Research article

ERT-based Investigation of a Sinkhole in Greene County, Missouri

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Abstract: Investigating sinkhole morphology and formation mechanisms is key to understanding their long term impact and susceptibility to development, and aids in the design of effective mitigation measures. In this study, ERT (electrical resistivity tomography), MASW (multichannel analysis of surface waves) and borehole data were used to image the subsurface morphology of an active sinkhole in Greene County, Missouri. The study reveals that the sinkhole developed along a natural surface drainage pathway above a pervasively fractured limestone. The subsurface image of the sinkhole depicts a zone of near-vertical water seepage and soil piping. Based on the nature of the overburden material, and the morphology and current/past surface expression of the sinkhole, it is concluded that the sinkhole is predominantly a cover subsidence type of sinkhole. However, it is possible that minor cover collapse occurred locally and in an area slightly to the north of the current active sinkhole.

Key words: sinkhole; ERT; MASW; piping; borehole control; subsidence

1. Introduction

Greene County, Missouri, is part of the Ozarks physiographic region and is known for its karst terrain. Karst terrain forms in carbonate and evaporitic rocks, primarily by dissolution and is typically characterized by numerous sinkholes, losing streams (swallow holes), springs, caves, and other related features. Because of this, karst areas are one of the most challenging environments in terms of groundwater, engineering and environmental issues [1]. Sinkholes are one of the most common karst structures in the world [2] and constitute a major hazard in karst areas [3, 4].

Greene County Missouri is known for the presence of karstic features such as caves, springs and, more importantly, sinkholes [5]. It is reported that more than 2500 sinkholes and 245 caves have been
identified in Greene County (Greene County Comprehensive Plan, page 52, 2007). As stated by Carbonel et al. in [6], catastrophic collapsing sinkholes may lead to injury, fatalities and cause significant damage to infrastructure. For example, the sinkhole that occurred in Nixa, Missouri, on August 13, 2006, swallowed a car, the garage it was parked in, and part of the adjoining house [7].

Geology, hydrology, and anthropogenic factors have an impact on the formation of sinkholes [7–9]. Hence, an understanding of the interaction between these factors assists in determining where and how a sinkhole may form. Moreover, the investigation of the formation mechanism and sinkhole morphology allows for design of applicable mitigation measures. Investigation of existing sinkholes is necessary from an engineering standpoint to locate and characterize the source of water, seepage/piping pathways, voids (if present) and variable depth to top of rock. As stated by Gutiérrez in [10], effective sinkhole investigation should integrate a variety of investigative approaches that include geological, geophysical, and geomorphological analysis. The author’s perspective is that any sinkhole can be mitigated using appropriate engineering technologies if the flow of piping water is effectively cut-off.

Geological analysis and geophysical methods can assist in characterizing sinkhole morphology, evolution and formation mechanisms, while geomorphological methods assist in the understanding of recent sinkhole activity and human influences [2]. Geophysical methods are often particularly useful as there is usually a good contrast between the physical properties of the sinkhole fill, which consists of either water, air, or soil, and the surrounding less disturbed strata. Geophysical methods that are commonly used for sinkhole investigation include seismic reflection and refraction [11], gravimetry [12], ground-penetrating radar [13, 14], electrical resistivity tomography [15–18], and multichannel analysis of surface waves [19, 20].

Electrical resistivity tomography (ERT) is routinely used in Missouri to image the shallow subsurface in karst terrain because undisturbed soil, carbonate rock, clay in-fill, and air-filled cavities are generally characterized by very high resistivity contrasts [5]. Multichannel analysis of surface waves (MASW) is also often a very appropriate method for sinkhole investigation, because variations in shear-wave velocity can be used to differentiate between various types of unconsolidated soils and bedrock [21].

In this research ERT and MASW techniques were employed together with a confirmatory boring to effectively characterize the subsurface morphology of an active sinkhole, hereafter referred to as Sinkhole_1. Moreover, historical maps were analyzed to reveal the evolution of the karst feature and land use changes. Analyses suggest the sinkhole developed along a natural north-south surface drainage pathway. Furthermore, the subsurface structure of the sinkhole depicts a vertical zone of moisture flow and associated soil piping. From the nature of the overburden material and the characteristics of the sinkhole, it is concluded that the sinkhole is predominantly a cover subsidence sinkhole (gradual subsidence) based on the sinkhole classification system described by Waltham et al. in [22]. Historical photographs suggest that cover collapse could have occurred in an area slightly to the north of the current active sinkhole.

