Rock Properties and Modelled Stress State Uncertainties: A Study of Variability and Dependence

Moritz O. Ziegler

Received: 5 October 2021 / Accepted: 3 April 2022 / Published online: 12 May 2022
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Abstract
The safety and sustainability of subsurface applications requires a profound knowledge of the local stress state which is frequently assessed using 3D geomechanical-numerical models. Various factors lead to generally large uncertainties in these models. The variabilities in the rock properties as one of the sources of uncertainties and their influence on the modelled stress state is addressed herein. A generic 3D geomechanical-numerical model is used to investigate the influence of different distributions of variability and their effect on different stress states. The variability in rock properties clearly affects the uncertainties in the stress state in a positive correlation with differences that depend on the affected component of the stress tensor. The basic observation is that largest uncertainties are observed in the normal components of the stress tensor where the variabilities apparently are most effective. The same rock property variabilities affect the shear components uncertainties to a significantly lesser extent. Variabilities in the Young’s modulus and the Poisson’s ratio chiefly affect the uncertainties in $\sigma_{xx}$ and $\sigma_{yy}$. The density variability, however, leads to highest uncertainties in $\sigma_{zz}$. In general, variabilities in the Young’s modulus are most effective, followed by the Density and then the Poisson’s ratio. Furthermore, an influence of the tectonic stress regime on how the variability in the rock properties affects the stress state is observed. At the same time only a small effect is observed for different stress magnitudes. The eventual uncertainties in a modelled stress state depend not only on the uncertainties in the rock properties but also whether the uncertainties are found mainly in the Young’s modulus, the Poisson’s ratio or the Density. These findings indicate the importance to regard variabilities in rock properties as a source for significant uncertainties in geomechanical-numerical models. It is proposed to use the derived relations for an inexpensive quantification of uncertainties by means of a post-computation assignment of uncertainties to a stress model.

Highlights
- The influence of rock property variability on modelled stress state uncertainties is investigated using generic 3D geomechanical-numerical modelling.
- The variability in the Young’s modulus has the largest effect on the uncertainties in the stress state.
- The uncertainties in the normal components of the stress tensor are primarily affected by rock property variability.
- The tectonic stress regime and the absolute values of rock properties affect the results.

Keywords 3D geomechanical-numerical modeling · Uncertainties · Rock property variability · Stress model uncertainties · Rock properties

1 Introduction

Subsurface geotechnical applications require a reliable description of the 3D stress state to ensure safe and sustainable operations. This can prevent economic losses related to damages of the infrastructure or due to loss of support from stakeholders after the occurrence of induced seismicity.
Information on the stress state is provided by a wide range of stress indicators (Amadei and Stephansson 1997) that probe very different rock volumes (Ljunggren et al. 2003). The volume ranges from \(10^{-2} - 10^{3}\) m\(^3\) for borehole related stress indicator up to \(10^8\) m\(^3\) for fault slip data or earthquake focal mechanisms (Amadei and Stephansson 1997). However, stress measurement campaigns indicate that even within small rock volumes the stress state can be highly variable (Martin and Christiansson 1991; Cuisiat and Haimson 1992; Singha and Chatterjee 2015; Liu et al. 2020; Feng et al. 2020). Assuming that this is related to the variability of elastic rock properties, leads to a generally small representative elementary volume (REV), i.e. the volume in which the rock properties are assumed to be homogeneous (Amadei and Stephansson 1997).

To predict the stress state 3D geomechanical-numerical models are used (Henk 2009; Stephansson and Zang 2012; Singha and Chatterjee 2015; Liu et al. 2020). Such a model in its basic form requires a geological model with distinct rock units that are assigned appropriate rock properties (Van Wees et al. 2003; Ahlers et al. 2021; Andersen et al. 2022). Within the model volume the stress state variability is controlled by the overburden and differences in the lithology which are a result of a complex succession of rock units and/or heterogeneities in the rock properties of a single unit (Warpinski and Teufel 1991; Martin and Chandler 1993; Hergert et al. 2015). While the succession of rock units is addressed by accordingly designed geological models, the heterogeneities require information on the rock properties, their variability and the according REV which are divided in individual levels of heterogeneity, one for each rock property (Amadei and Stephansson 1997). This information is usually not available to a sufficient degree of detail since only a few—if any at all—rock property measurements are available (Leijon 1989; Amadei and Stephansson 1997). As a result, the modelled stress state is usually subject to significant uncertainties due to the unknown variability of rock properties (Hergert et al. 2015; Ziegler et al. 2016).

