Single Pick Cutting Rock Load Identification Based on Improved Regularization Method

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Abstract To explore the relationship between the cutting vibration and the cutting load of a single pick, this paper studied a new method for a single pick cutting rock load identification. This paper improved the low accuracy problem of the regularization method in the inverse process of frequency response function in the traditional load identification method by introducing a filter operator. By combining the inverse pseudo excitation method and the improved regularization method, the identification of the load dependent on the vibration signal was realized. A single pick cutting rock test equipment was built, which could simulate the actual working conditions of pick cutting rock in underground or tunnel. By changing cutting speed, cutting angle, cutting line spacing and cutting depth of the single pick, the change trends of real cutting load and identification load were obtained. The load identification method proposed in this paper was consistent with the change trend of the real load under the single pick cutting state. Therefore, the method had good recognition accuracy and the maximum load recognition error was 17.35%. Compared with the traditional load identification method, the identification error was reduced by a maximum of 1.98%. This method can identify the cutting load of single pick and modify the morbidity problem of frequency response function matrix. The method has a better recognition effect on the cutting load of the pick than the traditional recognition method. The research could benefit the design of the cutting system and the arrangement of the pick on the coal mine or tunneling machinery.

Keywords Pick, Regularization method, Load identification, Cutting test

1 Introduction

The cutting system is the most important mechanical system in coal mining or tunneling machinery. It is the key to determine the cutting performance and cutting efficiency. The pick is the part of the cutting system that is in direct contact with the cutting object, which bears the cutting load and completes the tasks of rock entering and rock breaking. Parameters such as cutting line spacing, cutting speed, cutting angle and cutting depth of the pick according to the arrangement and combination of the pick play a essential role in the design and manufacture of the cutting system. Therefore, it is one of the current research topics of tunneling machinery to study...
the effect of cutting parameters on rock breaking load based on hardness and other material properties of the cutting object.

Acquiring the cutting load generally adopts the method of directly measuring the cutting electricity. However, the cutting electricity is often mixed with auxiliary electricity such as walking electricity, hydraulic electricity and dust removal electricity, which are not easy to distinguish. The cutting electricity also contains comprehensive cutting information such as the feed force and the rotation force, which make it difficult to obtain the information of the pick breaking load (Wang et al. 2013). Therefore, it is necessary to build a single pick rock breaking test system and to establish a method to study the influence of a single pick cutting parameters on the cutting load. Dognuoz et al (2014) used picks with different degrees of wear and conducted cutting tests on a variety of different types of rocks. They obtained the law of the single pick cutting energy. Yang et al (2015) conducted cutting experiments on picks with different cutting angles and studied the influence of pick installation parameters on pick wear.

Load identification is the inverse problem of structural dynamics. It is a process of identifying excitations according to characteristic parameters and responses of the structure itself. Load identification technology has been widely used in various fields such as structural health monitoring in civil engineering (He et al. 2011) and durability testing in the automotive field (Raath and Waveren 1998). Load identification methods can be roughly divided into three categories: time domain method (Liu et al. 2016), frequency domain method (Wu et al. 2018) and modern intelligent method (Lee and Liu 2014). Time domain method includes series expansion method (Li and Deng 2016), Kalman filter method (Zhi et al. 2018) and inverse system method (Wang et al. 2016). The application in complex mechanical systems has great limitations, because the input and output of the time domain system are relatively complicated convolution relationship and the amount of calculation after discretization is too large. Modern intelligence method includes neural network method (Zhou et al. 2019), wavelet transform method (Hassan et al. 2015; Patel et al. 2018) and genetic algorithm method (Wei and Zhang 2018; Treetrong et al. 2014). It is rarely used in practical engineering applications, because the modern intelligence algorithm requires a large number of training samples and the establishment of the topology structure is difficult (Ren et al. 2018). Although the frequency domain method also has many shortcomings such as morbidity of frequency response function and modal truncation, it is widely used in actual engineering situations because it requires fewer training samples and the calculation amount is small.

The frequency response matrix in the load inversion problem is often morbid. The proper handling of the morbidity problem is the key to the success of load identification. Choi et al. (2006; 2007) used Tikhonov regularization method to improve the stability of load identification results and compared the effect of different regularization parameter selection methods on load identification accuracy.

