ROAD DESIGN ON LOW BEARING CAPACITY SOILS

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Abstract. Soil with weak bearing capacity, like peat and organic grounds, is widespread in Latvia. During the geotechnical investigation for road reconstruction projects, in many cases the discovered soils with low physical-mechanical properties are located below the existing road structure. It is a challenge for a road design engineer to find a way how to ensure road load-bearing capacity and prevent the occurrence of various deformations. Various methods and technologies for ground structure reinforcement and stabilization are being developed worldwide. During design of road structures, it is important to analyse the geotechnical situation and to identify the main reasons why deformations could occur in the ground layers under the road structure. Each of the developed technologies for weak and unstable soil reinforcement, stabilization is designed to solve a specific problem. Inaccurately and carelessly identifying the causes of problems, road deformations can affect the performance of the recently designed road and even make the existing problem worse. The aim of this paper is to show the soil strengthening methods used in Latvia and to analyse the benefits and disadvantages of these methods.

Keywords: construction, geotechnical design solutions, low bearing soil, peat, roads, soil reinforcement.

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Introduction

Latvia is in a tectonically stable, geologically ancient area to the northwest of the Eastern European craton, at the same time to the west of the Eurasian lithosphere plate. Three basic elements can be distinguished in Latvia's geological structure: 1) the Proterozoic bedrock, which forms a solid, crystalline bedrock beneath the sedimentary rocks; 2) layered sedimentary rocks; 3) their quartering cover, which was largely formed by icing (Stinkulis, 2019).

The temperate climate of Latvia with more precipitation than evaporation, as well as the gentle wavy terrain and clayey, poorly permeable sediments in the bog valleys are favourable conditions for the formation and development of bogs. Nowadays, bog complexes in Latvia are complex dynamic systems that have been established and developed for thousands of years and are still growing intensively horizontally and vertically, significantly influencing landscape dynamics. Bogs occupy more than 10% of the territory of Latvia, where at least 30 cm thick peat layer accumulated during the last 11 700 years. They are distributed throughout the country, but their location in the natural areas varies. The largest swamps are found in Eastern Latvia, Central Latvia and Seaside Lowlands, where the largest swamps are located – Teiču swamp (14 074 ha), Cenas moorland (10 600 ha) and Great Ķemeri moorland (5000 ha).

Figure 1. Bog distribution in the territory of Latvia
Bogs in Latvia have developed in negative relief forms of the Earth’s surface, mainly due to the action of the last glacial ice and its melting waters (see Figure 1). The exception is the coastal areas where the formation of the terrain was significantly influenced by the geological processes caused by the development stages of the Baltic Sea. The marshes in the territory of Latvia were formed by overland swamping or overgrowing of shallow water bodies, and their formation was significantly influenced by the terrain (Kalniņa, 2019).

Road construction over soil with weak bearing capacity presents great challenges to the intending road builder not only in the landscapes and terrain that have to be crossed but also in the management of the engineering properties of soil, high water content, high compressibility and low strength. The road engineer must overcome these engineering obstacles and considerations of low bearing capacity and excessive settlement in order to be able to construct safe, stable and serviceable road embankments. Soil problems with poor bearing capacity are encountered worldwide, which has led to the development of various methods and technologies for ground structure reinforcement and stabilization over the years. In order to be able to offer the most economical and rational reinforcement solution during the design process, it is necessary to precisely identify the main risks that could affect the road capacity (Guyer, 2018).

1. Objectives

The aim of this publication is to evaluate technologies and methods for road design on low bearing capacity soil, to evaluate the advantages and disadvantages of each method and to look at Latvia’s experience in geotechnical design of roads.

Bearing capacity is the strength of soil to support the loads applied to the ground. The bearing capacity of soil structure system is the maximum average contact pressure between the foundation and the soil, which should not produce shear failure or excessive settlement in the soil.

