Abstract. This study focuses on the finite element simulation of piles with different models in sandy soils using the software PLAXIS 3D V20. The parametric study has conducted to investigate the influence of multiple parameters on the axial capacity of steel piles in sandy soil, including the cross-section variables in two cases: open and close-ended piles. The typical circular and square cross-section open and close-ended piles were selected as the reference for comparison with variables cross-section piles. The open-ended tapered pile 3b showed an increase in the maximum load capacity about 210% more than the open-ended circular section, while the close-ended tapered pile 3b showed an increase of about 176% in the axial load capacity more than the solid close-ended circular section. In terms of the effect of pile’s type, all of the close-ended sections outperformed the open-ended sections, with the circular section showing a 146% increase in its close-ended section, while the tapered 3b section showed the lowest difference between the close-ended and the open-ended sections with just 120% increase. These results showed that the tapering pile is much more efficient than any straight-sided pile or even circular pile. The results also showed that a short open-ended pile's capacity is smaller than the corresponding closed-ended pile.

Keywords: Tapered pile; square pile; circular pile; open and close-ended piles; finite element method.
experimental field study, the bearing capacity of bored-cast-in-place tapered piles was investigated by [5]. He indicated that the tapered piles tested had a general bearing capacity exceeding 20-30 percent of the specific bearing capacity of straight cylindrical piles of the same length. Local studies have been conducted to investigate the effect of loading type and soil relative density on steel piles' performance [6, 7]. The tapered pile is shown in Figure 1.

The main objective of this work is to investigate the effect of different pile cross-sections on axial load capacity by means of:

1. Using the geotechnical package PLAXIS 3D for modeling conventional straight-sided and tapered piles’ sections.
2. Using the hardening soil model with small strain to accurately simulate soil behavior
3. Investigating the effect of section geometry on solid and pipe piles' behavior in terms of load-displacement curves.

Materials

Soil. The Hardening Soil model is an advanced model for modeling the behavior of multiple soil types, including soft and rigid soils [8]. The soil displays a decreasing stiffness when exposed to primary deviatoric loading, and permanent plastic strains grow at the same period. A hyperbola can approximate the observed relationship between the axial strain and the deviatoric stress in the special case of a drained triaxial measure. This relationship was initially established by [9] and utilized later in the renowned hyperbolic model [10]. However, the Hardening Soil model exceeds the hyperbolic model by far: first, using the principle of plasticity rather than elasticity, second, by using soil dilatancy, and third, by introducing a yield limit. The parameters of sand are listed in Table 1. They are selected from the study conducted by [11]. All of the selected pile models have been modeled as solid sections (tetrahedral elements) and hollow sections (plate elements). The thickness of the hollow sections was kept as 1.44 mm as investigated by [11].

| Soil properties            | Unit | Value      |
|----------------------------|------|------------|
| Dry unit weight, $\gamma$  | kN/m$^3$ | 15.5      |
| Secant modulus, $E_{50}$   | kPa   | 15000      |
| Odometer modulus, $E_{OED}$| kPa   | 15000      |
| Unloading/reloading modulus, $E_{UR}$ | kPa | 45000 |
| Cohesion, $c$              | kPa   | 0.1        |
| Friction angle, $\phi$     | Degree | 31         |
| Dilatancy angle, $\psi$    | Degree | 1          |
| Poisson’s ratio, $\nu$     | -     | 0.2        |
| Power for stiffness stress dependency, $m$ | - | 0.625 |
| $\gamma_{0.7}$            | -     | $0.176 \times 10^{-3}$ |
| $G_{0,ref}$                | kPa   | 75000      |
Steel Piles. The pile was modeled as a volume pile with three cross-sectional shapes (circular, square, and varied). The input properties of the piles are shown in Table 2:

| Pile properties | Unit | Circular | Square | Tapered |
|-----------------|------|----------|--------|---------|
| Unit weight, $\gamma$ | kN/m³ | 78.5     | 78.5   | 78.5    |
| $E$ | kPa | 200E6 | 200E6 | 200E6 |
| $R_{inter}$ | - | 0.8 | 0.8 | 0.8 |

Pile cap. The pile cap was modeled using plate element, which also consists of triangular surface elements with 6 nodes and 3 translational degrees of freedom per node (ux, uy and uz)[10]-[11]. As for the load, a vertical point load of 0.5 ton (5 kN) was applied at the cap’s center. The loading value was chosen based on trial and error to ensure that all piles would reach their capacity. Figure 2 shows a mesh view of the pile cap while Table 3 presents the cap’s properties.

