Modelling Morphological Changes and Migration of Large Sand Waves in a Very Energetic Tidal Environment: Banks Strait, Australia

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Abstract: Banks Strait, Tasmania, Australia, has been identified as a potential site for the deployment of tidal turbines. In this study, the characterisation of sediment transport and large sand waves for this site is performed. Observations of bed level change collected from surveys in 2018 showed a migration of large sand waves over a period of nine months. Migration rates in an excess of one hundred meters for nine months were found, which are large compared to the rate reported at other coastal sites, by several meters per year. A validated hydrodynamic model is coupled with a morphodynamic model to perform sensitivity tests and identify what parameters influence migration to better understand sediment dynamic in the Banks Strait. Numerical analysis showed a constant shift of the sand waves profile in an eastward direction, consistent with the observations. This migration was strongly linked with tidal asymmetry, with a residual current flowing towards the east. The principal parameters driving the migration of sand waves in the Banks Strait were found to be sediment sorting, bed friction and residual current. This study gives new insights for the seabed of Banks Strait and provides an assessment of the natural variability of sediment for future tidal farms deployments.

Keywords: sediment transport; sand waves migration; tidal energy; numerical modelling

1. Introduction

Increasing activity in ocean renewables and blue economy is driving the need to understand natural sediment processes and their variability and also to understand the potential influence of anthropogenic activities on them. In coastal environments, sediment plays an important role in the functioning and structuring of coastal ecosystems and is a crucial component for tidal site assessment [1,2]. In shallow straits with sandy bed formations, tidal motion influences the bedform, and rhythmic, large-scale bedforms are commonly present. Tidal sand waves are the most dynamic bedform and very large sand waves were found recently in the Banks Strait, a promising site for future deployment of tidal farm, connecting the Bass Strait to the Tasman Sea (Figure 1). Such large sand waves of more than five meters heights and wavelengths of more than 100 m have also been found in other areas, such as the Taiwan banks, San Francisco bay, North Sea, and in more energetic sites, such as the Torres Strait (Australia), in the Alderney Race (English channel) and are listed alongside other observed sand waves in Table 1. Sediment dynamics and migration of sand waves alter the profile of the seabed and may pose risks to the tidal energy industry, exposing cables and affecting the stability of offshore structures. It is essential to investigate the mechanisms responsible for sediment dynamics, as part of a global environmental impact assessment of large arrays installations, to better understand the influence induced by human activities. Moreover, regulators want to know how tidal farms might affect the marine environment, thus an assessment of the natural variability of...
the sediments must be carried out at prospective sites. Given the scarcity of in situ data, improving our understanding of the complex processes of sediment transport and sand waves migrations is one of the challenges in high energetic sites.

Figure 1. Localisation of Banks Strait, eastern part of Bass Strait, with the morphology of Banks Strait in 1969 from Slater in the zoom [3].

Table 1. Characteristics of sand waves from observations.

| Environment          | Source          | Location               | Wave Length (m) | Wave Height (m) | Migration Rate (m/Year) | Max Tidal Velocity (m/s) |
|----------------------|-----------------|------------------------|-----------------|-------------------|-------------------------|--------------------------|
| Coastal regions      | Besio [4]       | North Sea              | 120–500         | 2–10              | 1–8                     | /                        |
| Coastal regions      | Tonnin [5]      | North Sea              | 250–370         | 1.3–4             | /                       |                          |
| Coastal regions      | Van Dijck [6]   | North Sea              | 100–800         | 1–10              | 0–40                    | /                        |
| Coastal regions      | Knaapen [7]     | North Sea              | 110–340         | 0.7–3.4           | 0–8.4                   | /                        |
| Coastal regions      | Roos [8]        | Southern North Sea     | 145–760         | 1.5–7.3           | /                       |                          |
| Coastal regions      | Bellec [9]      | Western Barrents Sea   | 300–700         | 4–19              | /                       | 1                        |
| Coastal regions      | Hoozemans [10]  | Dutch Sea              | /               | /                 | up to 200               | /                        |
| Strait               | Daniell [11]    | Torres Strait (AUS)    | ~62             | 5–10              | 15–48 m over 7 month    | 2                        |
| Strait               | Katoh [12]      | Bisanseto Sea (JAP)    | 80–180          | 2–6               | up to 20                | /                        |
| Strait               | Malikides [13]  | Bass strait (AUS)      | 55–1730         | 2–12              | /                       | 1                        |
| Strait               | Zhou [14]       | Taiwan Banks           | 100–2000        | 1.5–15            | 1–5                     | 1                        |
| Strait               | Blunden [15]    | Alderney Race (UK–FR)  | /               | 5–10              | 0–70 m for 50days       | /                        |
| Strait (not shallow) | Santoro [16]    | Messina strait (ITA)   | 50–150          | 0.5–6             | /                       | 1.5                      |
| Bay                  | Barnard [17]    | San Francisco (USA)    | 30–220          | 4–10              | 7                       | 2–2.5                    |
| Tidal Bay            | Dalrymple [18]  | Bay of Fundy (CAN)     | 10–215          | 0.15–3.4          | /                       | 0.5–2                    |
| Tidal inlet          | Bujsman [19]    | North Sea              | 125–250         | 1–7               | 0–90                    | /                        |
| Tidal inlet          | Bartholdy [20]  | Danish Wadden Sea      | 50–250          | 1.3–3.6           | average: 32             | 1.5–1.25                 |

Few studies on sediment dynamics and seabed characterisation exist for the south of Bass Strait. Only earlier studies, mostly conducted in the 1980–1990s, are available [3,13,21–27]. More recently, two surveys by Geoscience Australia (GA) took place in this area (GA 226/233) [28,29]. Of these studies, only one study has focused on the Banks Strait [3], which describes the geology of this channel as cold-water carbonates, and is a valuable source of sediment grabs information in this region (Figure 1). Information on the Banks
Strait can be found from larger studies of the Bass strait, such as [24] who proposed a map of the sediment types for Bass Strait, including the Banks Strait, with results in accordance with the observations of [29], who indicates for Banks Strait a mean grain size between 0.5 and 2 mm (coarse sand). Sand waves in the Banks strait area were first reported by Slater in 1969 [3] (Figure 1) and then by Malikides in 1988 [13] based on the work of [3] but were not studied. During these studies, only sand waves around Flinders Island were investigated with sediment sampling and side-scan sonar [13,23]. Since then, no other seafloor features of Banks Strait has been investigated, due to limited data and harsh conditions in this energetic site.

Observations and numerical modelling are the keys to understanding the sediment and sand wave dynamics. However, full-scale bathymetry survey are often limited, as surveys are expensive and challenging in highly energetic sites with current speeds above 2 m/s. The initial formation of sand waves has been extensively studied with linear and nonlinear stability analysis [4,30–36]. From these studies, the migration of sand waves was found to occur as the result of the residual current and tide asymmetry. More recently, more complex numerical shallow models were used to investigate the formation, migration and also the effect of parameters (tidal forcing, significant wave height, turbulence scheme, sediment size) on the evolution of sand waves: Tonnon [5] with the study of artificial sand waves, Borsje [37] who studied two turbulence models associated with bed load transport only, Van Gerwen [38] who focussed on formation of sand waves with comparable migration patterns to observations, and Wang [39] who examined the sensitivity of sand waves to environmental factors. All of them found that the variation of the shape and evolution of the sand waves was linked to sediment grain sizes, tidal characteristics, and residual currents. So far, all of these studies considered medium sand in less energetic areas than the Banks Strait and did not include models with waves. According to King [40] and Fairley [41], waves could heavily influence sediment dynamic in energetic tidal environments. The presence of waves increases turbulence at the seabed, leading to an increase in the bed shear stress and therefore, a greater potential for sediment movement. In the Banks Strait, significant wave–tide interactions were found by [42,43]; thus, it is crucial to consider wave forcing for the sand transport in this tidal energetic site.