2 Location and geology of study area

Sinkhole_1, is located in Greene County, Missouri (Figure 1). The geology of Greene County comprises thick Mississippian-age limestones and cherty limestones underlain by Ordovician and Cambrian-aged strata (Table 1). Greene County lies on the western side of the Ozark Uplift and the rock layers regionally dip gently towards the west with minor faulting and folding.

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Figure 1: Location Map of the study area: the blue color region represents Greene County, Missouri.

The Mississippian age Burlington-Keokuk Limestone is the dominant bedrock exposed in the study area (Figure 2). In this bedrock, layers of limestone are interbedded with thin layers of chert and the presence of chert nodules within limestone layers.

The Burlington-Keokuk Formation is up to 270ft (82.3m) thick [23] but varies in thickness from place to place due to erosion. The limestone is a light gray, coarsely crystalline, and nearly pure calcite which is highly susceptible to solution. Uneven dissolution of this formation has resulted in highly irregular bedrock-overburden interface [24]. This limestone bedrock is mainly characterized by the formation of prominent knobs (pinnacles) of bedrock bounded by deep troughs (grikes) caused by dissolution in pre-existing fractures. The thickness of residuum is highly variable, in many areas stream erosion has removed the residuum and rock is at or very near to the surface, whereas in other areas it reaches to a thickness of about 40ft (12.2m) [25].

The limestone bedrock in Greene County was subjected to tectonic forces and has undergone some structural deformations during the Ouachita Orogeny. Orndorff in [26] has mentioned that the geological structures formed from this deformation appear to have controlled the development of karst. Generally, the faults in the study area are oriented northwest and northeast [27]. Joints are common structural features, similar to faults, where lateral and vertical displacements have not occurred. McCracken in [27], states that the bedrock in the study area is characterized by two nearly orthogonal joint sets that exhibit general strike orientations: N 20° W, and N 60° E and with vertical dipping.
| System    | Series    | Group   | Formation                        | Thickness (ft) |
|-----------|-----------|---------|----------------------------------|----------------|
| Mississipian | Osagean   |         | Burlington-Keokuk Formation      | 150-270        |
|           |           |         | Elsey Formation                  | 25-75          |
|           |           |         | Reeds-Spring Formation           | 125            |
|           |           |         | Pierson Formation                | 90             |
|           | Kinkerhookian | Chouteau | Northview Formation              | 5-80           |
|           |           |         | Compton Formation                | 30             |
|           |           |         | Cotter Formation                 | 600            |
| Ordovician | Canadian  |         | Jefferson-City Formation         |                |
|           |           |         | Ruobidoux Formation              | 150            |
|           |           | Gasconade Formation              | Upper Gasconade Dolomite | 350 |
|           |           |         | Lower Gasconade Dolomite         |                |
|           |           |         | Gunter Sandstone Member          | 25             |
| Cambrian  | Upper     | Elvins  | Eminence Formation               | 500            |
|           |           |         | Potasi Formation                 |                |
|           |           |         | Derby-Doerun Formation           |                |
|           |           |         | Davis Formation                  | 150            |
|           |           |         | Bonneterre Formation             | 200            |
|           |           |         | Lamotte Formation                | 150            |
| Precambrian |          | Crystalline rock |                              |                |

**Table 1:** Geologic and stratigraphic units in Greene County [23].

Karst features are prevalent almost throughout Greene County. The Burlington-Keokuk Limestone has been extensively affected by solution process resulting in the formation of numerous karst features: caves, springs, sinkholes, losing streams, cherty clay residuum, etc. As stated by Ismail and Anderson in [5], the sinkholes are formed when carbonic acid from atmospheric carbon dioxide, present in rainwater, percolates downwards into the subsurface and dissolves carbonate bedrock, enlarging fractures and joints into cavities that in most cases were in-filled with piped fine-grained soil as they developed, resulting in gradual subsidence at the surface.
3 Assessment of the sinkhole