Table 3 Pile cap model.

| Pile section | D or B (mm) | L/D or L/B |
|--------------|-------------|------------|
| Circular     | 30          | 15         |
| Square       | 30          | 15         |
| Tapered (1.5b) | 45       | 15         |
| Tapered (2b) | 60          | 15         |
| Tapered (2.5b) | 75         | 15         |
| Tapered (3b) | 90          | 15         |

Models

The parametric study has been conducted to investigate the influence of multiple parameters on the axial capacity of steel piles in sandy soil, including the pile's cross-section, the section type (hollow and solid). The typical circular cross-section was selected as the reference section. Additionally, square and varied (tapered) sections were investigated in this study. The circular pile dimensions have been selected depending on the experimental study conducted by [11]. Table 4 summarizes the dimensions of the FE models, and Figure 3 shows the dimensions of the typical cross-sections, while Figure 4 shows the details of the tapered sections.
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Table 4. Dimensions of the FE models.

| Pile cap model | Properties | Unit |
|----------------|------------|------|
| Unit weight, γ | 78.5       | kN/m³ |
| E              | $200 \times 10^6$ | kPa |
| Poisson’s ratio | 0.2       | -    |
| Thickness      | 5          | mm   |

Figure 3. Dimensions of the typical pile sections: a) circular section. b) square section.

Figure 4. Dimensions of the tapered cross-sections.
Results and Discussion

Figure 5 presents the results of the load-displacement curve for the solid and hollow circular and square pile. For the circular section, the solid section's axial loading capacity was 1622 N and 837 N for the hollow section. The solid circular section's ultimate load capacity was higher than the experimental results obtained by [11], which was 565 N while the open-ended section exhibited a closer value of load capacity (685 N). The difference in these values can be attributed to the rate of loading and different testing conditions. The square solid and hollow section's axial loading capacity was 2132 N and 1562 N, respectively, which were significantly higher than those of the circular section. These values shall be used as reference values to compare all the models analyzed in this study. The hollow circular and square sections showed the lowest axial load capacity compared to the solid circular and square because of the largest end bearing component in sandy soil.

![Figure 5. Load-displacement curve for the circular and square sections.](image)

Effect of Pile Shape. Figure 6 summarizes the results for the close-ended (solid) sections, and Figure 7 summarizes the results of the open-ended (hollow) sections. Figures 6 and 7 show the results of the tapered solid and hollow sections. It can be shown that the maximum axial load capacity of the tapered pile (3b) was 2868 N and 2387 N for the solid section and the hollow section, respectively. The remaining tapered models showed a decrease in the values of load capacity with decreasing the section size. The tapered section (2.5b) produced slightly lower values for a load capacity of 2650 N for the solid section and 2181 N for the hollow section. The (2b) section capacities were 2473 N and 1924 N for the solid and the hollow sections, respectively. The (1.5b) section showed the lowest load-capacity values for both sections, with 2324 N for the solid section and 1768 N for the hollow section. These results show that increasing the tapering area increases the axial load capacity for both the solid and the hollow sections. The results also demonstrated that the tapering pile is much more efficient than any straight-sided pile or even circular pile [14]. In terms of the displacement values, all of the tapered piles showed relatively similar values ranging from 113 mm for the (1.5b) section to 130 mm for the (2.5b) section for the close-ended (solid) sections. For the open-sided (hollow) sections, the displacement values also were relatively close, ranging from 102 mm for the (2b) section to 113 mm for the (1.5b) section. It should be noted that the displacement increases proportionally with axial pile capacity due to the failure of soil occurring at later loading stages; hence the pile is displaced further. However, it should be mentioned that there was no significant difference in the values of pile displacement due to the loose state of the utilized soil.
It should be noted that there was no significant difference in the values of load capacity with decreasing the tapering area increases. The results also demonstrated that the tapering pile is much more efficient than any straight-sided pile or even circular pile. In terms of the displacement values, all of the tapered models showed relatively similar values ranging from 102 mm for the (2b) section to 113 mm for the (2.5b) section for the close-tapered piles. These results show that increasing the tapering area increases the maximum axial load capacity.

Table 5 presents a summary of the axial load capacity results, while Table 6 summarizes the results for displacement.

