Palm Oil Fuel Ash as A Cement Replacement in Concrete

Jonida Pone, Ahmed Ash*, John Kamau and Fraser Hyndman

Department of Civil Engineering Group, Leeds Beckett University, UK

Received: 1 November 01, 2018; Published: 1 November 27, 2018

*Corresponding author: Ahmed Ash, Department of Civil Engineering Group, Leeds Beckett University, UK

Abstract

To produce concrete, cement is an essential material that binds together solid bodies but also is the largest producer of carbon dioxide (CO₂) emission. Up to 10% of global CO₂ emission comes from cement production thus making the sustainability of concrete a major issue that needs addressing. The processes of producing concrete consume heavily on natural resources such as sand, gravel, water, coal and crushed rock, mining of which damages the environment. It is however possible, that energy and cost efficiency can be achieved by reducing on the amount of clinker, and in its place utilizing partial cement replacements/pozzolans that require less process heating and emit fewer levels of carbon dioxide. This study investigates the effectiveness of agro waste ash by-product Palm Oil Fuel Ash (POFA) as an alternative material to replace Portland cement (OPC). Experiments were carried out by supplementing CEM I cement by weight in concrete mixes with POFA at 2.5%, 5%, 10%, 15% and 20% steps at the point of need, with water to cement ratio of 0.5. Results were compared with a control specimen, which was made with 100% cement. The results showed impressive compressive strength, especially at early age; in fact POFA specimens containing 2.5% and 5% POFA replacement displayed greater early compressive strength in comparison to the control, which is similar in behaviour to concrete containing silica fume which is an established partial cement replacement used in high strength applications. The results showed good repeatability and highlight the potential of POFA as an effective pozzolan which could enhance the sustainability and economic aspect of concrete.

Introduction

Sustainability has widely emerged in recent years to resist climate change and pollution caused by ineffective waste management. The cement industry, as one of the fundamental materials industries, plays a very important role in the social and economic development as well as imposes a great challenge in terms of its large consumption of natural resources and energy and the emission of greenhouse gases. Cement is one of the main constituents in concrete and is thus one of most utilized commodities in the world [1]. From an environmental perspective, the production of 1 tonne of cement directly generates about 1 tonne of CO₂ [2]. Cement production is therefore responsible for 7-10% of the world’s total CO₂ emissions; compare this to the aviation industry, which is 2.8%, three times less than the production that comes from cement industry [3-8]. As a result, the use of supplementary cementing materials (SCMs), like pulverised fuel ash (PFA) and ground granulated blastfurnace slag (GGBS) have been established over the past 30 years as alternatives to POFA at 2.5%, 5%, 10%, 15% and 20% steps at the point of need, with water to cement ratio of 0.5. Results were compared with a control specimen, which was made with 100% cement. The results showed impressive compressive strength, especially at early age; in fact POFA specimens containing 2.5% and 5% POFA replacement displayed greater early compressive strength in comparison to the control, which is similar in behaviour to concrete containing silica fume which is an established partial cement replacement used in high strength applications. The results showed good repeatability and highlight the potential of POFA as an effective pozzolan which could enhance the sustainability and economic aspect of concrete.
replaced with POFA in concrete by volume at steps of 0%, 2.5%, 5%, 10%, 15% and 20%. The 0% replacement, also referred to as the control was taken as the point of reference from which all performance was measured. Water to cement ratio (WCR) of 0.5 was used for the mixes to achieve a good balance of workability and strength in line with Abram’s law which states that the strength of concrete mix is determined by the WCR, with lower WCR having higher strengths and vice-versa [12]. The quantities of each mix were measured as detailed in Table 1. Workability was measured using the slump test method, whose apparatus were a slump cone and a tamping rod conforming to BS EN 12350-2:2009 [13]. Cube moulds for compressive measured 100mm x 100mm x 100mm, whereas cylinder moulds for tensile strength testing were 150mm in diameter and 300mm in height, conforming to dimensional guidelines of BS EN 12390-1:2012 [14]. The method used to make cubes conformed to BS EN 12390-2:2009 [15]. Cylinders conformed to BS EN 12390-4:2000 [16]. The specimens were left on the moulds for 24 hours after which they were stripped, marked and submerged in a water tank at temperatures of 20 °C±2 until their age of testing conforming to BS EN 12390-2:2009 [15]. Specimens were cured for up to 28 days. Compressive strength tests were conducted to BS EN 12390-4:2000 [16]. After the application of an initial load of 0.6±0.2N/mm².s, which, according to BS EN 12390-4:2000 [16] does not exceed 30% of the failure load, further constant load was applied at a rate of±10% until no further load could be sustained. Compressive tests were carried out at 7 and 28 days. Results were taken as an average of the three cubes per test, and expressed in N/mm².Tensile strengths were conducted to BS EN 12390-6:2009 [17]. The testing machine conformed to BS EN 12390-4:2000 [16] while packing strips conformed to BS EN 316:2009 [18]. Initial load was applied at a constant rate of stress of 0.04 N/mm².s, which, according to BS EN 12390-6:2009 [17] does not exceed 20% of the failure load. Further constant load was thereafter applied at a rate of±10% until no further load could be sustained. As POFA is a very fine low-density material the cement was replaced by weight and at lower percentage, similar dosage to Silica Fume.