In this paper, the inverse pseudo excitation method in the frequency domain method is used to identify the load of single pick rock breaking load. The modified regularization method is used to solve the morbidity problem encountered in the inverse process of the frequency response function matrix. A single pick rock breaking test equipment was built to obtain the trend of the pick cutting load with the cutting angle, the cutting speed, the cutting line spacing and the cutting depth. Finally, the identification load is compared with the actual measured value and the reason for the error is analyzed. The accuracy of the load identification after and before the method improvement is also analyzed. The test results show that the modified regularization method can improve the accuracy of single pick cutting load
identification. The research method can lay a theoretical foundation for the design of the cutting system of the tunneling machinery and the improvement of cutting efficiency.

2 Load identification method

2.1 Inverse Pseudo Excitation Method

One of the most used methods in load identification is the inverse pseudo excitation method (Jacquein et al. 2003). The motion equation of structure under random excitation is expressed as:

\[ [M][\ddot{x}] + [C][\dot{x}] + [K][x] = \{F\} \]  

(1)

Where \([M],[C],[K]\) represent mass matrix, damping matrix and stiffness matrix, respectively; \([F]\) represents the \(n\)-dimensional external force vector. The conversion formula of the power spectral density matrix is expressed as follows:

\[ [S_{YY}] = [H]^T[S_{FF}][H]^T \]  

(2)

In formula (2): \([S_{YY}]\) is \(l\times l\) order response power spectral density; \([H]\) is \(l\times m\) orders frequency response function matrix; \(l, m\) are the number of degrees of freedom of response and excitation, respectively, and \(l \geq m\); the superscripts \(^*\) and \(^T\) represent complex conjugate and transpose of the matrix, respectively. Both sides of the equation (2) are multiplied by the inverse matrix of the frequency response function matrix to obtain:

\[ [S_{FF}] = [H]^+(S_{YY})[H]^T \]  

(3)

In formula (3): \([S_{FF}]\) is \(m\times m\) orders excitation power spectral density matrix; \(\cdot^+\) and \(\cdot^T\) means seeking generalized inverse. The frequency response function \([H]\) is generally not a square matrix but a general form of direct inversion, which is a very computationally intensive task for complex structures. Seeking the generalized inverse directly is often ineffective.

For the inverse problem of multipoint \((l>m)\) arbitrary excitation, the known response spectrum matrix is decomposed into:

\[ [S_{YY}] = \sum_{j=1}^{r} \{b\}_{j}^T \{b\}_{j} \]  

(4)

Where \(r\) is the rank of \([S_{YY}]\), and \(m \leq r\), \(\{b\}_{j}\) is the \(j\)-th order feature pair of the Hermite matrix. To construct a pseudo response:

\[ \{y\}_{j} = \{b\}_{j} e^{j\omega t} \]  

(5)

To get inversion corresponding pseudo excitation:

\[ \{f\}_{j} = [H]^+\{y\}_{j} = [H]^+(b)_{j} e^{j\omega t} = (a)_{j} e^{j\omega t} \]  

(6)

Therefore, the excitation spectrum matrix can be obtained as:

\[ [S_{FF}] = \sum_{j=1}^{r} (a)_{j}^T \cdot (a)_{j}^T \]  

(7)

Where \((a)_{j} = [H]^+(b)_{j} , \{b\}_{j} = \sqrt{\lambda_{j}}(\psi_{j})^T \) is the eigenvalue of \([S_{YY}]\) and \(\psi\) is the eigenvector.

The frequency response function of the system can be obtained by the finite element method. The test device is a single pick and single swing device, hence can be simplified to a cantilever beam. The free mode and working mode of the cantilever beam could be obtained in (Song et al. 2018; Song et al. 2019). Therefore, the solution of the natural frequency response function of the test device simplified as a cantilever beam will not be explained too much in this paper.

2.2 Modified Regularization Method

The inverse pseudo excitation method needs to invert the frequency response function to solve the pseudo excitation or test excitation. The regularization method to solve the inverse matrix of the frequency response function is a method that is easy to understand and has a short calculation time. However, this method often results in a lower load identification accuracy because of morbidity matrix or improper selection of parameter values. Therefore, this paper established a new selection criterion for the key parameters in the regularization method to improve the accuracy of the traditional load identification method.

(1) Singular Value Decomposition

The singular value decomposition method is often used to calculate the generalized inverse of
the matrix. Singular value decomposition of $H$ as follows:

$$ [H] = [U][S][V] $$  

(8)

Where $[U]$ represents the left singular value vector; $[V]$ represents the right singular value vector; $[S]$ represents the singular value vector.