In this study, sediment observations and a high resolution sand transport model coupled with the hydrodynamic model with waves forcing (the shallow water model MIKE 21FM [44]) were combined to model and, therefore, enhance our knowledge of the sediment transport processes and the mechanisms driving the migration of sand waves in the Banks Strait. Numerical sensitivity experiments were performed to investigate the response of sand waves to environmental parameters and compared with the survey data set (including data from multi-beam surveys and sub bottom profiling). Parameters driving most of the sand wave morphology were identified in this high energetic site and compared with the literature. Section 2 presents the description of the model with the set up of hydrodynamics, sediment parameters and the surveys performed in 2018, followed by the validation of the hydrodynamic model. Finally the results for the sensitivity analysis are presented and discussed in Section 3, and the conclusion in Section 4.

2. Morphological Set Up

This section first describes the surveys performed by the AUSTEn project in the Banks Strait, whose main goal was to assess Australia’s tidal energy resource through several field surveys to highlight the opportunities available for the emerging tidal industry, the scientific community and the Australian Renewable Energy Agency (ARENA) [45]. Second, the model set-up for the hydrodynamics and sediment transport processes are presented as well as the method for the sand waves analysis. Then, the validation of the hydrodynamic model against in situ data from Acoustic Doppler Current Profilers (ADCP) deployed during the campaigns is described.
2.1. Survey of Banks Strait

The Banks Strait is a shallow channel located in the north east of Tasmania between Cape Portland and Clarke Island, with the tidal constituent $M_2$ as the dominant. Multiple data sets, including hydrodynamic (current speed, waves) and sediment data, were collected by the AUSTEn project [45] to develop and model the Banks Strait area during four campaigns: March, July and December 2018 and February 2019 [43,46–48]. During this period, significant wave height ($H_s$) were measured and varied between 1 and 2 m (maximum up to 5 m) with wave period between 7 and 12 s [48]. High current speed (>2 m/s) suitable for implementation of tidal farm were found near the area of the sand waves which were also found by numerical modelling of the area [43,47,49]. It should be noted that two ADCP frames were buried under at least 1 m of sand at the location of sand waves and could not be retrieved. Seabed characterisation performed during the AUSTEn project included the following: multi-beam transects to collect accurate bathymetry of the site; sub-bottom profiling for high-resolution imaging of sub-seabed sediments; penetrometer casts to describe the hardness of surficial seafloor sediments as well as the collection of sediment samples to determine sediment size and type with bottom grabs and sediment traps.

2.1.1. Multi-Beam/Bottom Grab/Sediment Traps

Bathymetry was measured with the Bathyswath interferometric bathymetry system (234 kHz unit from ITER Systems). The bathymetry survey covered an area of approximately 210 km$^2$ with depths ranging from 10 m to depths exceeding 90 m (Figure 2). To complete the sediment data set, bottom grabs and sediment trap samplings were performed using a Van Veen Grab sampler [50] and sediment sampling traps (Table A1 in Appendix A). Twelve bottom grabs were conducted during the December campaign: eight were successful, and four were rejected from analysis, given that the grabs trapped nothing indicating a rocky area. Sediments traps were put on six ADCP frames during the campaign in July and December 2018 and retrieved in December 2018 and February 2019 (Figure 2). All sediment samples were emptied into a plastic container after retrieval and stored in a refrigerator. For the grain size analysis, 100 g to 200 g of each sample were oven dried at 60 degrees Celsius for 24 h. The samples were analysed using a Sympatec QICPIC particle size analyser.

The Wentworth scale [51] was used to classify the sediment size from the mean diameter with the results shown on Figure 2. The sediment size for twelve locations were obtained; for different samples at the same location, the mean of grain size was taken (Table A2 in Appendix A). Sand was a dominant fraction in all samples, consisting mainly of coarse and very coarse sand (Figure 3a). The only exceptions were samples at the south of Swan Island where medium sand was found. The results for the sorting following the classification of [52] revealed that one sample consisted of well sorted (near Swan Island), eight moderately sorted and three poorly sorted sediment (concentrated in the middle of the strait). Mixed sediment was classified according to the modified Folk diagram [53], which relates the proportions of mud, sand and gravel by the ratio of sand to mud and the percentage of gravel (Table A3 in Appendix A). This analysis confirmed sand was the dominant fraction. Data from a previous seabed survey in Banks Strait [3] were used for comparison. The comparison of three samples showed the same classification, except for grab 6 (West of the Banks Strait), where it was slightly different: sandy gravel in 2018 and gravelly sand in 1966 [54]. This can be explained by the lower quantity collected for this sample (20,000 particles analysed against a mean of 325,000 for the two other samples), which does not cover the whole spectrum of sediment in this location.
Figure 2. Distribution of grain size in Banks Strait from the AUSTEn survey with the results of the multi-beam survey. Rock areas were defined with the results of the penetrometer and the classification of grain size from bottom grabs and sediment traps.

Figure 3. (a) Average particle size distribution of the seabed sediment found in the Banks Strait during the AUSTEn surveys. (b) Deceleration curves for penetrometer profiles taken in the Banks Strait showing example for rock (North West of Clarke Island; −40.56382, 148.08664) and sandy area (East of the channel; −40.67903, 148.23241).
2.1.2. Penetrometer

In situ measurements of surficial seafloor sediment strength was determined with the BlueDrop penetrometer described by [55,56]. The characteristics of the impact signature were analysed to obtain the kind of substrate—soft, medium, and hard substrate—based on the deceleration curves after impact. Hard substrate is indicated by the deceleration to zero velocity in a shorter period of time, than into a soft ground (Figure 3b). Each penetrometer drop included three casts from which the mean penetration depth (distance between free fall and complete stop after impact with seafloor) was taken. The majority of penetration depths range between 0.01 and 0.04 m, which is typical for bedrock dominated area. Five drops had penetration depths superior to 0.1 m characteristic of sandy area and were located South of the Clarke Island, towards Swan Island and at the east of the channel.

2.1.3. Sand Waves Area and Methods for Analysis

The multi-beam survey found large sand waves South of Clarke Island between South Head and Moriarty Point (Figure 4). These sand waves had their crest almost aligned perpendicular to the dominant tidal currents, with their key characteristics: wavelength, wave height and asymmetry (Figure 5) defined in Table 2.

![Figure 4. The location of the transect of sand waves in Banks Strait with a zoom of bathymetry from the multi-beam survey for this area. Bottom: Profile AA’ for the multi-beam survey (MB).](image)

Crest and troughs were detected as local maxima and minima [7,57]. In this study, the wavelength was defined as the true length between two adjacent trough points, whereas
wave height was defined as the difference between the crest level and the baseline of the two trough levels. (Equation (1)) and sand wave asymmetry were obtained with Equation (2) \[58,59\]:

$$H = z_{	ext{crest}} - \frac{z_s L_l + z_l L_s}{L}$$  \hspace{1cm} (1)

$$A = \frac{L_l - L_s}{L} = \frac{|x_l - x_{	ext{crest}}| - |x_{	ext{crest}} - x_s|}{L}$$  \hspace{1cm} (2)

with \(L\) as the wave length, \(L_l\) the length of lee side and \(L_s\) as the length of the stoss side (for symmetrical sand waves \(A = 0\)).