This can be addressed by an assignment of rock properties to REV based sub-volumes of a model instead of assignment of fixed rock properties for an entire unit. An established approach is to derive rock properties and their distribution from seismic observations of dynamic rock properties that are transferred to static ones (Bosch et al. 2010; Trudeng et al. 2014). This results in the desired heterogeneity in the rock properties. However, the sub-volumes sizes are either a result of the resolution of the seismic survey, the cells used during inversion, or the geomechanical models discretization. They are thus not true REVs based on the rock properties distribution. In addition to many assumptions during the conversion from dynamic to static rock properties which decreases significance (Mavko et al. 2009; Fjaer 2019), no uncertainties associated with the heterogeneities are quantified. Furthermore, often only a small number of rock property measurements are available, which inhibits both, assignment and a meaningful analysis. In order to compile any information on the uncertainties, unit wide changes in the rock properties are used as a proxy for measurement errors (Hergert et al. 2015; Ziegler et al. 2016). Even though this approach captures some uncertainties and indicates a rough estimation of the range in expected stress states, it is not an appropriate approach to assess the full uncertainties due to the missing of intra-unit heterogeneity. Thus, the current geomechanical-numerical modelling workflows disregard the full range of uncertainties in the stress state due to rock property variability. Any modelled stress state hence contains hidden uncertainties since neither the qualitative nor the quantitative influence of the rock properties variabilities on the stress state are known (Ziegler et al. 2016; Fraldi et al. 2019; Harper 2020). This challenge is frequently addressed by experiments backed by numerical models on the influence of heterogeneity on the failure process (Tang et al. 2000; Wong et al. 2006; Lei and Gao 2019) or by studying the heterogeneity of the stress state in local high-density stress measurement campaigns which can be seen, amongst others, as a result of heterogeneous rock properties (e.g. Martin and Chandler 1993; Wileveau et al. 2007; Schonenball and Davatzes 2017; Feng et al. 2020).

Herein, the perspective is reversed by starting with the rock property variability in order to investigate the impact of intra-unit variabilities of the rock properties on the six independent components of the modelled 3D stress tensor. A generic 3D geomechanical-numerical model with a single lithological unit is used. In order to simulate the variability of rock properties, they are individually assigned to sub-volumes that are assumed to be REVs. The variability is realised by definition of probability distributions for each rock property, herein the following three elastic properties: Young’s modulus, Poisson’s ratio, and Density. A normal distribution is assumed for all three rock properties' variability. The aim was to investigate the uncertainties in the stress state due to the variability in the rock properties. Therefore, sufficient models with different randomly assigned rock properties from the same distributions are computed in order to estimate the range of possible stress states. Thereby, the expected uncertainties in the six components of the modelled stress state are found. Furthermore, changes in the rock properties, their variability, the stress magnitudes, and the tectonic stress regime allow exploration of their respective influence on the resulting modelled
stress state uncertainties. The results show a first-order linear
dependence of the modelled stress state uncertainties on the
rock property variability. The rock stiffness (Young’s modu-
lus) is the key driver and the stress tensor’s normal compo-
nents have significantly higher uncertainties in comparison
to the shear components.

2 Method

2.1 General Concept and Model Setup

A 3D geomechanical-numerical finite element model to esti-
mate the influence of the variabilities in the following three
rock properties regarded in mechanical models: Young’s
modulus, Poisson’s ratio, and Density—on the modelled
stress state is used. The applied generic model with a single
lithological unit has an extent of 10 × 10 × 5 km³ and is
discretised with 3000 2nd order brick elements which for
numerical reasons are distinguished into 375 sub-volumes
(see “Appendix”) of which each is assumed to be a REV.
By assigning different and independent rock properties to
each sub-volume, their variability is realised. The respec-
tive properties are randomly drawn from normal distribu-
tions defined by a mean value and a standard deviation, i.e.
different and independent values for the Young’s modulus
(pictured in Fig. 1), the Poisson’s ratio, and the density are
assigned. The now realised distribution of rock properties
throughout the model volume is only one possibility out of
many different scenarios. As such, the assigned rock proper-
ties throughout an individual model scenario do not perfectly
follow the according distribution. Instead, the full distribu-
tion is realised by generation of various model scenarios (see
Sect. 2.3). The model describes the equilibrium of volume
(gravity) and surface forces (tectonic stress). The latter is
imposed by displacement boundary conditions which are
varied to find the best-fit (Reiter and Heidbach 2014) to a
synthetic stress state at a single point at a given depth in the
model volume (Fig. 1 and Sect. 2.3).

Fig. 1 Spatial distribution of rock properties (here exemplified by the
colour coded Young’s modulus) in a classic geomechanical model
with a single unit and accordingly only one set of rock properties
(left) and the presented approach (right). The star signifies a location
at 2900 m depth where knowledge on the 3D stress state is assumed
(see Sect. 2.3). This sub-volume has fixed rock properties and is
excluded from the random assignment of rock properties

2.2 Distribution of Rock Properties

The variability experienced in the properties of the same
lithology has different origins. Partly it is due to inherent
inhomogeneities in the rock (Martin and Chandler 1993;
Sone and Zoback 2013; Zhang et al. 2020) and partly due
to uncertainties that stem from deficiencies in our ability to
assess the rock properties at all and in particular throughout
the entire volume of a rock unit (Amadei and Stephansson
(1997) and references therein). The three mechanically rel-
evant rock properties that are discussed herein—in particular
the Young’s modulus—implicitly contains variability due
to additional rock properties that are not relevant for this
approach of mechanical modelling, e.g. porosity or rock
strength. Thus, the approach is not limited to a certain res-
ervoir type but universally applicable.