3.1 Aerial photos

An aerial photograph from 1960 and a series of historical google earth images (Figure 3) were analyzed to reveal the evolution of Sinkhole_1, anthropogenic factors and land use changes. The 1960 aerial photo on the top left of figure 3 shows a north-south elongated feature which is a row of trees along a natural north-south surface drainage pathway in the middle of a farmer’s field. To the north of the row of trees, there is a small surface depression. This suggests that Sinkhole_1 developed at this location originally as a result of the localized ponding of surface water immediately to the north of the zone of dense vegetation. The ponded water and piped fine-grained sediment percolated into the subsurface through the soil and underlying pervasively fractured limestone. The piping of soil lead to surface subsidence and the enlargement of the sinkhole over time.
Figure 3: Historical Aerial photos of Sinkhole_1; the paved road ways can give an idea about the scale of the images.

The second image, a Google Earth® image from March 1997, shows a well-defined circular depression, immediately west of the row of trees imaged in the 1960 photograph. The circular depression is the surface expression of the Sinkhole_1 in 1997. The sinkhole appears to have migrated to the west as a result of the broadening of the surface expression of the original area of subsidence. The observation that the zone subsidence is covered by vegetation and not characterized by scarp features indicates Sinkhole_1 is predominantly a cover subsidence type of sinkhole. The Google Earth® image from April 2003, shows a small surface depression to the north of the main circular depression. The steep angle and the lack of vegetation/grass cover on the scarp in the collapse feature suggest that this collapse may have occurred relatively abruptly. It is possible that this depression is a localized cover collapse feature. Cover collapse is a typical feature of sinkhole development in cohesive soils where the covering sediments contain significant amount of clay. The sediments spall into a cavity and as spalling continues, the cohesive covering sediments form a structural arch and eventually the cavity breaches the ground surface, resulting in sudden and dramatic collapse usually with steep angle scarps.

The series of historical images indicate that Sinkhole_1 is not an instantaneous collapse type of sinkhole; rather it appears to have developed gradually and evolved over time. Sinkhole_1 is therefore classified as cover subsidence. It appears to have initiated about the time the 1960 photograph was taken.
3.2 Borehole control

One borehole (BH1) was drilled to facilitate the correlation of the ERT profiles to the actual subsurface geology. The drilling was advanced to the bedrock surface using 8.5 inch (21.6cm) O.D. hollow-stem augers and bedrock was cored using HQ core barrels.

The borehole (BH1) was drilled along ERT profile T13 and ties at 97.5m mark on the profile (Figure 5 and 6a). It was drilled to a depth of 30m below ground surface. The first 2m comprises red clay residuum with chert, with brown silty loam with chert from 0 - 1.1m; 1.1m - 1.8m comprises red silty high plasticity clay, and finally 1.8m - 2m reveals low plasticity clay. Underlying the residual soil is Burlington-Keokuk Limestone, characterized, in core specimens, by numerous horizontal fractures. The rock quality was found to be fair to excellent.

The decomposition of the beds of limestone and chert formed a highly ferruginous deposit of clay mixed with broken and decomposed chert. The broken and decomposed chert gives more porosity to the residual clay soil. Furthermore, the high iron content of the ferruginous clay residuum causes flocculation and form blocky aggregates resulting in increased porosity of the soil. Therefore, as a result of the presence of chert fragments and the flocculated clay structure, the residuum clay soil has higher permeability than expected from a more uniform clay soil. From the borehole samples it is evident that the clay content increases with depth, and this supports the idea that there is piping of fine grained soils.

3.3 Multichannel Analysis of Surface Waves (MASW)

Multichannel analysis of surface waves (MASW) data were acquired in proximity to Sinkhole_1 and along west-east oriented ERT profiles (Figures 4 and 5). A 24 channel geophone array connected to a seismograph is used to record the seismic data. As presented in Figure 5, six MASW profiles (MASW1, MASW2, MASW3, MASW4, MASW5 and MASW6) were generated and used to verify and constrain the interpretation of the ERT profiles. The NEHRP (National Earthquake Hazard Reduction Program) site classification chart for different geological material, as published in 2000 by the International Building code, provides a basis for the classification of subsurface materials based on their shear wave velocity values. Based on this chart and the borehole control, the shear wave velocity of soil is generally less than 1200feet/sec (366m/sec). An example dispersion curve and corresponding 1-D shear wave velocity profile from MASW5 is presented as Figure 4.
**Figure 4:** A dispersion curve (left); and 1-D shear wave velocity profile (right) of MASW5 presented as sample; acquisition parameters used are 10ft (3.05m) off-set, 5ft(1.52m) geophone spacing, and aligned E-W. Red color arrow indicates interpreted depth to top of rock.

