS                  | 1775.65    | 2263.46    | 2709.82     | 3246.04     | 4082.25     |
| **Ogun River catchment** | | | | | |
| Snyder               | 6018.71    | 7672.82    | 9120.66     | 10790.02    | 13364.33    |
| SCS                  | 2812.87    | 3585.63    | 4292.72     | 5142.16     | 6466.84     |

RESULTS AND DISCUSSIONS

Two methods of synthetic unit hydrograph were adopted to determine the ordinates for the development of peak runoff hydrograph for eight catchments listed. The values of the ordinate from the synthetic unit hydrograph methods were presented in Tables 1 and 2 while the runoff hydrograph peak flows (m³/s) for the eight river catchments are presented in Table 3.

It was observed for Faw-Faw river catchment that the values obtained for SCS method is higher by 63.31% than that of Snyder method. For Oba river catchment, the values obtained for SCS method is higher by 45.88% than that of Snyder. Also for Awon river catchment, the values obtained for SCS method is higher by 40.66% than that of Snyder. Likewise, for Ogumpa river catchment, the value obtained for SCS method is higher by 54.46% than that of Snyder method. For Opeki river catchment, the values obtained for SCS method is higher by 23.20% than that of Snyder. For Otin river catchment,
Figure 2. Runoff hydrograph of different return periods for Faw-faw River: a) SCS method, b) Snyder method

Figure 3. Runoff hydrograph of different return periods for Oba River: a) SCS method, b) Snyder method
Figure 4. Runoff hydrograph of different return periods for Awon River: a) SCS method, b) Snyder method

Figure 5. Runoff hydrograph of different return periods for Ogunpa River: a) SCS method, b) Snyder method
Figure 6. Runoff hydrograph of different return periods for Opeki River: a) SCS method, b) Snyder method

Figure 7. Runoff hydrograph of different return periods for Otin River: a) SCS method, b) Snyder method
Figure 8. Runoff hydrograph of different return periods for Osun River: a) SCS method, b) Snyder method

Figure 9. Runoff hydrograph of different return periods for Ogun River: a) SCS method, b) Snyder method
the values obtained for SCS method is higher by 39.11% than that of Snyder. For Osun river catchment, the values obtained for SCS method is higher by 13.14% than that of Snyder method. For Ogun river catchment, the values obtained for Snyder method is higher by 52.06% than that of SCS method. This implies that, the percentage difference shows that for values of peak flows obtained by Snyder and SCS methods varies from 13.14% to 63.30%.

The runoff hydrograph for the river catchment based on the unit hydrograph obtained with SCS method are presented in Figures 2a to 9a, while those obtained with Snyder method are presented in Figures 2b to 9b for adoption at the study area.

CONCLUSION

The percentage difference for values of peak flows obtained with Snyder and SCS methods varies from 13.14% to 63.30%. However, SCS method is recommended because it utilized additional morphometric parameters such as watershed slope and the curve number (CN) which is a function of the properties of the soil and vegetation cover of the watershed in the estimation of ordinate required for the development of peak runoff hydrograph in the river watersheds.

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