In the following, the five main reasons for variability in
rock properties are indicated. (1) Systematic lateral changes
in the lithology due to fluent transitions may remain unde-
tected due to limited measurements. In addition, 3D geome-
chanical-numerical modelling usually simplifies a variable
lithology by a single set of rock properties. (2) A certain
random heterogeneity in the lithology is expected in most
cases which adds noise to the rock properties. (3) If a rock
property measurement is available, e.g. from a core sample,
it is unclear whether the core is representative for a signifi-
cant part of the lithological unit or if it is an exceptional out-
lier. The variability then has to be estimated from the range
of values found in various samples from different locations.
If only one sample is available, expert elicitation comes
into play to assign a likely variability which increases the
uncertainties. (4) If no rock property measurements of the
considered lithology are available, often reference datasets
of the same rock type but from a different location are used.
This bears the possibility for significant deviations which
have to be met by the assignment of a significant variability.
(5) Eventually, measurement errors and uncertainties due
to necessary assumptions are added to the results of labora-
tory tests.
The three elastic properties of rocks that are discussed herein—in particular the Young’s modulus—showcase a great variability that depends, amongst others, on the rock type, location, weathering, or burial history (Phoon and Kulhawy 1999; Cai 2011; Zhang et al. 2020). A possible correlation of the rock properties with or a dependency on each other is disregarded herein. No universally applicable correlation or dependency is known. As default average values used to define the center of the distribution, values are chosen that represent average rocks—a Young’s modulus of 50 GPa, a Poisson’s ratio of 0.28, and a density of 2700 kg/m³ (Aladejare and Wang 2016; Bär et al. 2020).

Rock properties and their variability are studied with regards to several issues. These range from literature studies that compile a very broad view on different rock types—igneous, sedimentary, and metamorphic—and their variability (Aladejare and Wang 2016; Bär et al. 2020) to detailed local studies that explore the rock property variability in an individual location and lithology (Pepe et al. 2016; Zhang et al. 2020). These studies results led to the assignment of the variabilities to the rock properties. In contrast to other studies, where a Weibull type distribution is found for the rock strength (Lobo-Guerrero and Vallejo 2006; Wong et al. 2006) herein, a normal distribution is applied, where a variability of 1σ includes roughly 68% of the rock properties values. This is due to the comprehensive regard for reasons for variability addressed in this study which involves measurement errors. Furthermore, in most field applications the few data records on rock properties disallow a reliable estimation of correlation or dependency is known. As default average values used to define the center of the distribution, values are chosen that represent average rocks—a Young’s modulus of 50 GPa, a Poisson’s ratio of 0.28, and a density of 2700 kg/m³ (Aladejare and Wang 2016; Bär et al. 2020).

2.3 Model Calibration and Uncertainty Estimation Due to Rock Property Variability

The presented generic model features an assumption on the stress state which is realistic according to various stress measurement campaigns and data compilations (Brudy et al. 1997; Hickman and Zoback 2004; Morawietz et al. 2020). In a depth of 2900 m a strike slip stress regime with \( \sigma_{xx} = 100 \text{ MPa}, \sigma_{yy} = 48 \text{ MPa}, \) and \( \sigma_{zz} = 77 \text{ MPa} \) is assumed. By varying the displacement boundary conditions the model is calibrated at a single sub-volume at that depth (Fig. 1, Reiter and Heidbach 2014). The rock properties in this sub-volume used for calibration are fixed to the mean values of rock properties as stated in Sect. 2.2. This model scenario with a spatial variability in rock properties provides a single quantification of the resulting spatial stress variability. However, the rock properties are assigned to the elements in an arbitrary way. Thus, this model scenario is only one possibility amongst many (Fig. 2a, b). No exhaustive information on the range of other possibilities, i.e. the uncertainties of the stress state at a given location due to the variabilities in the rock properties is provided yet.

An estimation of uncertainties requires a full scan of all possible distributions of the rock properties variability. This can be achieved by numerous model scenarios with the same model geometry and the same calibration point, but different rock properties from the same probability distribution. It is automatized using Python scripts that automatically assign the rock properties (Ziegler 2021), calibrate the desired stress state (Ziegler and Heidbach 2021), and evaluate the effect of the randomly assigned variability. Once a sufficient amount of different model scenarios have been evaluated the script is halted and the uncertainties of all components of the stress tensor are displayed.

In order to estimate the sufficient amount of model scenarios, a break condition is defined. It is tied to the tapering off of changes in the stress states standard deviation from all model scenarios that have been computed up to this point. The criterion is tested after each model scenario run by an estimation of the standard deviation of each stress tensor component in this and all previous model scenario runs by

\[
\tilde{\sigma}_{ij,n} = \sqrt{\frac{1}{n-1} \sum_{k=1}^{n} (\sigma_{ij,k} - \overline{\sigma}_{ij,k})^2}
\]

with the average stress

\[
\overline{\sigma}_{ij,n} = \frac{1}{n} \sum_{k=1}^{n} \sigma_{ij,k},
\]

where \( n \) is the number of model runs that have been computed. If the changes in the standard deviations compared to the ten previous condition checks \( n - 1, n - 2, \ldots n - 10 \) are \(< 0.01 \text{ MPa} \) for each of the six independent components of the stress tensor, the break condition is met and the script is halted.

It is then assumed that even with additional model scenarios no significant changes in the stress states standard deviation or in the average stress state will occur due to the randomly assigned rock properties. This implicitly assumes that the parameter space has been scanned sufficiently, i.e. the rock properties have been evenly distributed throughout the model volume in the sum of the model scenarios. The uncertainties in the stress state due to the rock properties variability can now be quantified. In order to prevent a premature break of the script this procedure is not limited to one location but repeated at 150 locations throughout the model volume.
2.4 Analysis Procedure

The result of the model workflow and analysis is the standard deviations of the six independent stress tensor components throughout the model volume. They are estimated from all model scenarios once the break criterion of the script was met. The rock properties for each element of the models only lithological unit are drawn from the same distribution of rock properties and thus a smooth average stress gradient is achieved (Fig. 2c) and the standard deviation is comparable throughout the model volume (Fig. 2d). The only deviation is observed at the location used for calibration of the model, where the rock properties are fixed on their average value in all model scenarios (Fig. 2d).

