Crack monitoring using multiple smart materials; fiber-optic sensors & piezo sensors

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\textbf{ABSTRACT}

This article focuses on health monitoring of structures using multiple smart materials. In this research, two fiber-optic sensors, namely fiber Bragg grating (FBG) and fiber-optic polarimetric sensor (FOPS), are investigated for damage detection in the beam specimen. FBG is used for local strain measurement while FOPS is used for global strain measurement. Both FBG and FOPS show significant changes in the strain due to damages in the specimen. Also, at the center of the specimen, piezoelectric wafer active sensor (PWAS) is attached. The electromechanical admittance (EMA) signature of the specimen beam is recorded by PWAS. The changes in the amplitudes of the peaks obtained at various frequencies in this EMA signature are analyzed, and it is shown that the peak amplitudes respond differently to damages and to change in loading. Thus, multiple smart materials (FBG, FOPS, and PWAS) are used to get improved information on the health of the beam.

\textbf{KEYWORDS}

Fiber-optic polarimetric sensor (FOPS); fiber Bragg grating (FBG); piezoelectric wafer active sensor (PWAS); crack/damage monitoring

\section{Introduction}

Large and heavy mechanical and civil structures need to be monitored on a regular basis for cracks or damages. Generally, the strain changes abruptly in these structures due to damages. Structures such as mechanical blades, shafts, aero-wings, and civil structures have to be monitored for strain variations due to cracks and damages\textsuperscript{[1–4]}. Different smart materials are being used to monitor the health of structures. Fiber-optic sensors and piezoelectric wafer active sensor (PWAS) are among the most used smart materials for structural health monitoring (SHM).

In fiber-optic sensors, information is primarily conveyed through the change in phase/polarization/frequency/intensity of input signal. Different parameters such as strain, pressure, temperature, and twist are measured by different fiber-optic sensors for SHM of different structures\textsuperscript{[5–9]}. The fiber-optic sensing technologies are...
attractive in damage detection as they facilitate with real-time nondestructive monitoring of different mechanical and civil structures [10]. Out of all the fiber-optic sensors, fiber Bragg grating (FBG) and fiber-optic polarimetric sensor (FOPS) have been researched the most in the last two decades. FBGs are suitable for local SHM or in other words they are used for local strain measurement [11]. Thus, FBG can only monitor damages only in its vicinity. FBG sensors can be easily embedded into composite structures for their SHM [12]. Recently, small-diameter FBG was developed for the measurement of non-homogenous internal strain fields [13]. FOPS measures strain in the entire length of a structure. The increased strain due to the damage or crack in a structure is picked up by FOPS irrespective of the damage location in the structure. Thus, FOPS monitors damages in different structures globally [14–16]. However, the location of the damage might not be achieved using FOPS.

Piezoelectric materials are active smart materials as they actuate under electric field and also sense the reactions [17,18]. Piezoelectric sensors rely on the piezoelectric effect. PWASs are used for the determination of pressure, acceleration, strain, or force [19–23]. Recently, small and conformal piezoelectric ceramics and wafer transducers, either being surface mounted or being embedded into the structures, have been widely studied in admittance signature and for generating and receiving guided waves for structural integrity monitoring [24,25]. Thus, piezoelectric sensors can play a very important role in SHM industry.

Since a single smart material cannot provide enough information to interpret the damage condition in an engineering structure, multiple smart materials were employed to get a better picture of damage state in the structures [25,26]. This article presents the application of multiple smart materials, FBG, FOPS, and PWAS, for crack monitoring in a fixed-fixed beam. In this research, it is shown that the data obtained from all these three smart materials give improved information on cracks in the beam. FBG monitors cracks in the central region (local monitoring) of the beam, whereas FOPS monitors cracks in a certain length (global monitoring) of the beam. Static and dynamic tests were conducted for global SHM of beam structures. However, FOPS is used for global strain measurement in this research for the first time. Additionally, PWAS’s monitoring ability was also studied for incremental loading and cracks. PWAS’s electromechanical admittance (EMA) signature responds differently to the change in load on the beam and to the cracks happening in the beam. The impact of the cracks on EMA signature is abrupt and higher, while on the other hand, loading makes a steadier and lower impact on the EMA signature.