Table 5. A summary of the results of the axial load capacity.

| Type          | Circumferential | Square | Tapered 1.5b | Tapered 2b | Tapered 2.5b | Tapered 3b |
|---------------|-----------------|--------|--------------|------------|--------------|------------|
| Solid         | 1623 N          | 2132 N | 2323 N       | 2473 N     | 2650 N       | 2868 N     |
| Hollow        | 1107 N          | 1562 N | 1759 N       | 1924 N     | 2181 N       | 2387 N     |
| Ratio of load-capacity to solid circular section | | | | | | |
| Solid         | 100%            | 131%   | 143%         | 152%       | 163%         | 176%       |
| Hollow        | 68%             | 96%    | 108%         | 118%       | 134%         | 147%       |
| Ratio of load-capacity to solid square section | | | | | | |
| Solid         | 76%             | 100%   | 82%          | 90%        | 102%         | 134%       |
| Hollow        | 51%             | 73%    | 108%         | 118%       | 134%         | 111%       |
Table 6. A summary of the displacement results.

| Shape     | Circular | Square | Tapered 1.5b | Tapered 2b | Tapered 2.5b | Tapered 3b |
|-----------|----------|--------|--------------|------------|--------------|------------|
| Solid     | 184 mm   | 149 mm | 113 mm       | 122 mm     | 130 mm       | 120 mm     |
| Hollow    | 108 mm   | 213 mm | 113 mm       | 102 mm     | 109 mm       | 102 mm     |
| Ratio of |          |        |              |            |              |            |
| displacement to | Solid 100% | 81%   | 61%          | 66%        | 71%          | 65%        |
| solid circular section | Hollow 59%    | 116%  | 61%          | 55%        | 59%          | 55%        |
| Ratio of |          |        |              |            |              |            |
| displacement to | Solid 123% | 100%  | 76%          | 82%        | 87%          | 81%        |
| solid square section | Hollow 72%   | 143%  | 76%          | 68%        | 73%          | 68%        |

**Displacement Contours.** In this section, graphical representations of the total displacement in the cross-sections will be displayed in Figures 8 to 13 show the state of the FE models at the moment of soil failure.

Figure 8. Total displacement for the circular sections, A. Solid section, and B. Hollow section.

Figure 9. Total displacement for the square sections, A. Solid section, and B. Hollow section.
Table 6. A summary of the displacement results.

| Shape Type | 1.5b | 2b | 2.5b | 3b |
|------------|------|----|------|----|
| Circular   | 184 mm | 149 mm | 113 mm | 122 mm | 130 mm | 120 mm |
| Solid      | 184 mm | 149 mm | 113 mm | 122 mm | 130 mm | 120 mm |
| Hollow     | 108 mm | 213 mm | 113 mm | 102 mm | 109 mm | 102 mm |

The ratio of displacement to solid circular section:

- Solid: 100%, 81%, 61%, 66%, 71%, 65%
- Hollow: 59%, 116%, 61%, 55%, 59%, 55%

The ratio of displacement to solid square section:

- Solid: 123%, 100%, 76%, 82%, 87%, 81%
- Hollow: 72%, 143%, 76%, 68%, 73%, 68%

Figure 10. Total displacement for the tapered 1.5b sections, A. Solid section, and B. Hollow section.

Figure 11. Total displacement for the tapered 2b sections, A. Solid section and B. Hollow section.

Figure 12. Total displacement for the tapered 2.5b sections, A. Solid section, and B. Hollow section.
Conclusions

This study deals with the effect of pile type, pile shape on the axial loading capacity. Two FE models (solid and hollow) have been conducted in this study using the FE package PLAXIS 3D V20. The following can be concluded from the results:

- The open-ended (hollow) circular model has demonstrated the lowest values of load capacity of 1107 N.
- The close-ended (solid) tapered section (3b) showed the highest load capacity of 2868 N with an increment ratio of 176% and 134% compared to the solid circular and solid square sections.
- Increasing the upper cross-sectional area of the tapering section increases the load capacity.
- The load capacities of close-ended (solid) piles were higher than the open-ended (hollow) pipes for all the utilized sections.
- The close-ended (solid) circular section showed the highest pile displacement of 184 mm.
- Both of the open-ended (hollow) tapered sections 2b and 3b have resulted in the lowest pile displacement values with only 102 mm for both sections.
- There was no significant difference in terms of pile displacement in all of the models due to the soil's loose state.

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- Increasing the upper cross-sectional area of the tapering section increases the load capacity.
- The load capacities of close-ended (solid) piles were higher than the open-ended (hollow) pipes for all the utilized sections.
- The close-ended (solid) circular section showed the highest pile displacement of 184 mm.
- Both of the open-ended (hollow) tapered sections 2b and 3b have resulted in the lowest pile displacement value with only 102 mm for both sections.
- There was no significant difference in terms of pile displacement in all of the models due to the soil's loose state.

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