Since the matrix is morbid at the natural frequency during the inversion, in general, the frequency response function inverse matrix $[H]^+$ can be expressed as:

$$ [H]^+ = \sum_{i=1}^{m} \frac{1}{s_i} u_i^T v_i $$  

(9)

Where $H^+$ is Moore-Penrose generalized inverse; $u_i$ represents the left singular value vector; $v_i$ represents the right singular value vector; $s_i$ represents the singular value.

(2) Modified Regularization Method

If: ① The singular value of matrix $H$ gradually becomes zero; ② The condition number of matrix $H$ is too large, that is, the ratio between the largest singular value and the smallest singular value of structure matrix $H$ is larger; when one of the above conditions is met or all conditions are met at the same time, the problem is ill-posed. To seek a set of stable approximate solutions to the equation, a filter operator $g_\lambda(s)$ is introduced in equation (9) as follows:

$$ g_\lambda(s_i) = \frac{1 + \sigma s_i^2}{s_i^2}, \sigma \geq 1 $$  

(10)

Where $\lambda$ is the regularization parameter. The operator $g_\lambda(s)$ is a modified operator including the traditional Tikhonov regularization operator. Therefore, equation (9) can be written as:

$$ [H]^+ = \sum_{i=1}^{m} \frac{1 + \sigma s_i^2}{s_i^2} u_i^T v_i $$  

(11)

With the increase of $\sigma$, the convergence order of the relative error of the regularization solution increases with it. From equation (9), it is found that the regularization parameter $\lambda$ plays an important role in the final solution. When the selected regularization parameter is larger, the load cannot be well identified; when the selected regularization parameter is smaller, the regularized solution of load identification will be unstable and cannot reasonably approximate the load identified. Therefore, a reasonable selection of regularization parameter is the key to the success of regularization solution. At present, the most used method for selecting regularization parameter is the L-curve criterion (Hansen 1999). However, the L-curve is sometimes too smooth to find the $\lambda$ value corresponding to the maximum point of the bending derivative on the curve. Therefore, this paper used the GCV criterion to select the optimal regularization parameter. The GCV function is expressed as (Mao et al. 2010):

$$ G = \frac{\| [u]^T [S_{FF}]^{-1} [H]^T [V] \|_F^2}{\text{trace}((1 - [S_{FF}]^{-1} [H]^T [V])^2)} $$  

(12)

Where $H_{\text{reg}} = (H^T H + \lambda I)^{-1} H^T$, and satisfies $[S_{FF}]_{\text{reg}} = [H_{\text{reg}}]^T [S_{FF}] [H_{\text{reg}}]^T$. When the GCV function takes the minimum value, the corresponding $\lambda$ value is the optimal regularization parameter.

3 Single Pick Cutting Hard Rock Test

3.1 Test Device

The test used the single pick test equipment of the National Engineering Laboratory of Coal Mining Machinery of China, as shown in Figure 1. The test equipment can simulate the process of the pick rock breaking, can test the cutting force, the wear state and the dust amount of a single pick and can change the cutting angle, cutting depth, cutting speed of the single pick relative to the cutting object to obtain test processing such as three-direction cutting force, which provides fundamental test support for the design of pick and the cutting system. The test equipment is connected to an octagonal ring dynamometer and a multichannel data acquisition system, which can monitor and collect the three-directional force in the cutting process in real time. The sampling frequency is up to 20 kHz. The pick can be rotated in the direction of the arrow and moved back and forth. The pick can be moved left and right. The rotation speed of the pick determines the cutting
The installation angle of the pick relative to the pick seat determines the cutting angle. The front and back movement displacement of pick determines the cutting line spacing. The left and right movement displacement of rock determines the cutting depth. The relative movement of the pick and the rock completes the cutting process.

To explore the applicability of the above-mentioned load identification method, this paper added a vibration acceleration test system to the single pick cutting test equipment. The system consists of KGS18 mine three-direction vibration acceleration sensor and YHZ18 mine vibration monitoring analyzer. The installation position of the test sensor is shown in Figure 2. The sensor layout method can refer to the reference (Yang et al. 2017). After collecting the vibration acceleration value, the three-direction force is obtained of the pick cutting by adopting the inverse pseudo excitation method and the modified regularization method, which is compared with the force measured by the test equipment force measurement system to verify the accuracy of the method.
3.2 Test object