2. Theory
2.1. FBG
A Bragg grating is a periodic structure of the refractive index, fabricated by exposing a photosensitized (or hydrogenated) fiber to a high-power pulsed excimer laser (248 nm) through a phase mask. When the light from a broadband source is launched into the core of the FBG fiber, a single wavelength is reflected back whereas the rest of the signal
is transmitted. This reflected wavelength is known as Bragg wavelength ($\lambda_B$) and it is given by

$$\lambda_B = 2n_{\text{eff}}\Lambda,$$  \hspace{1cm} (1)

where $n_{\text{eff}}$ is the effective refractive index of the mode propagating along the fiber and $\Lambda$ is the grating period of the FBG. Equation (1) suggests that if FBG goes through any longitudinal strain, the grating period ($\Lambda$) of FBG will vary resulting in the Bragg wavelength shift. Thus, the longitudinal strain shifts the Bragg wavelength through expansion or contraction of the grating periodicity [27]. This Bragg wavelength shift is read by the FBG interrogator, and the corresponding strain is calculated as

$$\varepsilon = \frac{\Delta\lambda_B}{\alpha},$$  \hspace{1cm} (2)

where $\Delta\lambda_B$ is the shift in the Bragg wavelength and $\alpha$ is the wavelength-strain sensitive factor and its value is 0.67 $\mu\varepsilon^{-1}$ pm for 850 nm.

### 2.2. FOPS

In FOPS, high birefringent (Hi-Bi) fiber-like, bow-tie fiber is used. In Hi-Bi fiber, the light travels in two mutually orthogonal modes of polarization: fast mode and slow mode. Fast mode travels faster than the slow mode in the birefringent medium. The polarization of light coming out of Hi-Bi is fixed. This difference in velocities introduces an additional phase between both the components. In case of Hi-Bi fiber, this additional phase is given by

$$\phi = \Delta\beta L,$$  \hspace{1cm} (3)

where $L$ is the length of the fiber and $\Delta\beta (=\beta_{\text{fast}} - \beta_{\text{slow}})$ is the difference in propagation constant between two orthogonally polarized modes which travel along fast and slow axes, respectively. $\Delta\beta$ is called the ‘birefringence’ of the fiber. From Equation (1), it is clear that there are two factors which can cause a change in the additional phase; one is the change in the length (or axial strain) of the fiber core, $L$, and another is birefringence ($\Delta\beta$) of the Hi-Bi fiber used. Upon the application of longitudinal strain, the length $L$ of the core of Hi-Bi fiber changes which means the phase ($\phi$) between both the fast and the slow modes changes. As a result, the polarization of the light coming out of the fiber changes. If the longitudinal strain is produced increasingly in the Hi-Bi fiber, the polarization of the light coming out of the fiber changes sinusoidally. Since the light finally passes through an analyzer, the output intensity (or voltage $V$) of the light which is measured by the photodiode also changes sinusoidally [15]. In the loading test conducted with FOPS, a normalized output parameter called state of polarization (SOP) is calculated, which is given by

$$\text{SOP} = \frac{V - V_{av}}{V_R},$$  \hspace{1cm} (4)

where $V_{av} = (V_{\text{max}} + V_{\text{min}})/2$ is the average photodiode voltage and $V_R = (V_{\text{max}} - V_{\text{min}})$ is the range of the variation of the photodiode voltage from first loading step to last.
loading step. Following photodiode voltage $V$, SOP also changes sinusoidally. The cycle of SOP variation depends on the beat length ($L_b = 2\pi/\Delta \beta$) of the Hi-Bi fiber used. A phase change of $2\pi$ in SOP cycle means a change of $L_b$ in the length of Hi-Bi fiber used as FOPS. The SOP cycle is segmented to calculate total strain in the specimen [28].