![Figure 5. Definition of sand wave characteristics.](image)

The height of the sand waves was between 10 and 36% of the water height, which is the range generally observed \[60\]. Three of the sand waves had heights larger than 5 m, corresponding to very large sand waves \[61\]. Most of the sand waves had asymmetry profiles with a steeper slope on the east side.

| Sand Waves | Wavelength (m) | Wave Height (m) | Asymmetry |
|------------|----------------|----------------|-----------|
| SW1        | 902            | 13.4           | -0.58     |
| SW2        | 281            | 5.6            | 0.08      |
| SW3        | 194            | 6.6            | 0.46      |
| SW4        | 289            | 3.6            | 0.67      |

To further characterise the survey area, transects were undertaken in December 2018, using a sub-bottom (SB) profiling system. Georeferenced, sub-bottom data were acquired, using an acoustic profiling system (StrataBox 3510 HD, Syqwest Inc., Cranston, RI, USA) operating at 3.5 kHz. Transects were focused regions previously mapped for bathymetry and possible sandy sediment. Most of the sub-bottom data determined that the seafloor in the Banks Strait is largely homogenous rock or reef. However, the region to the South-East of the Clarke Island had a significant region of sand coverage on the seabed with the sand waves identified during the bathymetry survey. To compare the total load transport observed with the one from the numerical model, residual bed load sediment transport rates were calculated for the crest from the two data sets (multi-beam and sub bottom) over the 9 month period with the following:

$$q_{b,obs} = 0.5HV_{mig}$$  \hspace{1cm} (3)

where \(H\) is the height of sand waves and \(V_{mig}\) is the migration speed \[52\].
Using the Soulsby-Van Rijn equations below for total load transport by waves plus current [52], the residual total load transport was estimated for the observations with the relation \( \frac{A_{ss}}{A_{sb}} = 3.63 \), with \( A_{sb} \) equal to \( q_{b,obs} \) (term \( A_{ss} \) gave the suspended load transport). A factor of 2 was added in this relation, given that [52] indicated that bedform migration could underestimate the bedload transport by this factor. Then, this relation was reinjected into Equations (4) and (5) to obtain the total load rates:

\[
q_t = \bar{A}_s \bar{U} \left[ (\bar{U}^2 + \frac{0.018}{C_D} U_{rms}^2)^{1/2} - \bar{U}_{cr} \right]^{2.4} \left( 1 - 1.6 \tan \beta \right) \tag{4}
\]

with \( \bar{U} \) equal to the depth-averaged current velocity, \( U_{rms} \) the root-mean square wave orbital velocity, \( \bar{U}_{cr} \) the threshold current velocity, \( C_D \) the drag coefficient due to current alone and \( \beta \) the slope of bed in streamwise direction.

where

\[
A_s = A_{sb} + A_{ss} \tag{5}
\]

\[
A_{sb} = \frac{0.005h(d_{50}/h)^{1.2}}{[(s-1)g]^{1.2}} \tag{6}
\]

\[
A_{ss} = \frac{0.012d_{50}D_4^{-0.6}}{[(s-1)g]^{1.2}} \tag{7}
\]

\[
D_4 = \left[ \frac{g(s-1)}{\nu^2} \right]^{1/3} d_{50} \tag{8}
\]

\[
C_D = \left[ \frac{0.40}{\ln(h/z_0) - 1} \right]^2 \tag{9}
\]

where \( g \) is the acceleration due to gravity, \( h \) is the water depth, \( \nu \) is the kinematic viscosity of water, \( s \) the relative density of sediment and \( z_0 \) the bed roughness length.

\[
\bar{U}_{cr} = 8.5(d_{50})^{0.6} \log_{10} \left( \frac{d_{50}}{d_{90}} \right) \quad \text{for} \quad 0.5 \leq d_{50} \leq 2 \text{ mm} \tag{10}
\]

\( U_{rms} \) can be calculated from the parameters \( H_s \) (significant wave height), \( T_z \) (zero-crossing period of waves), and \( T_n \) (scaling period for waves) with the help of a curve derived from the JONSWAP spectrum [52].

\[
T_n = \left( \frac{h}{g^{1/2}} \right) \tag{11}
\]

\[
T_z = 11 \left( \frac{H_s}{g} \right)^{1/2} \tag{12}
\]

The results are described in Section 3.1 and compared to the numerical results in Section 3.2.

2.2. Numerical Model

MIKE21 FM was used to model hydrodynamics and sediment transport in the area of the Banks Strait [44,62]. To model sediment transport, the module quasi three-dimensional “Sand Transport” (STPQ3D) was used, which determines the transport of non-cohesive particles (a common feature in high energy sites) based on both the hydrodynamic conditions (current and waves) and sediment properties [63]. The integrated momentum approach of [64] was used for the time evolution of the boundary layer, due to the combined wave-current motion. The mean bed shear stress was calculated from the turbulent boundary layer model and an additional constant velocity, found by iteration, was added to the wave orbital motion. Under the action of the wave and current, sediment transport rates were derived by linear interpolation of a sediment transport table using inputs from the
hydrodynamic simulation. The bed load transport was calculated from the instantaneous Shields parameter using the Engelund and Fredsøe formula, with the suspended transport calculated as the product of the instantaneous flow velocities and sediment concentration [64,65]. When generating the sediment transport table, the model calculated bed and suspended load separately but saved only the total load to speed up the simulations; therefore, the description of sediment transport in this case was in equilibrium conditions. For the morphodynamic evolution of the bed, the Exner sediment continuity equation was solved at each timestep using the following:

\[
- (1 - n) \frac{\partial z}{\partial t} = \frac{\partial S_x}{\partial x} + \frac{\partial S_y}{\partial y} - \Delta S
\]  

where \( n \) is the bed porosity, \( z \) is the bed level, \( t \) is the time, \( S_x \) is the total load in the \( x \) direction, \( S_y \) is the total load in the \( y \) direction and \( \Delta S \) is the sediment sink or source rate.

The numerical domain extended between 38° S and 44° S and 141° E and 150° E based on [47]. Near the model boundaries, mesh sizing of 3 km was used with resolution increasing at the approach of the Banks Strait with a sizing of 100 × 10 m for the area around the sand waves near Clarke Island. Sensitivity tests were performed for mesh size at the location of the sand waves, with a density of 10 m found to be the best compromise (time cost/accuracy) to represent the crests and troughs in the sand waves area as shown in Figure 6. The model data shoreline, bathymetry, wind data and open boundaries are detailed in Table 3. Data sets from the Centre for Australian Weather and Climate Research (CAWCR) were used for the wave components with a spatial resolution of 4 km around Australian coastlines [66]. These data sets were used as a forcing in the sand transport module.