In order to test different probability distributions of rock properties, the described procedure is repeated in a different batch of model runs. Within each batch the calibrated stress state and the probability distributions of the rock properties are the same. The only variation between model scenarios is the randomly performed assignment of rock properties based on the specified probability distributions. Different batches, however, allow to change the calibrated stress state and the probability distributions of the rock properties in order to investigate their effect on the uncertainties of the stress state.

The bottom row of Fig. 2 only shows the resulting average stress state (Fig. 2c) and the associated uncertainties in the stress field (standard deviation, Fig. 2d) due to one set of rock properties and their variability of $1\sigma = 30\%$ in all three rock properties. Also, only two components of the stress tensor ($\sigma_{xx}$ and $\sigma_{zz}$) are visualized. However, the aim is to characterize all six components of the stress tensor and their uncertainties due to differently large variability in the rock properties. Thus, the observed standard deviations from within each sub-volume are presented in a histogram (Fig. 3a) for different batches with different variabilities of the rock properties. To allow for a better comparability, the same information is also displayed in as boxplot (Fig. 3b) which will be exclusively used in the later figures.

3 Results

In order to investigate the influence of the variability in the rock properties on the stress states uncertainties, different batches with differently large variabilities in the three rock properties in only one of them are tested while the others are fixed. Furthermore, the influence of different stress regime (i.e., changes of the stress magnitude ratios in the sub-volume with the a priori known stress state and fixed rock properties), and the influence of different rock types—different mean rock properties—on the uncertainties of the stress field are investigated.
3.1 Influence of the Three Rock Properties

The resulting uncertainties in Fig. 3 are due to an amalgamation of the effects of the variabilities in the three rock properties Young’s modulus, Poisson’s ratio, and Density. To investigate the effect of the variability in each individual rock property, only one of the rock properties is assigned a variability while the others are fixed on their mean value. The results are shown in the three panels of Fig. 4, each panel for variabilities in one rock property.

As already indicated in Fig. 3, mainly affected by the variability in rock properties are the normal components of the stress tensor and in particular $\sigma_{xx}$ and $\sigma_{yy}$ (Figs. 3 and 4). The separate investigation of variabilities in only one of the

Fig. 3 a Colour-coded standard deviations of the modelled stress states components (x-axis) throughout the model volume and its variability (y-axis) as a histogram. Different variabilities between 10% (back) and 50% (front) of all three rock properties is assumed. b The same data is displayed with the variability (x-axis) and the standard deviation in the modelled stress state (y-axis) in a box plot. The bold line indicates the range of 50% of the data which contains the median (dot). The thin line represents the whiskers, the range of values that are no outliers but outside the interquartile range. The maximum length of the whiskers is the 150% of the interquartile range. Individual outlier data points are indicated by a circle

Fig. 4 Effect of variabilities in rock properties on the six components of the stress tensor. The three panels indicate the effect if only one rock property is subject to variabilities and the others are fixed at their mean. The x-axes shows the variability in the rock property in terms of $1\sigma$ in percent. The y-axes show the standard deviation of the modelled stress component. Each average and standard deviation, i.e. each data point represents a model setup like the one displayed in 3D in Fig. 2. Please note the different scales of the y-axes. For explanation of the boxplot refer to Fig. 3
three rock properties confirms that the uncertainties in the shear components $\sigma_{xy}$, $\sigma_{yz}$, and $\sigma_{zx}$ are generally small. The absolute magnitude of shear stresses in this model setup are expected to be small. However, only the variability of the standard deviations are interpreted which reduces the impact of absolute stress magnitudes. Nevertheless, a potential bias in the variability of the shear components due to the small absolute stress magnitudes is investigated in Sect. 3.2.

It is indicated in Fig. 4a that the main contributor to uncertainties in the stress state is the Young’s modulus. The uncertainties are mainly focused on the $\sigma_{xx}$ and $\sigma_{yy}$ components of the stress tensor. The $\sigma_{zz}$ component is next with a significantly higher uncertainty than the shear components but also clearly below the other two normal components. Conversely, variabilities in the density mainly affect the $\sigma_{xx}$ component of the stress tensor and only secondarily $\sigma_{yx}$ and $\sigma_{yz}$ components with the shear components again clearly negligible (Fig. 4b). In the upper kilometres, this can be easily explained by the overburden as the source for $\sigma_{zz}$ which depends on the density. The influence of variabilities in the Poisson’s ratio is only significant for the $\sigma_{xx}$ and $\sigma_{yy}$ components while all other components including $\sigma_{zz}$ are negligible (Fig. 4c). In all three panels of Fig. 4 the one sub-volume that is used for calibration and has accordingly no variability is clearly indicated by single outliers in $\sigma_{xx}$ and $\sigma_{yy}$.

For variabilities (up to 40% to 50%) the relationship between the variability in the rock properties and the uncertainties in the stress state follow a linear trend. For larger variabilities, this linear trend is flattening (Fig. 4). In particular for the Poisson’s ratio (Fig. 4c) this is clearly visible. This bend in the curve is caused by the absence of negative rock properties. With increasing variability, the drawing of negative values from the distribution becomes exceedingly probable (Fig. 5a). To prevent this, in such cases the negative value is discarded and a new value is drawn until a positive value is found (Ziegler 2021). This imposes a biased distribution of assigned rock properties. The somewhat more pronounced bend in the Poisson’s ratio curve (Fig. 4c) is due to the double limit of the Poisson’s ratio. Neither negative values nor values larger than 0.5 are allowed (Fig. 5b).