2.3. PWAS

In PWAS, the damage/load monitoring is based on EMA signature. The presence of damage/load on the host structure affects its impedance. In other words, the admittance which is the inverse of impedance is affected by the presence of damage or load. The admittance of the PWAS can be measured by an electrical analyzer, such as the Agilent’s LCR (inductance – L, capacitance – C, and resistance – R) meter.

An alternative voltage signal of 1 V root mean square is applied to the bonded PWAS patch (Figure 1) over the user-specified frequency range. The change in extracted admittance signature is an indication of structural loading or damage occurrence [25]. The admittance signature comprising of peaks is a function of the stiffness, mass, and damping of the host structure. The admittance signature is also the function of the properties of PWAS patch and the additive glue with which it is bonded [25,29]. In principle, the EMA-based SHM technique is similar to the conventional global vibration techniques. The major difference is only with respect to the frequency range employed. The frequency is typically <100 Hz for the global vibration technique while it is <400 kHz for EMA-based SHM.

The amplitudes of peaks obtained in the admittance signature of PWAS are proportional to the natural frequencies of the host structure, and shifting of these peak amplitudes indicates changes in natural frequencies of the structure. The change in the frequencies arises due to crack/load in the structure. This technique employs low-cost and low-power PWAS patches which can be non-intrusively bonded to the structure and can be interrogated without removal of finishes. A one-dimensional approach to model PWAS–structure interaction has already been proposed [30]. The admittance of the PWAS sensor is expressed as

![Figure 1. Schematic of specimen with multiple smart materials.](image-url)
\[ Y(\omega) = j\omega \frac{w_{a}l_{a}}{h_{a}} \left( \varepsilon_{33}^{T} - d_{31}^{2} \bar{Y}^{E} \right) + \left( \frac{Z(\omega)}{Z(\omega) + Z_{a}(\omega)} \right) d_{31}^{2} \bar{Y}^{E} \left( \frac{\tan kl_{a}}{kl_{a}} \right), \]  

(5)

\[ Y(\omega) = G + jB, \]  

(6)

where \( Z_{a} \) and \( Z \) are the mechanical impedances of the PWAS patch and the host structure, respectively; \( j \) is the imaginary unit; \( \omega \) is the angular frequency of the driving voltage; \( w_{a}, l_{a}, \) and \( h_{a} \) are the width, length, and thickness of the PWAS patch, respectively; \( \bar{Y}^{E} = Y^{E}(1 + j\eta) \) is the complex Young’s modulus of the PWAS material at zero electric field; \( d_{31} \) is the piezoelectric constant; \( \bar{\varepsilon}_{33}^{T} = \varepsilon_{33}^{T}(1 - j\delta) \) is the complex dielectric constant; \( \eta \) and \( \delta \) denote the mechanical loss factor and the dielectric loss factor of the PWAS material, respectively; \( \kappa \) is the wave number which is related to the angular frequency of excitation \( \omega \) by \( \kappa = \omega \sqrt{\rho / \bar{Y}^{E}} \); and \( \rho \) is the material density of the structure. \( G \) is a real component of admittance, i.e. conductance, and \( B \) is an imaginary component of admittance, i.e. susceptance. Equation (5) indicates that the admittance of PWAS patch is directly related to the mechanical impedance of the host structure. Therefore, any change in the admittance signatures is the indication of a change in the structural integrity. The signature comprises of both the conductance and the susceptance.