![Bathymetry Sand waves 16/03/2018](image)

**Figure 6.** Difference of bathymetry between the profile extracted from multi-beam data and from the model.

| Vertical discretisation | 2D |
|------------------------|----|
| Shoreline              | Geoscience Australia (GA)/Australian Hydrography Service(AHS) [67,68] |
| Bathymetry             | GA/AHS/AUSTEn Project [45,67,68] |
| Tidal constituents     | \( S_1, K_1, O_1, P_1, Q_1, M_2, S_2, N_2, K_2 \) and \( M_4 \) [69] |
| Wind/Sea level Pressure| ERA 5 [70] |
| Waves                  | Centre for Australian Weather and Climate Research (CAWCR) using Wawewatch III [66] |
| Validation             | 5 ADCP Measurements [47] |
The models were run for the period of 16 March 2018 to 7 December 2018, with a one week spin up to match the data collected for sand waves migration (one survey in March 2018 with the multi-beam and one survey in the same area with transect conducted with the sub-bottom profiler in December 2018). A Barotropic model was used, given that the water column is well mixed and uniform in this energetic environment in the area [71]. For all simulations, the Manning value was set to 32 given the results of the sensitivity study of [47], representing a coarse sand habitat [52]. Sediment data were specified at a constant d50 of 0.8 mm (coarse sand) for the reference case. A sediment transport table was created based on the hydrodynamic characteristics of Banks Strait (Table 4) and served as a reference for the sensitivity tests. For the bed layer, an infinite supply of sediment was assumed as in [72].

Table 4. Specifications of parameters for the sediment transport table.

| Parameters                        | First Value | Value Max | No of Points |
|----------------------------------|-------------|-----------|--------------|
| Current speed                    | 0.1         | 4.1       | 20           |
| Wave height                      | 0.1         | 12.6      | 25           |
| Wave period                      | 1           | 29        | 14           |
| Wave height/Water depth          | 0.05        | 20        | 20           |
| Angle current/waves              | 0           | 360       | 12           |
| Grain size                       | 0.8         | 0.8       | 1            |
| Sediment grading                 | 1.1         | 1.1       | 1            |

Feedback between the evolving bed and the hydrodynamic model was included in the simulations to ensure dynamic interaction of the morphodynamic system. The bed evolution can be accelerated at each time step by means of a morphological speed up factor (SUF). A simulation of 1 month with a SUF of 10 represents 10 months. The reference scenario was compared to a simulation with a SUF of 20. Given that the difference was negligible (less than 0.01%) for the profile AA' of the sand waves, this factor was applied to all sensitivity tests to optimize the computational time of 12 days to 20 h; all the tests are listed in Table 5.

Table 5. List of sensitivity cases (SC).

| Reference case | d50 | Layer Thickness | Sorting | Manning | ST Formula | Waves Forcing | Tidal Current | Residual Current |
|----------------|-----|-----------------|---------|---------|------------|---------------|---------------|------------------|
| Reference case | 0.8 | Infinite        | 1.1     | 32      | cf. STQP3  | Hs            | Tides         | Tidal            |
| SC1            | 0.4 |                 |         |         |            |               |               |                  |
| SC2            | 1.2 |                 |         |         |            |               |               |                  |
| SC3            |     | Varying         |         |         |            |               |               |                  |
| SC4            |     |                 |         |         |            |               |               |                  |
| SC5            |     |                 |         |         |            |               |               |                  |
| SC6            |     |                 |         |         |            |               |               |                  |
| SC7            |     |                 |         |         |            |               |               |                  |
| SC8            |     |                 |         |         |            |               |               |                  |
| SC9            |     |                 |         |         |            |               |               |                  |
| SC10           |     |                 |         |         |            |               |               |                  |
| SC11           |     |                 |         |         |            |               |               |                  |
| SC12           |     |                 |         |         |            |               |               |                  |
| SC13           |     |                 |         |         |            |               | Tides/2        |                  |
| SC14           |     |                 |         |         |            |               | Tides × 2      |                  |
| SC15           |     |                 |         |         |            | Hs × 2        | Tides/2        |                  |
| SC16           |     |                 |         |         |            | Hs/2          | Tides/2        |                  |
| SC17           |     |                 |         |         |            | u + 0.005 m/s | u + 0.1 m/s    |                  |
| SC18           |     |                 |         |         |            |               |               |                  |
2.3. Hydrodynamic Model

2.3.1. Validation of the Hydrodynamic Model

The model was calibrated and validated in a previous study with tidal forcing only [47] against five ADCP measurements, for 35 days and then for a whole year to assess the model for seasonal variations. Given the inclusion of waves in the model, as well as the slight alteration to the geographical extent and finer mesh in the key area, three statistics metrics (standard deviation, correlation coefficient and centered root mean square) were recalculated for depth-averaged velocities (DAV) to check the accuracy of the new domain (called R4 ND). Results showed good agreement with the field measurements and were slightly higher than the previously developed model (Tables 6 and 7).

A comparison between the model with waves forcing and the observations for the significant wave height (average height of the highest one-third of the waves) was performed, a slight time shift can be noticed, but overall the general pattern was reproduced. However, the model did not capture all the peaks which may lead to a difference in the sediment dynamic results (Figure 7).

![Figure 7. Timeseries of significant wave height for the Acoustic Doppler Current Profiler CW1 (148.05684; -40.5294) and the numerical model for the period 14 July 2018–2 September 2018.](image)

| Stats        | R4 [47] (Pure Current) | R4 ND (Waves + Current) |
|--------------|------------------------|-------------------------|
| STD (0.403)  | 0.409                  | 0.406                   |
| R            | 0.929                  | 0.944                   |
| RMSE         | 0.153                  | 0.135                   |
Table 7. Comparison of Statistics metrics (STD: m/s, R: unit-free, RMSE: m/s) between numerical models and field measurements for autumn (22 March 2018 to 9 July 2018 for 3 ADCP) and winter (13 July 2018 to 6 September 2018 for 2 ADCP).

|          | Autumn |          |          | Winter |          |          |
|----------|--------|----------|----------|--------|----------|----------|
|          | STD    | R        | RMSE     | STD    | R        | RMSE     |
| R4 [47]  | (Pure current) | 0.384 | 0.922 | 0.149 | 0.302 | 0.861 | 0.155 |
| R4 ND    | (Waves + Current) | 0.38 | 0.943 | 0.127 | 0.299 | 0.908 | 0.126 |

2.3.2. Estimation of the Residual current

The tidal residual current can be of significance for the movements of sediments and thus the migration of sand waves [73]. Residual currents were calculated from the ADCP data (Table A4) and extracted from the simulations (Table 8). Results showed good correlation in the channel for the direction and the values were in the same range but weaker for most of the ADCP stations in the model. The observed residual currents account for the asymmetry of the tides as well as the residual large-scale circulation characterized by the sub-Antarctic surface water (SASW) or the East Australian Current (EAC) outside of the channel [74]. However, the simulations only accounted for tidal residual current, as no large-scale ocean circulation currents were applied at the boundaries. The analysis of residual current vectors from the simulation around the area of the sand waves indicated the presence of two eddies between Clarke Island and the sand waves (Figure 8). The residual direction was in the ebb direction towards the east, in the direction of the migration of the sand waves. The value of residual current in the area of the sand waves were slightly higher to those observed in the literature, such as in the North Sea, where Borsje [37] and Van Gerwen [38] used in their model a typical value of 0.05 m/s for residual current. According to [75], 10 percent of the observed residuals are in the range 0.15–0.25 m/s; the Banks Strait is, therefore, one of the few places with a high residual current.