2. Road construction design on low bearing capacity soils

There are several techniques to improve the bearing capacity of the soil and two major construction strategies, including low bearing
capacity soil removal and left in place. Removal means doing away with the low bearing capacity soil prior to the construction – soil excavation, replacement and displacement. Weak soil left in place entails all the constructions that are undertaken directly on this soil in order to avoid bulk earthworks and the methods are consolidation, ground improvement to reduce deformation, load modification, stabilization and piling methods.

Construction over low bearing capacity soil can essentially be subdivided into five broad classifications:

- rerouting;
- excavation of weak soil and replacement;
- replacement;
- displacement;
- soil left in place.

Excavation can be rated as the safest and most popular option of new or existing road structure over low bearing capacity soil. It is also easier to explain to the customer and to the public about the idea and effectiveness of such a solution. Excavation provide removal of all weak material under the road out to expose a firm layer of sufficient bearing capacity to accommodate the new structure. Thereafter an embankment of appropriate thickness is constructed on the exposed firm layer to enable the design to be fulfilled with a minimum threat of settlement or shear failure (Munro & MacCulloch, 2006).

Methods that leave the low bearing capacity soil in place and avoid the disadvantages of bulk earthworks are now becoming increasingly more attractive to engineers as road construction budgets reduce and more cost-effective solutions are sought. Environmental and waste minimization considerations are also added as advantages for methods that build on the weak soil in place. Methods that leave the weak soil in place can be divided in six groups of techniques that utilize the underlying low bearing capacity soil as a load bearing layer. Those groups are:

- strength improvement
- load modification
- reinforcement
- vertical drainage
- piling
- soil stabilisation

In Latvia until 2018, the weak bearing capacity soil exchange method was the main and mostly used for ground stabilization. In 2018 at one of the road building sites during the construction work significant deformations of the road structure occurred. As a result, construction works were stopped, and additional project solutions were developed.
For the first time in Latvia the combined soil column and pile column construction method had been used (Latvian State Roads, 2018). After the implementation of the project, soil bearing capacity provision has become a hot topic in Latvia.

3. Evaluation of road geotechnical design methods

The selection of a proper solution for the construction or improvement design of a road over weak soil will usually be based on rational considerations, such as acceptable budget and other resources, together with the performance requirements expected for the new structure. The most important thing for a road design engineer is to correctly identify the causes, problems, and factors that could affect the road capacity. The factors which engineer has to consider to rationally and safely choose the right method for ground reinforcement by evaluating the pros, cons and limitations of each method is discussed in (Munro, 2004) (see Table 1).