Table 1: Proportions of POFA concrete mix using CEM.

| Composition with POFA replacement | Cement (kg) | POFA (kg) | Coarse Aggregates (kg) | Fine Sand (kg) | Waters (litres) |
|-----------------------------------|-------------|-----------|-----------------------|---------------|-----------------|
| Control                           | 6.5         | 0.00      | 19.5                  | 6.5           | 3.2             |
| 2.5%                              | 6.4         | 0.08      | 19.5                  | 6.5           | 3.2             |
| 5%                                | 6.2         | 0.16      | 19.5                  | 6.5           | 3.2             |
| 10%                               | 5.9         | 0.33      | 19.5                  | 6.5           | 3.2             |
| 15%                               | 5.5         | 0.49      | 19.5                  | 6.5           | 3.2             |
| 20%                               | 5.2         | 0.65      | 19.5                  | 6.5           | 3.2             |

Results and Discussion

Compressive strength

Figure 1: Compressive strength Development Chart (CEM I and w/c 0.5).

Compressive strength is the most important property of concrete, and it measures how much load concrete structures can sustain before failing. (Table 2) and (Figure 1) show the compressive strengths at 7 and 28 days of hardened concrete with 0%, 2.5%, 5%, 10%, 15% and 20% POFA replacement. Replacements of up to 10% achieved strengths that were above the targeted class C32/40 at 28 days, is among strength classes listed by BS EN 1992-1-1: 2004 [19] and BS 8500-1:2015 [20], as being suitable for structural applications. It is possible to predict the higher replacements could achieve strengths that are far above this class strength at 91 days or longer, due to the fact that pozzolanic concrete continue to gain strength up to and beyond 91 days. Palm Oil Fuel Ash as A Cement Replacement in Concrete. Mod App Matri Sci 1(1): 2018. MAMS.MSID.000102. DOI: 10.32474/MAMS.2018.01.000102.
Oil Fuel Ash can also be used as an alternative to cement in highway pavement as the minimum 28-day compressive cube strength requirement using pozzolans for highway pavements in the UK is 9.6N/mm². Therefore 2.5%, 5% and 10% replacement of POFA is sufficient to use in highway pavement and road construction. It is worth noting, as POFA is a very fine and low-density material, a 10% partial cement replacement constitutes a large volume of POFA, similar to silica fume. Like similar fume [21] at low percentage cement substitution of up to 5% POFA yields greater early age strengths in comparison to the control. In limited work reported elsewhere, other researchers [7,9,11] found the optimum level of replacement to be at 15%-20% which are different results from this research. The findings suggest that Palm Oil Fuel Ash is an effective pozzolan to replace cement at low percentages. The 2.5% and 5% POFA replacements had remarkable high early strength concrete compare to the control one. According to [22,23], the early age strength is due to the hydration of cement, with POFA acting as a filler of voids and contributing to the strength gain, while the latter age strength in pozzolanic concrete is associated with the reaction of SiO₂ present in the POFA with free lime Ca(OH)₂ from the hydration of cement in a secondary reaction over time, to form calcium silicate hydrate (C-S-H). The optimum replacing level of cement by POFA is at 2.5%. This amount of replacement could be effective in large projects where large amounts of OPC are used and an early high compressive strength is required. The replacement of OPC with POFA helps in decreasing the pollution as well as has many economic benefits. Palm Oil Fuel Ash particle size can be compared to that of silica fume and therefore could have similar applications to silica fume especially in terms of mix design. It can be used in high-strength concrete and could include long span bridges, mainly of precast and prestressed girders to allow for longer span in structural bridge design and high-rise sky scrapers by building smaller columns and increasing usable space.