Table 8. Comparison of residual current between ADCP (location in Table A4) and numerical model.

| Period of Deployment | Stations | ADCP Residual (m/s) | ADCP Direction (Degree) | ADCP Cardinal Direction | Model Residual (m/s) | Model Direction (Degree) | Model Cardinal Direction |
|----------------------|----------|---------------------|-------------------------|-------------------------|---------------------|--------------------------|--------------------------|
| 12 July 2018          | CW-1     | 0.063               | 246.6302                | WSW                     | 0.0626              | 265.8477                 | W                        |
| 23 March 2018         | CW-2     | 0.085               | 119.2808                | ESE                     | 0.0564              | 143.5048                 | SE                       |
| 23 March 2018         | CW-3     | 0.037               | 117.4155                | ESE                     | 0.0426              | 122.8949                 | ESE                      |
| 23 March 2018         | CW-4     | 0.0534              | 257.946                 | WSW                     | 0.0615              | 217.1545                 | SW                       |
| 13 July 2018          | CW-4b *  | 0.0434              | 264.0938                | W                       | 0.0876              | 120.6333                 | ESE                      |
| 23 March 2018         | CWTb-1   | 0.1453              | 130.2978                | SE                      | 0.101               | 142.7076                 | SE                       |
| 23 March 2018         | C-1      | 0.118               | 132.6295                | SE                      | 0.1103              | 145.2016                 | SE                       |
| 12 July 2018          | CW-2b    | 0.1082              | 156.8022                | SSE                     | 0.0657              | 155.9911                 | SSE                      |

* outside of the channel.
3. Results

First the analysis of sand waves is presented. Secondly, the reference case scenario is analysed. Then, sensitivity studies are completed to investigate which parameters influence the most migration and are discussed.

3.1. Dynamics of the Sand Waves South of Clarke Island

The comparison between sand waves in March and December showed a distinct migration pattern of the sand waves towards the east, indicative of a dynamic sediment transport regime (Figure 9). To directly measure the long term migration rates and directions of sand waves would require several bathymetric surveys with multi-beam over a period of years which is not presently available. Caution, therefore, should be taken about the evolution of the sand waves here because for December, only one transect was available in the area. Migrations rates were quantified (Table 9) using the equations described in Section 2.1.3. These results were comparable to other areas, such as tidal inlets. For example in the Marsdiep inlet (North sea) [76] found migrations rates up to 90 m/y and [20] an average of 32 m/y in the Gradyb inlet (Danish Wadden Sea). These rates were superior to rates in the coastal shelf: ref. [4] up to 8.8 m/y, ref. [6] up to 40 m/y, ref. [5] an average of 5.5 m/year and [7] up to 9 m/y. More recently, [15] found a migration of up to 70 m over a 50-day period in the Alderney South Banks, another promising tidal energy site.
3.2. Reference Scenario

The model was developed to evaluate the sediment dynamics in the Banks Strait and the reference case revealed a high value of the mean bed shear stress, the principal parameter driving sediment transport (Figure 10). For coarse sand, the critical shear stress was in the range of 0.27–1.26 (N/m$^2$) [77,78] and bed shear stress in Banks Strait exceeded this critical value. The bed level change map showed an erosion and accretion pattern in the sandy area south of Clarke Island and at the potential location of a tidal farm. The comparison of the two data sets of the AUSTEn project for the sand waves south of Clarke Island in 2018 (Figure 9) showed a migration towards the east, in accordance with the study of Slater [3] who showed that sediment transport rates in Banks Strait were in the east direction. For the reference scenario, migration in the eastward direction was also observed but with less migration than the observations (Table 10, Figure 11). Averaged values of bed shear stress in the range of 1.9 to 5.5 Pa were found along the profile AA' with maximum values between 15 and 6 Pa, characterizing the high energetic site, which explained the high migration rates. This area was located near the eddies (Figure 8), at the corner of Clarke Island with residual flow from different direction and with more frictional interaction with the seabed.

A comparison between the total load transport rates estimated from observed crests migrations (total load rates from Equation (4)) and numerical model results were performed, with the results shown in Table 11. Sediment transport rates from the model did not match those estimated from the sand wave migration with a minimum of 80% of difference for C2 (crest of SW2) corresponding to 3.08 m$^3$/day/m. Better agreement was observed by Blunden [15] in their studies of sand waves in the Alderney race, with a minimum difference of 4% (0.17 m$^3$/day/m) and a maximum of 70% difference (2.27 m$^3$/day/m), close to the minimum difference results here. Differences from 1.2 to 3 were found for the results between the model based on the equation of Engelund and Fredsøe and the Soulsby-Van Rijn equation (Equation (4)). The discrepancies between the rates calculated

![Image of Migration of Sand Waves](image-url)

**Figure 9.** Comparison of two transects (MB: multi-beam, SB: sub-bottom) for the profile AA’ (localisation on Figure 4).

**Table 9.** Migration rates for sand waves.

| Sand Waves | Migration Rate (Crest) for ~9 Months (m) | Migration Rate (Trough) for ~9 Months (m) |
|------------|-----------------------------------------|-------------------------------------------|
| SW1        | 27.4                                    | -25.4/99                                  |
| SW2        | 135                                     | 87/99                                     |
| SW3        | 21                                      | 7/87                                      |
| SW4        | -50                                     | -7/24                                     |

The model was developed to evaluate the sediment dynamics in the Banks Strait and the reference case revealed a high value of the mean bed shear stress, the principal parameter driving sediment transport (Figure 10). For coarse sand, the critical shear stress was in the range of 0.27–1.26 (N/m$^2$) [77,78] and bed shear stress in Banks Strait exceeded this critical value. The bed level change map showed an erosion and accretion pattern in the sandy area south of Clarke Island and at the potential location of a tidal farm. The comparison of the two data sets of the AUSTEn project for the sand waves south of Clarke Island in 2018 (Figure 9) showed a migration towards the east, in accordance with the study of Slater [3] who showed that sediment transport rates in Banks Strait were in the east direction. For the reference scenario, migration in the eastward direction was also observed but with less migration than the observations (Table 10, Figure 11). Averaged values of bed shear stress in the range of 1.9 to 5.5 Pa were found along the profile AA’ with maximum values between 15 and 6 Pa, characterizing the high energetic site, which explained the high migration rates. This area was located near the eddies (Figure 8), at the corner of Clarke Island with residual flow from different direction and with more frictional interaction with the seabed.

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with Equation (4) and those estimated from the migrations of the sand waves were still very high with a minimum difference of 75% for C2. Using the migration of the sand waves to obtain the bed load rates was not conclusive in this case. The challenge for the observation in December was that only a sub-bottom transect was available not a full multi-beam survey, so an error margin (<15 m) could exist for the exact position of the sand waves.

Figure 10. Mean Bed shear stress for the reference scenario for the period 22 March–7 December 2018 in the Banks Strait; **Bottom:** Magnitude of bed level change (left) and Magnitude of Total Load (right) averaged for the same period. (Profile AA’ represented by a black line, with the red star showing the possible location for a tidal farm).
Table 10. Characteristics of the sand waves modelled in the Banks Strait south of Clarke Island.

| Sand Waves | Wavelength (m) | Wave Height (m) | Asymmetry | Migration Rate (Crest) for ~9 Months (m) | Migration Rate (Trough) for ~9 Months (m) |
|------------|----------------|-----------------|-----------|------------------------------------------|------------------------------------------|
| SW1        | 907            | 12.5            | 0.52      | 83.3                                     | 59.5/59                                  |
| SW2        | 290            | 4.8             | 0.08      | 43                                       | 59/40                                    |
| SW3        | 199            | 5.1             | 0.34      | 50                                       | 40/64                                    |
| SW4        | 294            | 2.53            | 0.61      | 43                                       | 64/39                                    |

The only direction pattern not reproduced in the propagation of sand waves in the model was the migration of SW4. Observations showed an increase of the sand waves, which was not captured by the model. This could be explained by the change of directions observed in the residual current, residual sediment transport and in the orientation of the sand waves at this particular location (Figures 8 and 12). This increase in the sand wave height could also be related to a dominant bed load transport in this area. Moreover, the model did not reproduce well the peaks of large waves during storm events, which may also explain the discrepancies for the sediment transport rates. Large waves or swell resulting from storms may affect the magnitude and direction of sediment transport for a short period [73]. The region further north of the sand waves was not surveyed and unknown topographic features may also influence the magnitude of the current. Moreover, the model was not forced with large-scale ocean circulation at the boundaries, which might influence the results. The observed migrations highlighted how the large-scale circulation combined with asymmetry of the tides and waves interaction was important in this area, which may not be the case for other areas such as the Alderney race in the English Channel. Nevertheless, the model showed a migration in the correct direction for the sand waves. Sensitivity experiments were performed to further analyse the major driver of sand wave migration and to determine whether the model results could be improved.