### 3. Experiments and results

Multiple smart structures are used to get improved information on damage state in an aluminum beam. The dimensions of the specimen are \( 800 \times 50 \times 2.67 \text{ mm}^3 \) and three smart materials, FBG, FOPS, and lead zirconate titanate (PZT), are bonded on the specimen. The schematic of the cross-section of the specimen is shown in Figure 1. Each smart material has its own instrumentation system. FBG uses an interrogator which includes a light source. For FOPS, the light source and data acquisition (DAQ) system are different as illustrated in Figure 1. PZT signal is obtained through LCR meter. All the important properties of FOPS, FBG, and PZT patch are given in Table 1. The beam was fixed at both the ends, and the load was applied at the center of the specimen beam through a hanger (130 g) as shown in Figure 2. FBG and PZT are at the center, and the length of FOPS is 45 cm as highlighted. The picture of actual experimental setup with all the three different instrumentation is shown in Figure 3. First, the beam was loaded

| Properties for PZT and Al | Values for PZT | Values for Al | Property of FBG | Values | Properties of FOPS | Values |
|---------------------------|----------------|--------------|----------------|--------|-------------------|--------|
| Young’s modulus           | \( 63 \times 10^9 \) N/m\(^2\) | \( 68.95 \times 10^9 \) N/m\(^2\) | Bragg wavelength \( (\lambda_{B}) \) | 870 nm | Beat length \( (L_{b}) \) | 2.8 mm |
| Poisson’s ratio           | 0.31           | 0.33         | Wavelength-strain-sensitive factor \( (\alpha) \) | 0.67 \( \mu\text{e}^{-1}\) pm | Fiber length \( (L) \) | 45 cm |
| Density \( (\rho) \)      | 7700 kg/m\(^3\) | 2715 kg/m\(^3\) | Grating size | 1 cm |
| Radius \times thickness   | 10 mm \times 0.8 mm | | | |
| Damping coefficient       | 0.34           | | | |
| Piezoelectric constant \( (d_{31}) \) | \( -1.75 \times 10^{-10} \) m/V | | | |
incrementally in the steps of 200 g using weights, and the measurements were taken using FBG interrogation system, FOPS’s DAQ system, and PWAS LCR meter. Later, all the weights were removed and a crack was made at location 1 as shown in Figure 1. Again the beam was loaded in the same loading steps and readings were acquired from all the three sensing systems. Furthermore, additional cracks were made at locations 2, 3, 4, and 5 one by one and the readings were acquired every time in the same loading steps.

### 3.1. Monitoring with FBG

The strains measured by FBG at every load step for every crack scenario are shown in Figure 4. From ‘No crack’ to ‘Cracks1, 2, 3, and 4’, the load versus strain lines are almost overlapping as the cracks are at locations which are too far to be detected by FBG. However, the slope of the load versus strain line increases significantly when a crack is made at location 5 (central location) which is within the detection range of FBG. Thus, FBG measures strain in the central region of the beam providing crack information in the central part of the beam only.

![Figure 2. Schematic of experimental setup with cracks at locations 1, 2, 3, 4, and 5.](image)

![Figure 3. Lab experimental setup.](image)
3.2. Monitoring with FOPS

In the case of FOPS, the voltage measured by DAQ system is recorded with increasing load for each crack. Furthermore, the SOP is calculated using Equation (2) and then it is plotted against the increasing load as shown in Figure 5. SOP varies sinusoidally as the load progresses. Though it is difficult to infer anything directly in Figure 5, the cycle period of SOP decreases when the cracks occur in a part of the beam where FOPS fiber is present. Since the beat length of the Hi-Bi fiber used here in this experiment is 2.8 mm, one complete cycle of SOP variation corresponds to 2.8 mm increment in the fiber length or specimen length. The total length of the FOPS used in this experiment is 45 cm as shown in Figures 1 and 2 which means that FOPS measures the total strain produced in the whole 45 cm length of the specimen beam. The SOP cycles corresponding to different crack scenarios (from ‘No crack’ to ‘Cracks 1, 2, 3, 4, and 5’)) are shown in Figure 5. Each SOP cycle can be segmented to calculate the total longitudinal strain.