**Figure 5:** Alignment and location of acquired geophysical data and borehole control: West-east oriented blue lines represent ERT traverses (254.5m long each). The red color circle represents the approximate location of the surface expression of Sinkhole_1.
3.4 Electrical Resistivity Tomography (ERT)

Sixteen west-east oriented 2D electrical resistivity tomography (ERT) profiles were taken at the site with the intent of imaging and characterizing the shallow subsurface in the vicinity of Sinkhole_1. The ERT profiles were acquired along sixteen west-east oriented traverses spaced at 6.1m (20ft) intervals. The traverses are labeled T1, T2, T3, to T16 (Figure 5). Each 2D-ERT profile extends to a maximum depth of 36.5m (120ft). The west-east direction was selected as it is nearly perpendicular to major joints and regional geological structures in the study area, which have a general approximately north-south orientation. ERT data were acquired using an AGI R-8 Supersting multi-channel and multi-electrode resistivity system with 168 electrodes spaced at 1.52m (5ft) intervals and using a dipole-dipole electrode array.

The borehole control and MASW interpretations were superposed on the respective ERT profiles to help verify the interpretations of the electrical resistivity images. Based on the borehole data (BH1) (Figure 6a), the top of weathered rock corresponds approximately to the 125 ohm-m resistivity contour interval on ERT profile T13. Therefore, the resistivity contour value of 125 ohm-m is interpreted as the top of weathered rock on all of the other ERT profiles. Further, BH1 overlain (superposed on T13; Figure 6a) indicates that the zone of relatively low electrical resistivity (Zone A) is not a soil/water filled cavity, but rather fractured and weathered rock with fair to good quality. Therefore, the relatively low resistivity values (less than 125 ohm-m) are attributed to the presence of moisture and/or clay filled fractures. Hence, Zone A is interpreted as a moist fractured rock with some clay infilling fractures. MASW5 ties ERT T13 at 61m mark (Figure 6). The “resistivity” top of weathered rock at the MASW5 location is estimated to be 20ft (6.1m). The 1-D shear wave velocity profile in MASW5 (Figure 6b) shows an abrupt increase of velocity from 1300ft/sec (396m/sec) to 1650ft/sec (503m/sec) at a depth of 20.5ft (6.2m). This increase presumably marks the boundary between dense residual soil and the top of weathered rock and is interpreted as the “acoustic” top of rock. The additional acquired MASW data also correlate with ERT profiles, in terms of estimated depths to top of rock. Figure 7a shows MASW1 tied to the ERT profile T1 at 183m mark; the “resistivity” and “acoustic” top of weathered rock are 2.4m and 2.1m respectively. In Figure 7b, Zone B has very low electrical resistivity (less than 125 ohm-m), but its average shear wave velocity in MASW4 is about 1800ft/sec (549m/s). Similarly, the low resistivity zone (Zone C) in Figure 7c, is characterized by a shear wave velocity ranging from 1450ft/sec (442m/sec) to 1800ft/sec (549m/sec). These values of shear wave velocity are consistent with that of fractured rock. Moreover, strata with comparable resistivity values and similar geological conditions encountered in the borehole was fractured rock (Figure 6a). Hence the most plausible interpretations of zones B and C is that they are zones of fractured rock probably with moist clay infill.

Based on control available, the following interpretational guidelines were established: moist soils are characterized by resistivity values of less than 125 ohm-m; dry soils by resistivity values greater than 125 ohm-m; moist weathered and/or fractured rock by resistivity values less than 600 ohm-m; moist fractured rock with moist piped clay/soil-fill by resistivity values less than 125 ohm-m; and drier, possibly less weathered rock by resistivity values greater than 600 ohm-m. Large air filled cavities should be characterized by very high resistivity values, but dependent on the conductivity of the surrounding material and depth/size/shape of the void. The ERT data acquired at this study indicated that the active sinkhole is not underlain by any substantive air-filled cavities. Four ERT profiles crossing...
the sinkhole (T7, T8, T9, and T10), with interpreted depths to moist weathered rock and depths to drier, possibly less weathered rock, are shown as Figure 8.