Table 11. Estimated total load magnitude for the crests of SW1 to SW4 (profile AA').

| $Q_t$ (m$^2$/d) | C1   | C2   | C3   | C4   |
|-----------------|------|------|------|------|
| Model           | 9.08 | 3.71 | 2.92 | 2.02 |
| OBS             | 1.03 | 0.63 | 0.03 | −0.06|
| Equation (4)    | 11.4 | 2.5  | 1.46 | 0.67 |

Figure 11. Comparison of the migration of the waves between observation and model.
3.3. Sensitivity Tests: Morphology

The reference scenario was compared to sensitivity tests with varying grain size, distribution (constant or varying), sorting of sediment and differencing bed frictions.

3.3.1. Influence of the Median Grain Size (d50)

The migration of sand waves using a d50 of 0.4 mm (medium sand), 0.8 mm (coarse sand, reference scenario) and 1.2 mm (very coarse sand) with a sorting of 1.1 are shown on Figure 13. This range of values was found during the AUSTEn surveys in the Banks Strait. The sediment grain size altered the evolution of the sand waves, with differences noticed between the different classifications. Coarse and very coarse sand showed the same pattern for the evolution of the sand waves with a smaller increase in the crests for coarser sand. For finer sand, the sand wave heights were flattened, certainly due to an increase of erosion of crests and deposition away from the sand waves, resulting from the high velocity fine particles, which are easily lifted up and can be transported further. This was in accordance with the findings of [5]. For coarse and very coarse sand, the difference was negligible; the migration for medium sand was 55% slower than the coarse sand case.
3.3.2. Grain Size Distribution

An interpolation of sediment samples was performed to test whether more detailed information on sediment distribution and grain sizes in the area was necessary. For this distribution map of sediment, 1085 samples of seabed sediment (AUSTEn project and Marine Sediments Database [45,54]) were used to interpolate median grain size with the Inverse Distance Weighting method (Figure A1 in Appendix A). The varying grain size model resulted in less migration towards the east than the reference scenario, with 76% less migration for the sand wave SW2 than the reference case (Figure 13).

![Migrations Sand Waves: Influence of grain size](image)

**Figure 13.** (Top): Comparison of the migration of the sand waves between three grain sizes (d50) (SC1/SC2), (Bottom): Comparison of the migration of the sand waves between a constant grain size and a variable distribution of grain size (SC3) based on interpolation of the MARS database and the recent AUSTEn data set [45,54].

3.3.3. Layer of Thickness

The SB data showed that the seafloor in Banks Strait is largely homogeneous rock or reef, except in the area of sand waves. However, the layer thickness of sediments above bedrock in the Banks Strait could not be determined precisely and seemed to vary according to the SB survey. As a result, an infinite supply of sediment was chosen for the reference scenario (also the default case in MIKE21). To try to reproduce a scenario with bedrock and the sandy area, the simulation SC4 was set with a non-erodible layer, except at the location of the sand waves where 5 m was chosen for the area of SW2–SW4 and 12 m for SW1 (value estimated from the SB survey). The migration of the sand waves was slower...
with a non-erodible layer around the sand waves (Figure 14). Berthot [79] tested 2 m and 4 m of availability of sand in his model for the evolution of sandbank and showed that when the layer thickness was smaller, the evolution was slower.

Figure 14. (Top): Comparison of the migration of the sand waves between a variable availability of sand (SC4), Deigaard (Middle) Four sorting value for the sediment (SC5/6/7). (Bottom): Three different values of Manning Strickler (SC8/SC9).
3.3.4. Sorting

Sorting values of 2 to 1.1 (reference case) were varied to determine their influence on the migration of sand waves. The value of 2 corresponded with poorly sorted, 1.62 for the limit between poorly and moderately sorted [52], 1.3 was chosen to see the sensitivity in the range of well sorted sediment, and 1.1 was the default case in MIKE21. All the values tested gave higher migration rates for SW2 crest, compared to the reference case. When the sediments were more mixed, the evolution was faster, with the migration rates being very sensitive to this sorting parameter (Figure 14).

3.3.5. Bed Friction

The bed friction represented in the model by the Manning number (M) was modelled with a constant value of 32 (from the calibration of [47]) for the whole domain for all of the runs, except for two simulations where the Manning number was modified just at the location of the sand waves with the value of 28 for SC8 and 40 for SC9. With a smoother bed (M = 40) at the location of the sand waves, the migration rates for SW2 were two times higher compared to the reference case approaching the observed rates and shape (asymmetrical) (Figure 14). With a smaller Manning number (M = 28), the evolution of sand waves for SW2 was almost twice as small compared to the reference case and slightly more symmetrical. A smaller Manning number induced an increased bed shear stress, leading to more turbulence and possibly more suspended load transport which could explain the slower evolution.

3.4. Sensitivity Tests: Tides & Waves, Pure Current

The reference scenario (wave and current) was compared to simulations without wave forcing (SC10/11/12) for the period of 9 months. For the pure current cases, three different sediment transport formulae were tested: Van Rijn (VR) equations [60,80] in equilibrium and non-equilibrium mode and Engelund and Fredsøe (EF) [65] in non-equilibrium mode. The EF formula in equilibrium mode was not tested, given the instability [81] found in their sensitivity tests for energetic sites. For the three cases with current forcing only, the migration of crests and troughs was not well represented as shown in Figure 13. The sand waves were totally flattened, and the bottom of the seabed was lower with the EF formula. Auguste [81] found that the EF formula was more sensitive to basic physical properties, such as strong currents, than the VR formula and may not be suitable in a highly energetic site. For the reference scenario (wave + current) the depth-averaged current was reduced up to 0.1 m/s with an average reduction of 0.02 m/s for the profile AA’. The wave–current interaction is important in the Banks Strait [82] and thus simulations with just tidal current alone are not relevant here for investigating the evolution of the sand waves as shown in Figure 15 with the flat lines for pure current.

Another sensitivity test was carried out to investigate the influence of tidal amplitude on the migration of sand waves. The amplitude for this test case was modified by a factor of two when compared to the reference scenario (Figure 16). Higher amplitude showed larger migration of the sand waves towards with east (+68% for SW2) with more deposition in the troughs, inducing a reduction in the sand wave height. The smaller amplitude resulted in almost no migration of the sand waves, with a slight reduction in the sand wave height compared to the initial state (16/03); the results were comparable to the study of [38] for a reduction of the amplitude of symmetrical tide.
**Figure 15.** Comparison of the migration of the waves for simulation with (reference case) and without waves (SC10/11/12) with different sediment transport formula: VR = Van Rijn [80]; EF = Engelund and Fredsøe [65] (E = Equilibrium, NE = Non-equilibrium).