Figure 4. Strain measured by FBG for different crack scenarios.

Figure 5. SOP cycles of FOPS for different crack scenarios.
45 cm length of the specimen. Thus, FOPS can be used to calculate the total strain over a certain length. The total strain in 45 cm length is plotted against increasing load for different crack scenarios in Figure 6. The load versus strain lines corresponding to ‘No crack,’ ‘Crack 1,’ and ‘Cracks 1 and 2’ are almost overlapping. This is because of the fact that in these cases, the cracks are outside the 45 cm length of FOPS (see Figure 2 for crack locations). When additional cracks are induced in the part of the specimen where FOPS is present (Crack locations 3, 4, and 5), the slope of the load versus strain line increases significantly which is apparent in Figure 6. Thus, FOPS measures strain in its entire length, leading to global strain monitoring for the considered specimen.

3.3. Monitoring with PWAS

EMA signatures of the specimen were obtained for every loading step (from 0 to 1930 g) for the frequency range from 10 to 150 kHz at a sweep step of 0.25 kHz. This process was repeated for every crack scenario (from ‘No crack’ to ‘Cracks 1, 2, 3, 4, and 5’). EMA signatures with increasing loads for ‘No crack’ condition are shown in Figure 7. EMA signatures are too condensed to be seen clearly. Therefore, signatures are segmented in four frequency ranges as to study the movement of peaks properly. An expanded view of EMA signatures in range 1 (66–75 kHz) and range 2 (78–83 kHz) is shown in Figure 8. Similarly, the expanded view of range 3 (90–125 kHz) and range 4 (125–150 kHz) is shown in Figure 9. From Figures 8 and 9, it is clear that the peaks are either going down or going up and their movement is very consistent. This indicates that if the load increases progressively, the peaks will continue to move in one direction.

Furthermore, an analysis was carried out to assess the importance of EMA signatures for load/crack monitoring. This is done by looking at the pattern of root mean square deviation (RMSD) index [31]. RMSD is used to compare the signatures of two different states. RMSD has been widely accepted and used for estimation of the amount of variation between two signatures. It is defined as
\[
\text{RMSD} - k(\%) = \sqrt{\frac{\sum_{i=1}^{N} (y_i^k - x_i^1)^2}{\sum_{i=1}^{N} (x_i^1)^2}} \times 100, \quad (7)
\]

where \(i (=1, 2, 3 \ldots N)\) represents the peak number in the signature and \(k (=2, 3, 4 \ldots N)\) represents the loading step number. \(x_i^1\) is the amplitude of \(i\)th peak at base loading step which is the first loading step here in this case and \(y_i^k\) is the amplitude of the same \(i\)th peak at \(k\)th loading step. In this study, the RMSD essentially compares the base loading step (or the first loading step) signature with the later signature with \(k\)th loading step, i.e. the RMSD indices are calculated with respect to (w.r.t) the first loading step. The values of RMSD indices with increasing loading steps for all the crack scenarios are given in Table 2. From this table, it is clear that RMSD indices increase (column wise)

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**Figure 7.** EMA signatures with increasing loads for ‘No crack’ condition.

**Figure 8.** Expanded region of EMA signature in range 1 and range 2.
consistently with increasing load for every crack scenario or in other words increasing RMSD index is an indication of increasing load on the structure. However, the RMSD values (row wise) with increasing number of cracks do not make any sense. This reflects to the fact that the boundary conditions of the beam get disturbed abruptly after every crack as every crack is made manually using a fine saw. Making crack using saw disturbs the base line every time.