| Solution                        | Advantages                                                                 | Disadvantages                                                                 |
|---------------------------------|-----------------------------------------------------------------------------|-------------------------------------------------------------------------------|
| Rerouting                       | Avoids potential problems with soils.                                       | Requires alignment revision.                                                  |
| Excavation of weak soil and replacement | Proven, reliable, well known technology                                     | Problems with disposal of excavated material and high quality of fill material. |
|                                 |                                                                             | High water table. Not the cheapest solution                                   |
| Displacement or partial excavation | Proven technology. The displaced weak soil to the sides of the embankment can enhance the embankment stability | Not suitable for all soil types. Requires substantial quantities of fill material for the buried embankment and longer construction time for displacement and surcharge affects to be effective |
| Displacement assisted by water jetting | Established intermediate technology. Does not require weak soil excavation. Should achieve a good bearing capacity on the displaced embankment structure | Same as for displacement or partial excavation                                 |
| Displacement by blasting        | Used together with displacement and/or partial excavation solutions. Established intermediate technology. Does not require weak soil excavation. Should achieve a good bearing capacity on the displaced embankment construction | Same as for displacement or partial excavation. Operating with explosives. Can only be used in open sites with no utilities, etc. |
| Solution                                | Advantages                                                                 | Disadvantages                                                                                                                                 |
|----------------------------------------|-----------------------------------------------------------------------------|-----------------------------------------------------------------------------------------------------------------------------------------------|
| Strength improvement through preloading| Minimizes amount of fill material. Does not require weak soil excavation and disposal | Time needed for preloading can extend construction time. Unpredictable loading schedule and possibility of required double handling. Requires comprehensive site investigation and laboratory testing ahead of works and onsite monitoring system |
| Strength improvement by surcharging     | Improves the bearing capacity of the underlying weak soil so that it can support the weight of the in-service embankment. The time for consolidation and secondary compression can be accelerated | The time needed for surcharging can extend construction time. Requires comprehensive site investigation ahead of works and onsite monitoring system |
| Strength improvement by stage construction | Minimizes secondary compression settlement of the new embankment. Higher embankments can be constructed without shear failure in the underlying weak soil. Does not require weak soil excavation and disposal | Long construction time needed for the various stages to take effect can extend the embankment construction time. Requires onsite monitoring system |
| Load modification by profile lowering   | Less fill material required. Reduces loadings on the underlying weak soil and the amount of land required | Requires a modification of alignment. May not be possible if bridge clearances or other structures are critical. May give problems with bearing capacity of embankment |
| Load modification by pressure berms     | Increases stability and the depth and length of the critical slip circle. Low grade fill material (even weak soil) can be used as fill mass in berms | Requires additional fill material and additional land for the wider construction. Increases the overall weight of the embankment. Consolidation settlements may be increased as a result of the spread of load from the pressure berm |
| Load modification by slope reduction    | Increases stability and the depth and length of the critical slip circle | Requires additional fill material and additional land for the wider construction. Increases the overall weight of embankment. Consolidation settlements may be increased as a result of the spread of load from the wider slopes |
| Load modification by lightweight fill   | Less bearing capacity necessary on the weak soil foundation. Usually does not need the underlying weak soil to be strengthened. Lighter embankment construction generally means less future settlement | Price and transport of the specialized lightweight materials. Design and placing of lightweight materials may require special arrangements. Environmental considerations particularly with groundwater. Bearing capacity of the lightweight embankment may be limited |
| Solution                          | Advantages                                                                 | Disadvantages                                                                                                                                 |
|----------------------------------|-----------------------------------------------------------------------------|------------------------------------------------------------------------------------------------------------------------------------------------|
| Embankment strengthening using    | Limited site disturbance. Provides reinforcement and stability effect for the short and medium term. | The overall settlement of the embankment is not reduced. The geotextile/geogrid requires gently operations. Creep may affect the long-term performance of the geotextile. Geogrid requires higher quality fill material |
| geotextiles and geogrids         | Reduced differential settlements and lateral stresses on the weak surface. Minimizes need for embankment fill material. No excavation and disposal |                                                                                                                                                  |
| Embankment strengthening using    | Limited site disturbance. Provides and stability reinforcement effect for the short and medium term. | The overall settlement of the embankment is not reduced. Can be damaged by construction equipment during placing of embankment fill. High element of manual labour required for fabrication of the raft. Timber raft must be submerged. Relatively unpredictable period of bio segregation |
| timber raft                       | Reduced differential settlements and lateral stresses on the weak surface. Minimizes need for embankment fill material. No excavation and disposal |                                                                                                                                                  |
| Embankment strengthening using    | Limited site disturbance. Provides long term stiff foundation for the embankment. Reduced differential settlements and lateral stresses on the weak surface. Minimizes the need for fill material. No excavation and disposal | Overall settlement of the embankment is not reduced. Relatively long curing time for concrete. High element of manual labour required for fabrication of the raft |
| concrete rafts, galvanized steel  |                                                                                                                                  |                                                                                                                                                  |
| sheeting                         |                                                                                                                                  |                                                                                                                                                  |
| Vertical drainage                 | Reduced time of primary consolidation and secondary compression                                                             | Acceleration of primary consolidation and secondary compression results in significant settlements during the construction period. Performance of drains affected by buckling, heave, smear |
| Piling                           | No excavation and disposal. Limited site disturbance. Minimal or no settlement. No additional time required for surcharge effects | Does not rely on strength of in-situ weak soil. No support assumed from surrounding soil. Usually needs a continuous concrete slab or geotextile load transfer platform. High depths to load bearing stratum. Expenses |
| Mass stabilization method         | No excavation and disposal. Reduces settlements and adds to bearing capacity of the weak soil. Smaller demand of fill material compared to other preloading techniques | The time needed for preloading can extend the construction time. Unpredictable loading schedule. Requires laboratory testing ahead of works and onsite monitoring system |
4. Experience of soil reinforcement methods in Latvia