**Figure 16.** Comparison of the migration of the sand waves for simulations with different amplitude for the tides (SC13/14) and different significant wave height (Hs) (SC15/16).
For the significant wave height (Hs), the same sensitivity tests were performed with a factor of 2 (Figure 16). No migration was observed for the sand waves with higher waves; the crests were flattened, except for SW1. Increasing waves height involved more turbulence and increased the bed shear stress compared to the reference case +17% along the profile AA’. In this shallow area, the amount of sediment lifted into suspension was greater with the higher wave height, which may have led to greater suspended load transport and deposition in other locations. This was in accordance with the findings of Tonnon [5] who showed no migration for the artificial sand waves, due to the increasing stirring effect with increasing waves. The reversal of SW1 can be explained by the dominance of the waves on the tides in case of higher waves as King [40] suggested in his study. Smaller waves led to higher crests and more migration towards the east. Smaller waves enhanced sand transport in the residual direction, and there was an increase in the total load transport of +87% along the profile AA’, compared to the reference case.

3.5. Sensitivity Tests: Residual Current

The residual current is one of the dominant factors for the migration of the sand waves. In order to determine the influence of the magnitude of residual current on the evolution of sand waves, a residual current was added to the tidal residual. In Section 2.3.2, the difference between ADCP residual and tidal residual from the model were measured with a maximum of 0.05 m/s. Thus, the value of 0.05 m/s was added in the forcing for the u component, and the double of this value was also tested. The sand waves migrated more towards the east approaching the observed rates for the high (+0.1 m/s) residual model (SC18), with an increase to the east of 47 m compared to the reference scenario for the crest of SW2. For the value of 0.05 m/s, the evolution was quite similar to the reference scenario with an increase of 0.6 m for the height of SW2 crest (Figure 17). A constant value for the residual current was added in this study, but residual currents vary seasonally, and the direction of the added component may also be important for the evolution of the sand waves [73].

3.6. Summary and Recommendations

3.6.1. Summary of Sensitivity Experiments

In the Banks Strait, the migration of sand waves was studied, and special attention was given to the influence of several parameters on the migration: properties of sediment (grain size, spatial distribution of sediment, layer thickness for the seabed, sorting of sediment and bed friction) and hydrodynamics (pure current, amplitudes of tides, height
of significant wave and value of residual current) (Table 5). Various model parameters were found to affect the migration of sand waves.

The cases of pure current (SC10/11/12) showed flat lines for the migration of sand waves, highlighting the importance of the wave–current interaction for sediment dynamics in the Banks Strait. Compared to tidal forcing alone, the combined wave–current model generates more turbulence from the wave boundary layer and from energy dissipation, due to wave breaking. More sediment is available to influence the sand wave migration. Spatial distribution of the grain size (SC3) and the non-erodible layer case (SC4) revealed less migration compared to the reference case. Variations of grain size (SC1/2) displayed similar results for coarse and very coarse sand but were two times slower for medium sand. Identification of the classification (Wentworth scale [51]) of sediment is an important element especially if finer sand is the dominant component. Amplitude of tides (SC13/14) and waves (SC15/16) modify the propagation of the sand waves and the dynamic of sediment with more migration than the reference case for higher values.

Of all the sensitivity tests cases, only three showed migration rates approaching the observed rates especially for SW2: poorly sorted sediment (SC7), mixed bed friction with a constant value in the domain and a higher value in the sand waves area (SC9) and an added value of 0.1 m/s for the residual current (SC18).

For the case of SC7 (poorly sorted sediments), the sediments were more mixed and thus reacted differently to critical bed shear stress. With an increase in the grading coefficient, finer fractions were suspended and coarser fractions moved, as the bedload had a greater tendency to settle. More migration was observed compared to the reference case, certainly due to an increase in the deposition rates. Values between 2 and 1.62 may correspond to the sorting representation for the 12 AUSTEn samples, but more detailed in situ data are needed to optimize the value of this coefficient and were not available. Further processes that could be investigated is the separation of the sand waves in different size fractions; observations have mostly found coarser sediment on the crests and finer in the troughs as discussed by Roos [8] and Van Oyen [34]. For this analysis, more sediment samples are needed at the location of the sand waves.

Simulation using a higher Manning coefficient value in the sand waves area (SC9) led to migration rates twice as great as those of the reference scenario. Using less friction in the model domain and thus less turbulence in this area could have implied more deposition. The hydrodynamic model was validated with a constant value of M = 32 [47]. However, a variable bed roughness should be tested in future work to see the influence of the distribution on the validation, ideally by using more in situ sediment and current data in proximity to the area to refine the validation process. For current data, this is a challenging task, given that two ADCP frames were buried and could not be retrieved at the location of the sand waves. Boat mounted ADCP can be used in future surveys to overcome this difficulty.

Increased residual current (SC18) resulted in an increase in the migration of the sand waves towards the east, with model results close to the measured migration rates of SW2. The tidal asymmetry is not the only driver of the sand wave migration. It is a combined effect of tidal asymmetry and large-scale ocean circulation. Further investigations about the variations of the residual current in this area or inclusion of large-scale ocean circulation at the boundaries could potentially lead to better agreement with observations but are outside the scope of this study.

3.6.2. Recommendations for Future Field Work

Based on these results, future field work should aim at deploying the instruments listed in Table 12 to collect data for preliminary environmental impact assessment for tidal energy sites, in complement to the EMEC/IEC-TS guidelines [83,84]. This will help tidal project developers and researchers to prepare while reducing cost for future campaigns for potential tidal sites.
Table 12. Recommendations for “Ideal” field surveys for tidal energy.

|                          | High Energetic Site | High Energetic Site with Known Bedforms |
|--------------------------|---------------------|----------------------------------------|
|                          | Limited Budget      | Desired                                |
| Multi-beam survey        | x                    | x                                      |
| Several multi-beam surveys (3 years) |                      | x                                      |
| Grab sampler/Sediment trap | x                | x                                      |
| Penetrometer             | x                    | x                                      |
| Sub-bottom profiler      |                      | x                                      |
| ADCP (90 days of data)   | x                    | x                                      |
| ADCP (1 year of data)    |                      | x                                      |

1 in the crests, troughs, and along the slopes of the bedforms.

3.6.3. Recommendations for Numerical Modelling

For numerical modelling, the authors recommend the following parameters for preliminary morphological assessment:

- Constant grain size identified as the most typical for the area considered based on the Wentworth scale/ISO 14688 [85].
- Optimized value of sediment sorting of sediment (if a large set of sediment samples is available).
- Infinite supply for the layer of thickness for a first approximation.
- Spatial distribution of bed friction coefficient (if a large set of in situ data is available).
- The site’s exposure to waves should be assessed when analysing sediment transport, especially in the presence of bedforms, such as sand waves. If the wave–current interaction is important, wave forcing or a coupled wave/current model should be considered.
- Ocean circulation forcing at the boundaries, if a larger model is available.