### 3.2 Influence of the Stress Magnitudes

The stress state itself is variable in terms of the absolute magnitudes and the differential stresses. It is investigated whether stress magnitudes affect the stress states perceptibility to rock property variability in a way that the same rock property variability results in different uncertainties of the modelled stress state. This influence is investigated by comparing the uncertainties in the three depths of $-600$ m, $-2300$ m, and $-4000$ m with accordingly different stress magnitudes while maintaining the same rock property variability.

Most prominently it is observed that uncertainties in $\sigma_{zz}$ are positively correlated with an increasing stress magnitude (Fig. 6). However, this is not uniquely due to the larger stress magnitude but rather due to the increase in the overburdens variability. The range of possible overburden increases with depth due to the variability in the density which adds up with every overlying sub-volume. Additional differences in uncertainties due to different stress magnitudes are observed but have only a limited significance compared to the three different rock properties themselves. The dominance of the uncertainties in $\sigma_{xx}$ over $\sigma_{yy}$ is established and becomes more pronounced with an increase in stress magnitude/depth.

Even though the absolute uncertainties in the shear stresses are negligible, it is interesting to note that $\sigma_{yy}$ is positively correlated with the absolute stress magnitudes/depth while $\sigma_{yz}$ and $\sigma_{zx}$ are negatively correlated. Still, the observed differences in how the the shear components are affected in different depths (i.e. by different absolute stress magnitudes) is small. This is in particular small in comparison to the normal components variabilities and also compared to how $\sigma_{zz}$ is affected. This indicates that the uncertainties in the shear components are not biased by their small absolute magnitudes.

### 3.3 Influence of the Stress Regime

In addition to the previously addressed different stress magnitudes, the orientation of the principal stress components is important for characterization of the stress state and define the three tectonic stress regimes normal, strike slip, and thrust faulting (Anderson 1905). In order to test the influence of the tectonic stress regime, identical principal stress magnitudes for model calibration are tested. The previous strike slip ($\sigma_{xx} = 100$ MPa, $\sigma_{yy} = 48$ MPa, $\sigma_{zz} = 77$ MPa) stress regime is changed to represent either a normal faulting ($\sigma_{xx} = 77$ MPa, $\sigma_{yy} = 48$ MPa, $\sigma_{zz} = 100$ MPa) or thrust faulting ($\sigma_{xx} = 100$ MPa, $\sigma_{yy} = 77$ MPa, $\sigma_{zz} = 48$ MPa) stress regime. This change in regime is achieved by altering the depth of

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**Fig. 5** Two probability density function for a normal distribution with $1\sigma = 30\%$ and $1\sigma = 60\%$ in relation to values that the rock properties are not allowed to take on (grey background). The Young’s modulus (left) may not take on negative values. The same holds for the density which is not pictured here. The Poisson’s ratio (right) may not take on negative values or values larger than 0.5.
the calibration point which changes the magnitude of $\sigma_{zz}$. Furthermore, boundary conditions are adapted to realise according magnitudes of $\sigma_{xx}$ and $\sigma_{yy}$ (Ziegler and Heidbach 2021). The analysis of the results is focussed on the respective depths in order to assure the comparability of all stress magnitudes (including $\sigma_{zz}$) and to exclude any influence of the stress magnitudes.

Figure 7 shows the results in a 3-by-3 matrix that juxtaposes the three different rock properties with the three stress regimes. The uncertainties in the modelled stress state due to variabilities in the Young’s modulus show a clear dependency on the stress regime (Fig. 7, left column). The differences are chiefly concentrated on the horizontal normal components $\sigma_{xx}$ and $\sigma_{yy}$ which also have the largest uncertainties in general. In a normal faulting stress regime uncertainties in $\sigma_{yy}$ are primarily affected by the Young’s modulus variability. Concurrently, in a thrust faulting regime uncertainties in $\sigma_{xx}$ are primarily affected by the Young’s modulus variability.

Variabilities in the density have no significantly different effect on the uncertainties in the modelled stress state in the different tectonic stress regimes (Fig. 7, central column). Despite a slightly larger uncertainty in $\sigma_{zz}$ in normal faulting stress regimes, followed by strike slip and finally thrust faulting no changes are observed. These deviations are probably due to the decrease in absolute $\sigma_{zz}$ magnitude from normal via strike slip to thrust faulting but are so small that they are not significant.

Variability in the Poisson’s ratios affects the uncertainties in the modelled stress state in a way that is clearly dependent on the tectonic stress regime (Fig. 7, right column). Contrary to the behaviour of the Young’s modulus, in a normal faulting regime the uncertainties in $\sigma_{xx}$ are primarily affected and in a thrust faulting regime uncertainties in $\sigma_{yy}$. Both stress tensor components are clearly less and almost equally affected by variabilities in the Poisson’s ratio in a strike slip regime.