4.1. Handbook of ground structure reinforcement and stabilization

Until 2019 there have been no recommendations, specifications or manuals in Latvia that summarise and describe the methods of soil consolidation, for example, road embankments over pile foundations, vertical drainage soils consolidation, ground piles under the embankment, etc. solutions in cases where road structure is designed over sections with poor bearing capacity. Then the structure cannot provide sufficient support in its natural way to ensure the necessary road surface stability throughout its intended lifetime. Thus, each designer is guided by his own experience, knowledge, prejudices and safety concepts, which creates the risk that irrational and unnecessarily expensive solutions can be developed, or perhaps cheap solutions that do not meet specific needs (Latvian State Roads, 2019).

In 2019, a handbook about road structure on soil with weak bearing capacity reinforcement and stabilization was developed. It summarises and describes soil reinforcement methods – road embankments over pile foundations, vertical drainage soil consolidation, ground piles under the embankment, etc. The handbook gives solutions in cases where road construction is designed in stages with poor bearing capacity so that it does not provide sufficient support in its natural way to ensure the necessary road surface stability throughout its intended lifetime. The handbook helps solve the common issues to deal with in terms of designing the weak soil reinforcements needed in road construction, and it will certainly be a useful support and assistant for road engineers to design subgrade reinforcement solutions and to rationalize the choice of solutions.

The handbook helps use the existing low bearing capacity soil in the most efficient and rational way to design a road. Procedures and methodologies have been developed that describe how to assess subsoil and how to develop specific feasibility studies, including life cycle cost analysis (LCCA), to provide the required bearing capacity and functionality of the subsoil based on expected traffic loads, geological and climatic conditions, properties of existing soils (Latvian State Roads, 2019).

For each project, the technical and economic justification of methods for soil reinforcement and improvement depends on the choice of
method(s) and its/their functions. In order to evaluate the justification of the use of a particular soil improvement and reinforcement method, it is necessary to evaluate the technical characteristics of the road construction, construction stages of the project and technological requirements, the requirements and quality criteria defined in the project, restrictions and non-technical risk assessment results. The geotechnical survey report and road construction parameter data should be evaluated before comparing and evaluating the optimal soil reinforcement and improvement methods for a specific project. In order to assess the need for soil reinforcement and improvement, the following cases and boundary conditions need to be considered:

- the subsoil cannot provide sufficient bearing capacity or the expected irregular and total deformations will exceed safe road operation requirements;
- steep slopes, retaining walls, etc.;
- need to construct a work platform or a bypass;
- need to identify or define operational requirements for the road structure;
- need to identify time, spatial and environmental constraints;
- need to clarify project-specific and site-specific restrictions;
- need to identify restrictions on the use of different soil reinforcement and improvement techniques;
- need to compare different methods of soil consolidation and improvement (Latvian State Roads, 2019).

4.2. Experience of soil excavation and replacement

In Latvia until 2018, the weak bearing capacity soil exchange method was the main and mostly used for ground stabilization. Such construction technology is expensive, time- and resource-consuming. Examples of the implemented objects:

- **P128 24.48 km to 32.02 km (Sloka–Talsi)** section of motorway, reconstructed in 2017. The road was last constructed during this period in the 1970s and the reconstruction was long delayed because the road surface for almost the entire length of the road was muddy. During construction 63,000 m³ of peat was excavated and replaced with a suitable soil.