4. Conclusions

A high resolution 2D hydrodynamic–morphodynamic model with wave forcing was used to explore the dynamics of sediment transport and of sand waves located in the Banks Strait, a promising site for tidal arrays deployment in Australia. The hydrodynamic model was validated against a large data set of ADCP data with good agreement for the depth average velocities and elevations. Averaged bed shear stresses for the whole period showed high rates largely higher to the critical value for coarse sand, highlighting the turbulent environment in this area. The area of South Clarke Island is very energetic, as evidenced by the silting of two ADCP frames up to 1 m within 5 months. Migration rates in excess of one hundred meters for the period of deployment (9 months) were found in the Banks Strait and were superior to the values usually found in the literature (several meters by year) for other sites. The developed model was able to reproduce migration patterns towards the east; however, the model produced smaller sand wave evolution rates compared to the observation. Long-term surveys on the scale of years/decades would indicate the validity of these shorter-term observations.

For preliminary environmental assessment in tidal energy sites, in situ data sets are necessary to validate the hydrodynamics and have insight about the sediment properties. The study investigated the parameters that mostly drive sediment transport under the action of waves and current and uses sand waves migrations south of Clarke Island to perform sensitivity testing. The sensitivity tests showed that sorting, bed roughness and residual current were key factors to the migration of the sand waves, whilst grain size, amplitude of tides and waves revealed weaker impact on sand wave migration. The migration of the sand waves in Banks Strait is strongly linked to the hydrodynamics, therefore any changes to the hydrodynamic conditions, such as tidal energy extraction may alter the sediment dynamics. The numerical model was run with wave forcing. Further
work should investigate the difference between this approach and a fully coupled spectral wave/hydrodynamic/sand transport model. The full coupled model may give better agreement since it may better capture higher peaks in waves heights during extreme events, which are significant for sediment dynamics.

This paper gives new insights into the seafloor characteristics in this area and provides crucial information for tidal projects developers: an increased knowledge of the substrate type for turbines foundations and an assessment of natural sediment activity for future turbines deployments. Further studies using this model are in progress to investigate the influence of tidal stream energy extraction on the morphology of the Banks Strait and especially the migration of sand waves.

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**Abbreviations**
The following abbreviations are used in this manuscript:

- ADCP: Acoustic Doppler Current Profiler
- AHS: Australian Hydrography Service
- ARENA: Australian Renewable Energy Agency
- AUSTEn: Australian Tidal Energy
- CSIRO: Commonwealth Scientific and Industrial Research Organisation
- DAV: Depth Average Velocities
- EAC: East Australian Current
- EF: Engelund and Fredsøe
- GA: Geoscience Australia
- M: Manning number
- SASW: Sub-Antarctic Surface Water
- SB: Sub-Bottom
- SUF: Speed Up Factor
- SW: Sand Waves
- VR: Van Rijn
### Appendix A

#### Table A1. Bottom Grab (G) and Sediment Traps (ST) characteristics.

| Latitude  | Longitude | Date                                      | Mean Grain Size | Wentworth Classification |
|-----------|-----------|-------------------------------------------|-----------------|--------------------------|
| G1/ST_C1  | −40.6727  | 148.2388                                  | 6 December 2019–16 February 2019 | 671 µm                  | CS                        |
| G2        | −40.69005 | 148.1296                                  | 8 December 2018 | 1087 µm                 | VCS                       |
| G6        | −40.6354  | 148.0332                                  | 10 December 2018 | 1390 µm                 | VCS                       |
| G9        | −40.59549 | 148.1606                                  | 10 December 2018 | 958 µm                  | CS                        |
| G10       | −40.60083 | 148.2083                                  | 10 December 2018 | 882 µm                  | CS                        |
| G11       | −40.68528 | 148.089                                   | 10 December 2018 | 1388 µm                 | VCS                       |
| G12       | −40.75377 | 148.118                                   | 10 December 2018 | 415 µm                  | MS                        |
| ST_CW2    | −40.701   | 148.2013                                  | 12 July 2018–7 December 2018 | 504 µm                  | CS                        |
| ST_CW4    | −40.7296  | 148.345                                   | 13 July 2018–7 December 2018 | 595 µm                  | CS                        |
| ST_CW1    | −40.5294  | 148.0568                                  | 12 July 2018–12 December 2018 | 1368 µm                 | VCS                       |
| ST_Swan1  | −40.68815 | 148.1228                                  | 6 December 2019–16 February 2019 | 584 µm                  | CS                        |
| ST_Swan2  | −40.68788 | 148.1205                                  | 6 December 2018–8 December 2018 | 718 µm                  | CS                        |

#### Table A2. Bottom Grab (G) and Sediment Traps (ST) characteristics with Folk diagram modified from [53].

| Mud% | Sand% | Gravel% | Sediment Type         |
|------|-------|---------|-----------------------|
| G1/ST_C1 | /     | 85      | 14                    | gravelly Sand          |
| G2    | /     | 89      | 11                    | gravelly Sand          |
| G6    | /     | 59      | 41                    | sandy Gravel           |
| G9    | /     | 88      | 12                    | gravelly Sand          |
| G10   | /     | 88      | 12                    | gravelly Sand          |
| G11   | /     | 68      | 32                    | sandy Gravel           |
| G12   | /     | 100     | 0.2                   | Sand                   |
| ST_CW2  | /     | 98      | 2                     | slightly gravelly Sand |
| ST_CW4  | /     | 97      | 3                     | slightly gravelly Sand |
| ST_CW1  | /     | 67      | 33                    | sandy Gravel           |
| ST_Swan1 | /     | 96      | 4                     | slightly gravelly Sand |
| ST_Swan2 | /     | 88      | 12                    | gravelly Sand          |

#### Table A3. Bottom Grab (G) from the 2018 AUSTEn survey compared to the 1966 survey performed by R.Slater [3].

|        | 2018                  | 1966                  |
|--------|-----------------------|-----------------------|
| G6     | sandy Gravel          | gravelly Sand         |
| G1     | sandy Gravel          | gravelly Sand         |
| G12    | Sand                  | Sandy                 |

#### Table A4. Details of ADCP [47].

| Name Station | Type of Instrument    | Longitude | Latitude | Depth(m) | Date of Deployment | End of Data Collected |
|--------------|-----------------------|-----------|----------|----------|--------------------|-----------------------|
| CW2          | RDI Sentinel V50 500 kHz | 148.10188 | −40.5848 | 46.47    | 22/03/2018         | 11/07/2018            |
| C1           | RDI Workhorse 300 kHz   | 148.23882 | −40.6727 | 57.94    | 17/03/2018         | 10/07/2018            |
| CW3          | Nortek AWAC 1 MHz       | 148.07778 | −40.5454 | 34.95    | 22/03/2018         | 16/06/2018            |
| CW4          | Nortek AWAC 1 MHz       | 148.09241 | −40.6664 | 30.67    | 15/03/2018         | 09/06/2018            |
| CWTb1        | Nortek Signature 500 kHz | 148.22626 | −40.6672 | 63.57    | 22/03/2018         | 09/07/2018            |
| CW1          | RDI Sentinel V50 500 Hz  | 148.05684 | −40.5294 | 27.11    | 12/07/2018         | 06/09/2018            |
| CW2bis       | RDI Sentinel V50 500 Hz  | 148.20132 | −40.701 | 46.08    | 12/07/2018         | 22/09/2018            |
| CW4bis       | Nortek AWAC 1 MHz       | 148.34497 | −40.7296 | 25.42    | 13/07/2018         | 08/09/2018            |
| C1bis        | RDI Workhorse 300 kHz   | 148.12498 | −40.6891 | 29.07    | 05/12/2018         | 15/02/2019            |
Figure A1. Interpolation map for grain size classification based on AUSTEn data set and samples from MARS [54].

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