The test object uses an alloy steel pick, whose material is 35CrMnSiA high-strength steel, the Vickers microhardness is 862 HV, the external elongation is 80 mm, the pick shank diameter is 38 mm, the pick tip diameter is 25 mm, the edge diameter is 60 mm and the pick tip angle is 80°. The cutting object uses special hard rock with a size of 1200 mm × 800 mm × 600 mm. The basic mechanical characteristics of the hard rock are obtained by performing uniaxial compressive strength tests and Brazilian tensile strength tests as shown in Table 1. The cutting test adopts orthogonal test method. The test variables are the cutting depth, the cutting line speed, the cutting angle and the cutting line spacing. The test conditions are shown in Table 2. Each test condition is repeated 3 times, the test temperature is 19 °C, the sampling frequency of the dynamometer is 500 Hz and the sampling frequency of the vibration acceleration collector is 10240 Hz.

| Rock type | γ (kg/m³) | UCS (MPa) | BTS (MPa) | E (GPa) | ν |
|-----------|-----------|-----------|-----------|---------|---|
| Sandstone | 2340      | 61.7      | 4         | 21      | 0.26 |

UCS = uniaxial compressive strength, BTS = Brazilian tensile strength, γ = density, E = Young’s modulus, ν = Poisson’s ratio

Table 2 Test conditions

| Test number | Cutting depth (mm) | cutting speed (m/s) | cutting angle (°) | cutting line spacing (mm) |
|-------------|--------------------|---------------------|------------------|--------------------------|
| 1           | 2                  | 1.5                 | 45               | 10                       |
| 2           | 2                  | 2                   | 49               | 20                       |
| 3           | 2                  | 2.5                 | 52               | 30                       |
| 4           | 3                  | 1.5                 | 49               | 30                       |
| 5           | 3                  | 2                   | 52               | 10                       |
| 6           | 3                  | 2.5                 | 45               | 20                       |
| 7           | 4                  | 1.5                 | 52               | 20                       |
| 8           | 4                  | 2                   | 45               | 30                       |
| 9           | 4                  | 2.5                 | 49               | 10                       |

4 Results and discussion

To validate the applicability of the load identification method, the data in this paper is not subject to data processing. Taking test condition 5 as an example, the time domain curve and frequency domain curve of the load measured by the dynamometer with the cutting time are shown in Figure 3. The vibration time domain signal and frequency domain curve of measuring point 1 under the same test condition are shown in Figure 4. The signal taken is perpendicular to the cutting plane direction. As can be seen from Figures 3(a) and 4(a), the load and vibration amplitude are violently fluctuating with cutting time, indicating that the pick rock breaking is a nonuniform stable process in the cutting process. This result is, on the one hand, due to the heterogeneity of the materials during the construction of the rock, on the other hand, due to the low power of the cutting motor used in the test. Therefore, the design of the cutting system should fully consider the impact of the peak load rather than the average load on the power of the cutting motor. As can be seen from the frequency domain responses in Figures 3(b) and 4(b), one of the characteristic frequencies of the cutting force is 3.62 Hz and that of the cutting vibration is 112.24 Hz. The cutting vibration...
frequency is 31 times the cutting force frequency, indicating that the characteristic value of the vibration signal contains the characteristic value of the cutting force, which is exactly an integer multiple of it. Therefore, the feasibility of the test can be determined and it further shows that the mechanical vibration and load have a certain correlation in the cutting process of a single pick.

Fig. 3 Time domain for a and frequency domain for b curves of cutting load

Fig. 4 Cutting vibration time domain for a and frequency domain for b curves
Figure 5 shows the variation trends of the load obtained by the dynamometer and the identified load by the method in this paper with the cutting depth, the cutting line speed, the cutting angle and the cutting line spacing. As can be seen from Figure 5(a), the loads violently increase nonlinearly with the increase of cutting depth. The influence of the cutting depth on the cutting system is greater than the other three factors. Therefore, when the cutting object is hard rock, the cutting depth should be as small as possible while considering the cutting efficiency.