The EMA signatures with increasing number of cracks for no load condition are shown in Figure 10. The process of making cracks creates a lot of disturbances in the specimen; therefore, the EMA signatures are disturbed and do not show any pattern similar to EMA signature with increasing load (see Figure 7). To study the effect of cracks on EMA signature of the specimen, the RMSD index is calculated for different crack scenarios. Differently from classical application of Equation (7), it is here proposed to consider index $k$ related to the number of cracks rather than the loading step number. This time EMA signature corresponding to ‘No crack’ condition was considered to be the base step ($x_1$) and the crack scenarios from ‘Crack 1’ to ‘Cracks 1, 2, 3, 4, and 5’ were taken with $k = 2$ to $k = 6$, respectively as shown in Figure 10. Hence, the RMSD indices

| Crack scenario | Load steps (g) | No crack | Crack 1 | Cracks 1 and 2 | Cracks 1, 2, and 3 | Cracks 1, 2, 3, and 4 | Cracks 1, 2, 3, 4, and 5 |
|----------------|---------------|---------|---------|---------------|-------------------|----------------------|------------------------|
| 130 (base load step) | 330 ($k = 2$) | 3.95 | 4.94 | 5.88 | 1.65 | 1.77 | 1.55 |
| | 530 ($k = 3$) | 7.34 | 5.9 | 8 | 2.61 | 2.16 | 2.79 |
| | 730 ($k = 4$) | 8.66 | 5.95 | 8.63 | 3.17 | 2.86 | 3.97 |
| | 930 ($k = 5$) | 8.88 | 6.177 | 8.81 | 3.66 | 3.5 | 5.18 |
| | 1130 ($k = 6$) | 9.86 | 6.725 | 8.91 | 4.21 | 4.05 | 6.12 |
| | 1330 ($k = 7$) | 10.29 | 7.42 | 9.21 | 4.89 | 4.56 | 7.08 |
| | 1530 ($k = 8$) | 11.08 | 8.177 | 9.6 | 5.59 | 5.09 | 8.07 |
| | 1730 ($k = 9$) | 11.75 | 8.83 | 10.19 | 6.28 | 5.69 | 8.87 |
| | 1930 ($k = 10$) | 12.35 | 9.46 | 10.59 | 6.91 | 6.19 | 9.44 |

Figure 9. An expanded view of EMA signature in range 3 and range 4.
were calculated with respect to ‘No crack’ signature. The RMSD indices with increasing cracks are given in Table 3. These signatures were obtained when there was no load. Cracks are induced at random locations thus their RMSD trends are also random. Clearly, the RMSD index increases drastically with increasing damage in the specimen.

The peaks in EMA signature of the specimen move consistently in one direction, and the value of RMSD index increases gradually with increasing load (Figure 7). On the other hand, the movement of peaks in EMA signatures is larger with visible disturbances with increasing damage in the specimen (Figure 10). Hence, the value of RMSD index increases significantly with increasing damage. In the latter case (Figure 10), only ‘No Load’ condition is considered as it is the case when the disturbance in the boundary conditions due to crack making process is minimum. The disturbances are higher when the beam is loaded, and therefore, the RMSD values change abruptly rather than consistently.

4. Conclusions

One smart material or sensing technology is not sufficient for damage monitoring in a structure. Improved information on damage can be obtained by using multiple smart materials on a structure. In this research, three smart materials, FBG, FOPS, and PWAS, are used for damage/load monitoring in an aluminum beam. FBG can monitor damages
in a very specific region (the central region in this case) through local strain monitoring. On the other hand, FOPS can monitor damages in a pre-decided length (45 cm in this case) of the specimen through global strain monitoring. In the case of PWAS, if the variations in EMA signatures are low but consistent, this will implicate the loading or unloading of the specimen. If EMA signature varies substantially but inconsistently, it will implicate the damage occurrence in the specimen. Thus, EMA signatures differentiate between loading and damage occurrence. The RMSD index calculated on the basis of number of cracks implies that damages cause a higher impact on RMSD index value. Thus, RMSD quantifies the severity of damage in the specimen as it increases by more distinguishable amount with an increase in damages. Multiple smart material employment has a great scope for further investigation.

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