### Table 2: Compressive strength for POFA using (CEM I and w/c 0.5).

| Compressive Strength (N/mm²) | 7 days | 28 days |
|-----------------------------|--------|---------|
| Control                     | 35     | 41      |
| 2.5%                        | 44     | 52      |
| 5%                          | 42     | 48      |
| 10%                         | 36     | 40      |
| 15%                         | 29     | 36      |
| 20%                         | 26     | 31      |

### Tensile strength

(Table 3) and (Figure 2) show the tensile strength results for POFA concrete at 28 days, the tensile strength initially increases with increasing POFA replacement up to 5% then subsequently decreases at higher levels; this trend is not consistent with the general behavior of PCRs [22-26] whereby the tensile strength decreases with increasing amount of PCRs.

### Table 3: Tensile strength for POFA using (CEM I and w/c 0.5).

| Tensile strength (N/mm²) | 28 days |
|--------------------------|---------|
| Control                  | 3.3     |
| 2.5%                     | 4.1     |
| 5%                       | 3.8     |
| 10%                      | 3.2     |
| 15%                      | 2.8     |
| 20%                      | 2.5     |

### Workability

Table 4 and Figure 3 show the slumps of POFA replaced mixes at different replacement levels. One of the basic attributes of any cementitious material is its workability or “consistence”, which is largely determined by how wet the concrete is. This is referred to as “slump”. Basically, the wetter the concrete, the higher the slump. Although slump is often seen as an indication of water content, it is more reasonably interpreted as a measure of consistence.
Consistency is a term that describes the state that concrete is when it is delivered on site, how easily can fresh concrete flow. Concrete is said to be workable when it is easily placed and compacted homogenously, however workability is very difficult to assess. Slump test is the most well-known method to examine the characteristics of concrete workability. It is used to measure the consistency of concrete as well. The slump test values depend on a variety of factors such as types and properties of concrete ingredients. Workability of POFA concrete was observed to decrease with increasing replacement; this is due to the high-water demand of POFA which can be countered by the use of admixtures. Like with silica fume [21], the water demand of concrete containing POFA increases with increasing amounts of POFA. This increase is caused primarily by the high surface area of the palm oil fuel ash.

Table 4: Slump readings for POFA mixes.

| Mix using CEM I and w/c 0.5 | Slump values (mm) | Slump Class |
|----------------------------|-------------------|-------------|
| Control                    | 150               | S3          |
| 2.5%                       | 110               | S3          |
| 5%                         | 100               | S3          |
| 10%                        | 90                | S3          |
| 15%                        | 35                | S1          |
| 20%                        | 15                | S1          |

Figure 3: Workability of POFA replaced mixes.

Conclusion

The usage of partial cement replacements or pozzolans is gaining popularity for a variety of reasons including enhancing concrete performance, reducing the cost of using traditional concrete ingredients and serve the environment. Cement production is one of the highest contributors to CO$_2$ emission. Partial cement replacements not only confer environmental benefits, but they also have a positive impact on concrete properties. This study investigated the effect of Palm Oil Fuel Ash as a pozzolan replacement in concrete and the main findings are:

a) Palm Oil Fuel Ash is an effective pozzolan to replace cement at low percentages with the optimum level is at 2.5%

b) Specimens made with 2.5% and 5% POFA replacement had higher strengths compared to the control at 7 and 28 days; very similar behaviour to silica fume

c) Workability decreases with increased amount of POFA unlike with PFA and GGBS concrete; like silica fume POFA has a high-water demand.

d) Based on strength findings, palm oil fuel ash concrete have the potential to be used in superstructures including long span bridges, mainly for precast and prestressed girders to allow for longer spans in structural bridge design and high-rise sky scrapers by building smaller columns and in creasing.

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Citation: Jonida P, Ahmed A, John K, Fraser H. Palm Oil Fuel Ash as A Cement Replacement in Concrete. Modern Approaches on Material Science 1(1): 2018. MAMS.MS.ID.000102. DOI: 10.32474/MAMS.2018.01.000102

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DOI: 10.32474/MAMS.2018.01.000102