### 3.4 Absolute Rock Properties

Initially, this study aims at representation of average rock types with regards to their mean properties. However, they are by no means representative for the entire diversity of existing rocks. Thus, the influence of the mean rock property value on the uncertainties in the stress state is evaluated, i.e. different variabilities around different mean rock properties are tested. Young’s moduli between 10 and 90 GPa (in steps of 20 GPa), Densities between 2100 and 3300 kg/m$^3$ (in steps of 300 kg/m$^3$), and Poisson’s ratios between 0.20 and 0.36 (in steps of 0.04) are evaluated. Previously, the variability was altered based on percentage of the mean value. However, with changes in the mean value the definition of
variabilities becomes ambiguous. Thus, each of the properties means are assigned (1) an absolute variability of 15 GPa (Young’s modulus), 810 kg/m³ (Density), or 0.084 (Poisson’s ratio) or (2) a relative variability of 30% to each rock property, e.g., 3 GPa for a Young’s modulus of 10 GPa or 27 GPa for a Young’s modulus of 90 GPa.

Even though the different mean rock properties increases or decreases the resulting modelled uncertainties, Fig. 8 illustrates that in contrast to changes in the tectonic stress regime, no relative changes between the uncertainties in the different components of the stress tensor occur. Considering variabilities in the Young’s modulus, the modelled uncertainties in σₓₓ are always larger than those in σᵧᵧ, no matter the absolute rock properties values.

For a fixed variability (Fig. 8 top row) of the Young’s modulus (± 15 GPa) and the Poisson’s ratio (± 0.084) higher mean rock property result in a decrease of the uncertainties (Fig. 8a, c). In particular in case of the Young’s modulus this decrease is dramatic while for the Poisson’s ratio it is almost negligible. Independent of the average rock property, no changes are observed in case of the density (Fig. 8b). Apparently, the effect of the increase in average rock property is mitigated by the relatively decreasing variability which is constant at 810 kg/m³.

If a variability of ± 30% relative to the average rock property is assigned to the Young’s modulus, changes in the absolute value of the rock property do not affect the uncertainties in the stress state (Fig. 8d). This is due to the

Fig. 7 Influence of the tectonic stress regime on the effect of variabilities in the rock properties on the components of the stress tensor. A normal faulting (top row), strike slip (middle), and thrust faulting (bottom row) regime are displayed. Axes are as in Fig. 4. Please note the different scales of the y-axes. For explanation of the boxplot refer to Fig. 3
The variabilities increase together with the average value of the rock property. The relative value of 1σ remains identical and is thus reflected in the stress state uncertainties that remains constant. Conversely, for an increase in the density an increase in the stress state uncertainties is observed with a relative variability of 30% (Fig. 8e). The same is observed for the Poisson’s ratio (Fig. 8f).

The Young’s modulus variabilities influence on the uncertainties in the modelled stress state is in agreement with the intuitive expectations. The densities variabilities influence on the uncertainties of the modelled stress state, however, behaves counterintuitive. This is because the density is affected by an increase in absolute variability with depth, i.e. the range of possible total overburden increases (see Sect. 3.2). This increase in overburden is
usually non-linear even without considering heterogeneities due to different rock units different densities (Warpinski and Teufel 1991). The overburden is responsible for the vertical normal stress according to

\[ \sigma_{zz} = \int_0^z \rho g dz \]  

(3)

with the density \( \rho \), the depth \( z \), and the gravitational acceleration \( g \). This formula indicates the possibility to estimate the uncertainties solely introduced by the density on the modelled stress state by an analytical approach as well. Furthermore, the other components of the stress state are affected by a variability in the density via the Poisson’s ratio \( \nu \) according to

\[ \sigma_{lm} = \sigma_{ij}(1 + \nu) - \nu \delta_{ij} \sigma_{kk} \]  

(4)

with the affected stress tensor \( \sigma_{lm} \), the unaffected stress tensor \( \sigma_{ij} \), and \( \delta_{ij} \) as the Kronecker delta. This explains the contrasting behaviour compared to the Young’s modulus in all components of the stress tensor.

The Poisson’s ratio behaves peculiar in that the average values are negatively correlated with the uncertainties in the stress field when assuming a constant variability and positively correlated when assuming a relative variability. Even though the Poisson’s ratio itself is not depth-dependent, the relationship with the overburden, described in the previous equations, makes the impact of its effects depth-dependent (as the density in Fig. 8e). Concurrently, the non-depth-dependent ties to the Young’s modulus lead to the similar behaviour (Fig. 8d).

4 Discussion

4.1 General Discussion

Numerical modelling of the in-situ stress state is commonly performed for engineering, exploratory, or seismic hazard assessment purposes by means of 3D geomechanical-numerical models (Goertz-Allmann and Wiemer 2013; Ziegler et al. 2015; Yoon et al. 2015; Jeanne et al. 2016; Blöcher et al. 2018). The uncertainties of such models are indicated to be large (Hergert et al. 2015; Ziegler et al. 2016) and recently approaches and methods to quantify these uncertainties became available (Lecampion and Lei 2010; Ziegler and Heidbach 2020). However, these approaches usually concern the uncertainties due to stress magnitude data records. Thus, the influence of uncertainties in the 3D geological model (Wellmann et al. 2010; Bond et al. 2015) is disregarded by assumption of perfectly known strata. In addition, perfectly known (no measurement error) and homogeneous (one set of rock properties for an entire unit) rock properties are assumed. However, rock properties are one of the key drivers of uncertainties (Hergert et al. 2015; Ziegler et al. 2016). Their influence on the stress field is indicated by heterogeneities found by extensive stress estimation campaigns (Warpinski and Teufel 1991; Zhao et al. 2015; Feng et al. 2020) and theoretically by means of modelling (Roche and van der Baan 2017; Lei and Gao 2019; Fraldi et al. 2019).