- **A12 114.34 km to 125.14 km (Rēzekne–Ludza)** section of motorway, commissioned in 2018. The construction works were complicated by the massive replacement of peat with a load-bearing soil at the base of the road – 300 meters long at a depth of six meters.
- **P5 20.54 km to 25.00 km (Tīnūži–Ogre)** section of motorway, commissioned in 2018. During the road construction a complete reconstruction of the road pavement structure was carried out – soil exchange, construction of new frost resistant road pavement and two asphalt pavement layers. **50 000 m³** of peat at the base of road construction was replaced with sand.

- **P62 44.15 km to 57.54 km (Bašķi–Preiļi)** section of motorway, commissioned in 2018. To stabilize the road foundations, the soil had to be exchanged to a depth of nine meters and was heavily burdened by heavy rain. Due to rains, the soil in the construction areas was dampened; therefore, the passage was difficult, and the traffic disrupted. **120 000 m³** of soil was exchanged within the site.

### 4.3. Pile columns

In 2018 season, the two sections of the national regional motorway Augšligatne–Skrīveri (P32) (47.20 km to 60.29 km and 61.27 km to 71.27 km) had the largest road repairs within the national road network. During the reconstruction of the road from 49.50 km to 50.00 km weak bearing capacity soil – a bog section with a peat layer at a depth of 10 m – was determined. This was revealed by additional research carried out by JSC “Ceļuprojekts”. The possible solutions for soil stabilization were also examined (Latvian State Roads, 2018).

During the road reconstruction project, the geotechnical research was conducted to a depth of four meters. The geological boreholes did not show weak bearing capacity soils, so it was assumed that weak bearing capacity had already been replaced. However, it turned out that there was an overgrown lake beneath the embankment soils. The

![Figure 2. Construction of combined columns and pile columns](image-url)
suspicion arose during the construction work and was confirmed by geological investigation. It was found that there were both peat and sludge under a thick layer of gravel. As a result, at more intense traffic and at higher loads, the road could be deformed, and the investment made would not be sustainable.

There were two options for solving the problem of the weak load-bearing soil:

• excavation of weak bearing capacity of soil up to a depth of 10 m in a sufficiently wide roadway area, including existing communications. It was not only expensive, but there was also the risk to disturb the soil layers near the road. In addition, soil replacement takes time, so it can settle and stabilize, which means that it would not be possible to continue the construction works and finish during the season. Using this method would cost almost one million euros (excluding VAT);

• applying new technology that had not been practiced on the national road network before and strengthening the ground with piles. Similar technology was used for bridges and building foundations, but roads had not had such an experience in Latvia. The cost of this method would be almost 600 thousand euros (excluding VAT), and with the construction of piles and combined columns, it would be possible to proceed immediately with the pavement without delaying the construction of the entire road section.

In order to stabilize this soil, a method of constructing combined columns and pile columns had been used for the first time in the national road network. 30 cm wide and up to 6 m long concrete columns were built, and above them 60 cm wide up to 2 m long gravel/crushed columns. A total of 952 columns were constructed, arranged in a 2.5 m × 2.0 m grid. A team of specialists from Poland came to Latvia to engage in the constructions works (Latvian State Roads, 2018).

4.4. Crushed stone stabilization berm construction

In 2019, the consequences of the road deformations on Peldu Street were eliminated and technical solutions for conservation were developed. Peldu Street is used as the main access point to Sigulda bobsleigh arena. The condition of the road structure was assessed as critical. Slippage of the road structure due to the Gauja water, groundwater and other factors made it dangerous to allow vehicles to drive along Peldu Street.

During the pavement structure monitoring, it was found that the deformations progressed over time and the crack width increased.
In order to prevent a complete collapse of the road structure and to restore the movement of the transport it was necessary to prevent the development of deformations.

Several slope and road reinforcement options were developed for conservation technical solutions. One of the main factors of the project was the limited time to complete the construction works and to restore the traffic on Peldu Street. Due to the limited construction time, the main solution was to construct mineral berm and wrap road construction layers in high-strength geotextiles.