Figure 5(b) shows the variation trends of the collected load and the identified load with the cutting line speed. As can be seen from the figure, the loads decrease slightly with the increase of cutting line speed, indicating that the inertial force plays a certain role in it. Figure 5(c) shows the variation trends of the loads with the cutting angle. As can be seen from the figure, the loads first decrease and then increase with the increase of the cutting angle. Therefore, there is an optimal cutting angle, which can produce a large rock breaking force. This is consistent with the conclusion of the reference (Yang et al. 2015). Figure 5(d) shows the variation trends of the loads with the cutting line spacing. The correlation coefficient between the loads and the cutting line spacing is small and no general law has been found. This may be due to the larger value of the cutting line spacing in the test and there is no coincident cutting area. Figure 6 shows the trace of the single pick cutting rock interface. It can be seen that the traces are selected as the cut line spacing is too large. The cutting track does not cross during the cutting process. However, in the actual tunneling or coal mining process, they will cause the relative change of the load because of the large number of picks and the different arrangement of the picks. Therefore, multiple pick cutting tests are needed to further determine the relationship between the cutting line spacing and the cutting load.

![Figure 5](image1.png)  
*Fig. 5 Comparison curves of real load and identified load for a) cutting depth, b) cutting speed, c) cutting angle and d) cutting line space*

As can also be seen from Figure 5, no matter what the parameters are, the change trend of the cutting load identified by the load identification method established above in this paper and the actual measured load is consistent. The maximum
error is 1.27 KN2/Hz and about 17.35%, which appears under the test condition of changing the cutting line speed. This may be due to the relatively independent relationship between load and speed, resulting in the vibration signal contains other unknown energy components. The frequency spectrum of vibration signal is processed under the test conditions, as shown in Figure. 4(b). In addition to the system’s natural characteristic frequency, it also includes a larger characteristic component and the energy value of this component increases with the increase of cutting line speed. The method described in this paper uses the amplitude of the frequency response function feature vector. Therefore, the increase of this energy component would have a certain impact on load identification (Zhang and Wang 2020; Fu et al. 2020; Maurya et al. 2020). In summary, within a certain range of motion speed, the load identification method in this paper has a high identification accuracy.

To compare with the load identification method before improvement, that is, compared with the regularization method which does not introduce a filter operator \( g(\lambda) \), the error quantization index is defined:

\[
\text{Error} = \frac{||S_1 - S_2||}{||S_2||} \times 100\% \tag{13}
\]

Where \( S_1 \) represents the root mean square of the amplitude of the identified load power spectrum; \( S_2 \) represents the root mean square of the real load power spectrum amplitude.

The identification error is shown in Table 3:

| Method           | Measuring point 1 | Measuring point 2 | Measuring point 3 |
|------------------|-------------------|-------------------|-------------------|
| Before improvement | 12.18             | 14.02             | 17.63             |
| After improvement | 10.20             | 13.11             | 17.35             |

The improved regularization method in all three measurement points can reduce the load identification error and the modified reduction error is up to 1.98%. It can also be seen from Table 3 that the farther the sensor is from the cutting surface, the greater the load identification error. The reason is that the components other than the cutting energy contained in the vibration signal have a greater impact on the load. Therefore, the problem of load identification can be meaningful if it is discussed within a certain range. Even though, the improved load identification method in this paper can modify the inverse distortion problem of the frequency response function around the natural frequency to a certain extent.

5 Conclusions

(1) A single pick cutting rock load identification method is presented. This method can identify the cutting load of a single pick according to the actual measured vibration and can use the inverse pseudo excitation method, singular value decomposition method and modified regularization method to modify the morbidity problem of frequency response function matrix. The test results show that the method has a better recognition effect on the cutting load of the pick and the recognition error is smaller than the traditional recognition method.

(2) With the increase of cutting depth, cutting line speed, cutting angle and cutting line spacing, the load of single pick cutting rock increases nonlinearly, slightly decreases, first increases and then decreases and changes irregular. It reflects the force trends of single pick cutting rock, which provide a certain research basis for the design of pick and cutting systems.

(3) The phenomenon of energy concentration in the frequency spectrum of the pick cutting vibration signal with the increase of
the speed should be further studied and analyzed, which provides another research topic for improving the accuracy of load identification.

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Author Contributions

Conceptualization, Lei Dong and Ruimin Shi; methodology, Bukang Wang and Junyuan Wang; validation, Siyu Zhai; formal analysis, Ruimin Shi and Lei Dong; investigation, Lei Dong; resources, Bukang Wang; data curation, Liang Dong; writing—original draft preparation, Lei Dong and Ruimin Shi; writing—review and editing, Lei Dong; visualization, Ruimin Shi; supervision, Ruimin Shi; project administration, Bukang Wang and Junyuan Wang; funding acquisition, Lei Dong and Ruimin Shi.

Conflicts of Interest

The authors declare that they have no competing interests.

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