Herein, only the influence of rock property variability on the modelled stress state is regarded and quantified. This is done by means of a one-unit generic hexahedron model that does not include any geological boundaries, faults, or other structures that are known to somehow affect the stress state. A more complex geometry is likely to have an additional impact on the variability of the stress state due to rock properties. Furthermore, for simplicity, the expected uncertainties due to measurement errors in the stress data records are disregarded by assuming a perfectly known stress state for calibration. Furthermore it is assumed that the rock properties in the REV used for stress state calibration are perfectly known. The assessment of the influence of aforementioned additional drivers for aleatoric uncertainties as well as any epistemic uncertainties are outside the scope of this work. Nevertheless, they should be addressed in future research.

This study is focused on heterogeneous rock properties and their influence on mechanical models of the in-situ stress state. Such models only require definition of the three regarded rock properties Young’s modulus, Poisson’s ratio, and Density. The properties are drawn from representative distributions which do not include all possible outliers of rock types with extreme properties. Such materials should be investigated separately which may even require a different rheological behaviour. The chosen properties are assumed to be a representation of the rock mass behaviour within a model element. It is implicitly assumed that the heterogeneity in the three properties is also caused by variabilities in additional parameters such as rock strength or porosity. With respect to the element size significant disturbances of the rock (e.g. wide faults, natural caves, mined out areas) should be discretized and not parameterised. Kinematic modelling approaches (Hergert et al. 2011) or fracture propagation models (Yoon et al. 2015) will face additional variabilities that need to be explicitly regarded. This also holds for Hydro-Mechanical or Thermo-Hydro-Mechanical models which require a range of additional property information such as permeability or thermal conductivity (Altmann et al. 2010; Ziegler et al. 2017).
4.2 Impact of Two-layer Geometries with Different Rock Property Variability

The results are scale invariant due to the linearity of the problem formulation which allows a broader application to real world settings. However, contrary to the previously presented generic approach, usually stress models contain more than just one lithological unit. Different units accordingly have different rock properties and different associated variabilities. The influence of the rock properties variability in one unit on the modelled stress states uncertainties in another unit is tested. A two-unit setting is investigated in an advanced approach which has an extent of 10 × 10 × 5 km³ (Fig. 9a) and is discretised with 3000 elements that are assigned to 375 sub-volumes. At a depth of 1500 m a horizontal boundary separates the sedimentary Top unit from the crystalline Bottom unit. For each of the two units, the rock properties for each individual element are drawn from the according probability distributions around the average rock properties (Fig. 9a). An estimation of the uncertainties in the stress state due to the initially specified variability is displayed in Fig. 9b which indicates the smaller uncertainties in the stress state of the upper unit due to the smaller variability in the rock properties.

In order to further test the behaviour, the variability in the upper unit is varied while the one in the bottom unit is

![Fig. 9](image_url)

Fig. 9 Generic two-unit-situation. a The rock properties for the individual elements are drawn from two different distributions indicated in the table, depending on the assigned lithology. The stress data record for model calibration (black star) is situated in an element with fixed rock properties. b Standard deviation in the $\sigma_{xx}$ (vertical profile) and $\sigma_{zz}$ (horizontal plane) components of the stress tensor. The area where stress data records are available and the rock properties are fixed is visibly indicated by the slightly darker area of reduced standard deviation in the upper unit.

![Fig. 10](image_url)

Fig. 10 Effect of the reduction in variabilities on the components of the stress tensor. In a two unit setting only the variabilities in the rock properties of the upper unit are reduced. The expected effects on the components of the stress tensor are shown in the upper part of the figure. The effect of this reduction on the second unit stress state, which still has the same variabilities in the rock properties, is shown in the lower part of the figure. For explanation of the boxplot refer to Fig. 3.
fixed. It is assumed that the rock properties variabilities in
the Bottom unit are \( 1\sigma = 30\% \) while variabilities between
\( 1\sigma = 10\% \) and \( 1\sigma = 40\% \) in the Top unit are used. The
resulting uncertainties in the modelled stress state of the
Top are shown in the upper part of Fig. 10 (marked Top).
Concurrently, the uncertainties in the modelled stress state in
the Bottom are not constant, even though the corresponding
rock properties variability are not changed at all (lower part
of Fig. 10). A positive correlation with the rocks variability
of the Top leads to smaller uncertainties in the stress state if
the Top units’ variability in rock properties is smaller. These
results indicate if more and well known units are present in
the model instead of an increase in uncertainties, rather a
decrease of uncertainties is expected.

This effect that even a reduction of the rock properties’
variability only in one unit leads to a reduction of the stress
states uncertainties in all units is beneficial for geomechanical
modelling that aims at predicting the in-situ stress state.
Often the rock properties knowledge for in-situ stress models
is limited to a restricted area of interest. Well-known rock
properties in this area hence potentially positively affect the
uncertainties in the entire model. In addition, a decrease in
variability in a certain unit, e.g. due to an extensive measure-
ment campaign that reduces the measurement error, is far
more feasible than a decrease of uncertainties in all present
units. In particular, this holds for shallow units which are
feasibly accessed from an economic and technical point of
view.

### 4.3 Applications

This study is designed to address the effects of rock prop-
erty uncertainties on the modelled 3D stress state in order
to enable quantification and reduction of model uncertain-
ties and improve interpretation of modelled stress tensors.
A case study and application of the findings on a particular
engineering project should be subject of future research.
This requires detailed data on the according rock properties
distributions. The intuitive approach is to apply the herein
presented method to a full-scale 3D geomechanical-numerical
model of a particular project. However, for large models
this is currently not possible due to the required computation
time that exceed any feasible amount of time by orders of
magnitude. Thus, such an exhaustive approach to quantify
the uncertainties is not an option.