4.5. Gabion wall construction

On the A2 motorway between Riga and Sigulda in 2019 in the area above the Rauņa River, the longitudinal profile of the road was raised, the embankment slopes reinforced with gabion support walls. Gabion support walls were designed for road section 92.34 km to 92.40 km. The existing embankment was constructed of moraine loam (geological survey has a bond Cu = 20 kPa), so there would be a high risk of slope slipping and soil crushing if the vertical load was increased by the additional embankment. The problem was solved with the help of soil anchors, which served to keep the old embankment body from slipping and falling. Gabions were intended to be built on crushed stone. Ground anchors were selected for the load of 350 kN; tensile force 120 kN (U-profile divider beam would be installed in the middle of the gabion to equalize the force). Tensioning was performed after the embankment was created and compacted to the full height of the tensioned gabion. The front of the gabion facade (~50 cm thick) was built after tensioning.

![Figure 3. Deformations on existing roads](image)
the anchors. Behind the walls of the gabions a vertical drainage system was built for draining the surface of the carriageway.

4.6. Consequences of wrong decisions made in road design

The road pavement design shall ensure that the materials used are designed to carry the intended load-bearing capacity by constructing the structural layers in accordance with the minimum requirements.
specified in “Road Specifications”. Each construction project is different due to pavement and ground solutions, technical specifications, different geotechnical, topographic and other conditions.

The road engineer considering all available data is responsible for making the right project decisions. However, in projects often a situation arises when, due to lack of experience or knowledge, the road engineer makes mistakes and chooses an inappropriate road design solution that fails to provide the required quality requirements. An example is the construction work started on the A2 motorway between Riga and Sigulda in 2019. Although the road construction is not over and a new layer of asphalt will be laid in the spring, on some sections of the road have appeared cracks. The road builder points out that the cracks are not caused by the pavement construction, but by the adjacent swamp. It has been found that there is also a layer of peat under the pavement structure and it has not been removed. Only the top of the road has been reconstructed. Old asphalt has been mixed with other components and a cement-related recycled mixture has been made. The road designer considered that the peat layers under existing road structure were sufficiently compacted. Now it is clear that additional geotechnical research will be needed and possibly it will be necessary to make some changes in project solutions. Geotechnical drilling and testing are expected in the spring. Only then it will be possible to understand why these cracks are occurring and to choose the right methods to ensure the longevity of the road structure.

Conclusions

Ground conditions for engineering works can never be totally certain and invariably constitute a significant risk for projects. Some uncertainty will always remain even after the most rigorous design procedures. For a road engineer during project development the analysis of the existing geological situation is one of the most important things to perform. Improperly or mistakenly determining the properties of existing soils can affect the performance of the road structure.

Road designers worldwide are facing poor load-bearing soils. It is a challenge for an engineer to find a way how to ensure road load-bearing capacity and prevent the occurrence of various deformations. Various methods and technologies for ground structure reinforcement and stabilization have been developed worldwide. Each of the developed technologies for weak and unstable soil reinforcement, stabilization is designed to solve a specific problem. Inaccurately and carelessly identifying the causes of problems, road deformations can affect the
performance of the recently designed road and even make the existing problem worse. It is very important to identify the main and most significant risks that can affect the road construction and to choose the most rational reinforcement method.

In Latvia until 2018 the only one ground reinforcement option for road design over peat were used. The national road industry lacks experience and knowledge using other methods. In 2018 season within the national road network P32 during the reconstruction of the road from 49.50 km to 50.00 km weak bearing capacity soil – a bog section with a peat layer at a depth of 10 m – was discovered. In order to stabilize this soil, a method of constructing combined columns and pile columns had been used for the first time in the national road network. 30 cm wide and up to 6 m long concrete columns were built, and above them 60 cm wide up to 2 m long gravel/crushed columns. Saving of 400 000 euros was achieved in this project. After implementation of this project, a group of road engineers started to work on the handbook of ground structure reinforcement and stabilization. It was released in 2019. Engineers in Latvia still have a lot to learn about ground structure reinforcement and stabilization on low bearing capacity soils, but it has been a considerable progress for the past two years. Our knowledge and experience over the years will grow.

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