Figure 6: (a) ERT profile T13 with overlaid MASW5 and borehole (BH1). MASW5 tied T13 at the 61m mark; BH1 ties T13 at the 100m mark; (b) MASW5 1-D shear wave velocity profile. MASW depth to top of weathered rock (“acoustic” top of rock) is 20.5ft (6.2m). Red color line on Figure 6b indicates interpreted depth to top of rock.
Figure 7: Correlation of the interpretation of ERT and MASW. The acquisition parameters: Offset, geophone spacing and alignment used are as follows in MASW1, 10ft (3m), 2.5ft (0.76m), N-S; in MASW4, 10ft (3m), 5ft (1.52m), E-W, and in MASW6, 10ft (3m), 5ft (1.52m), N-S.
Figure 8: 2D-ERT profiles (T7, T8, T9 and T10) with interpreted top of moist weathered rock and top of drier, possibly less weathered rock. The labels given for the white and black lines in T7 are the same for T8, T9 and T10.
3.5 Prominent joint set

A linear, north-south oriented zone of relatively low resistivity that extends through Sinkhole_1 is readily identified on Figure 9. This linear feature (labeled as joint set 1 in Figure 9) could be a zone of more intense fracturing (i.e. north-south trending joint set). Alternatively, in as much as the zone underlies a natural north-south trending surface drainage pathway, this zone of low resistivity could simply be the result of moisture with piped fines percolating into the subsurface over an extended period of time presumably with some attendant solution-widening.

![Figure 9: Parallel alignment of 16 W-E oriented 2D-ERT profiles with an approximate location of surface expression of Sinkhole_1 (in red). Each ERT profile has a length of 835ft (254.5m). Horizontal scale and vertical scale are not the same.](image)

In addition to the prominent north-south oriented linear feature (joint set 1), other roughly linear trends of low resistivity anomalies with different orientations are also observed in Figure 9. These anomalies are not visually prominent enough and also not well defined linear features to be interpreted...
as joint sets. However, their general linear trend gives an insight into the possibility of presence of other joint sets or lineaments.

3.6 Geological model of subsurface structure of the sinkhole

The interpreted 2D-resistivity profiles (T7, T8, and T9) together with information from the borehole log were used to generate geological models depicting the subsurface structure of Sinkhole_1 (Figure 10). Sinkholes develop as a result of interrelated processes, including bedrock dissolution, rock collapse, soil down-washing/piping and soil collapse. Any one or more of these processes may lead to the development of a sinkhole. The bedrock structure underneath the surface expression of Sinkhole_1 (Figure 10) does not show any sign of major rock collapse. In all the three images (Figure 10), there are zones interpreted as moist, intensely weathered rock with clay fill at various depths beneath and in proximity to the surface expression of Sinkhole_1. This implies that the major process involved in the development of the sinkhole is downward piping of fine-grained sediments, which fill existing and/or developing fractures. As mentioned in previously, the clay residuum has chert fragments and is not as cohesive as it would be expected from a more uniform clay soil. Hence, it is concluded that the sinkhole development involved predominantly a cover subsidence processes, possibly with minor localized cover collapse. From the subsurface structure and surface expression of the sinkhole, plus the nature of the overburden material, Sinkhole_1 is classified as predominantly a cover subsidence sinkhole.
Figure 10: Geological model of the subsurface structure of Sinkhole_1, reconstructed from interpreted 2D-ERT images; T7, T8, T9 and borehole control and MASW. Horizontal and vertical scale are not the same.

4 Conclusion and recommendation

Two-dimensional electrical resistivity profiles were acquired across and in proximity to a sinkhole in Greene County, Missouri. The acquired 2-D resistivity data were processed as 2-D resistivity profiles and the interpretation was supported by a complementary MASW and borehole control data.

The study shows that Sinkhole_1 developed along a natural surface water drainage pathway, possibly above a north-south oriented joint set, and is characterized by a visually prominent zone of low resistivity. The relatively low resistivity values are due to vertical water seepage and the associated piping fine grained soils primarily into preexisting fractures. This supports the principle that sinkholes develop in areas where water is ponded or temporarily retained and able to percolate into the subsurface. Thus a sinkhole can be mitigated using appropriate engineering technologies if the source of piping waters is shut-off.

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