Instead, it is proposed to establish an ad-hoc quantifica-
tion of uncertainties. The relationship between rock property
variabilities and the modelled stress state are investigated
using generic models such as the presented. The results are
used to generate lookup tables that contain the uncertainties
in the modelled stress state due to different variability distrib-
butions in the rock properties in different settings. Therefore,
sufficient combinations of rock properties and variabilities
need to be assessed to set up a significant lookup table. The
table is then used for post-computation assignment of uncer-
tainties to the modelled stress state.

The presented results indicate important factors that influ-
e a lookup table. The rock properties and their variability,
the stress magnitudes and the tectonic stress regime have an
impact on the models perceptibility to rock property vari-
ability. Accordingly, they need to be addressed in a lookup
table. Further research is required to investigate whether
there is a significant impact of the geometry, e.g. pinch-
ing or cropping out units, salt domes. In particular complex
geometries other than horizontal layering may have an effect
but an investigation is beyond the scope of this work. Fur-
thermore, the applicability and limits of such a lookup table
approach need to be addressed in future research.

Any application needs to address the possibility that
site-specific or rock-specific correlation and/or dependen-
cies between the rock properties exist. Their investigation
requires robust information on rock properties from compre-
hsensive measurement campaigns or databases such as Bär
et al. (2020). If a significant correlation or dependency can
be observed, the variabilities in all three rock properties can
be reduced, i.e. the variability in the modelled stress state
due to all three rock properties (Fig. 3) are reduced while the
variability in the modelled stress state due to each individual
rock property remains the same (Fig. 4). However, in terms
of correlation or dependencies, an approach that does not
consider them cannot underestimate the variability in the
stress state. Any correlation or dependency would necessar-
ily reduce the variability in the modelled stress state.

### 5 Conclusion

The presented generic model workflow quantifies systemati-
cally the impact of rock property variability on the uncer-
tainties of the six independent components of the 3D stress
tensor. In particular the model result for the one-layer prob-
lem show the following:

- The variability in the Young’s modulus has the largest
effect on the uncertainties, followed by the density and
the Poisson’s ratio.
- The normal components are significantly stronger
affected by a factor of 2–6 in comparison to the shear
components.
- The tectonic stress regime changes the results, but the
general findings are still valid.
- These general findings are independent from the num-
ber of layers that are implemented. If the variability of
rock properties in only one unit is reduced, a reduction
of uncertainties is also observed in the other units stress
state.
However, the application of this approach to real settings is in most cases not feasible due to extraordinary long computation time required. To mitigate this while also enabling a significant assessment of associated uncertainties, it is proposed to use the previously found relations between the variability in rock properties and the modelled stress states’ uncertainties for post-computation assignment of uncertainties. This lookup table approach will significantly reduce the cost but further research is required to evaluate the applicability, provide rules for implementation, and generate accordingly refined lookup tables.

Appendix: Numerical Implementation

Using the finite element method requires continuity of the field variable at the nodes of the elements. The field variable in this case is the displacement since the problem of the equilibrium of forces is a second order partial differential equation formulated in displacements. The stress tensor is estimated at the integration points that are located at pre-defined positions within the elements. However, to visualize the stress state the values are extrapolated from the integration points to the nodes and thus, the stresses at the nodes are a mean of the stresses estimated at the nodes that are shared by several elements. Thus, if elements that share a node have different rock properties the effect of these is reduced. This effect is stronger in 1st order finite elements where the stress is constant within the elements and significantly lower in 2nd order elements since stress varies linearly in these.

The generic model described herein is constructed entirely of brick elements and thus the following description refers to elements of the according geometry. For 8-node-elements with linear basis functions (1st order elements) at each node seven other elements border (Fig. 11a). In case of 20-node elements with quadratic basis functions (2nd order elements) there are between three and seven adjacent elements (Fig. 11b). High contrasts in the rock properties between neighbouring elements are thus smoothed in case of using 1st and 2nd order elements.

To mitigate this smoothing effect, the assignment of rock properties is not done element-wise but to groups of eight elements each (Fig. 11c). Such a group features seven nodes whose stress values are independent from other element groups with different rock properties. In each direction in space three of these consecutive independent integration points are available (Fig. 11c). At these nodes the full impact of variabilities in the rock properties can be observed.

Fig. 11 Number of adjacent elements for nodes of different element configurations (colour-coded). a A 1st order element and b a 2nd order element do not have any nodes completely independent from other elements. c Eight 2nd order elements grouped together have 7 nodes that are independent from other element groups. d The modelled standard deviation of the stress state on a vertical section of the same model in two different visualization approaches. Left: The standard deviation observed at all integration points is interpolated to the elements volume. The mesh with all elements is pictured. Right: Only the standard deviation of the stress state at the central nodes of 8-element-blocks are pictured. Each cell represents such an 8-elements-block.
Hence, only the stress state from these central nodes is used for displaying the results. At the borders to other elements—boxes a smoothed stress state that does not represent the actually expected uncertainties is observed (Fig. 11d) due to the amalgamation of effective rock properties.

Acknowledgements The author thanks two anonymous reviewers whose comments improved the manuscript. The work leading to these results has received funding from the Initiative and Networking Fund of the Helmholtz Association through the project “Integrity of nuclear waste repository systems - Cross-scale system understanding and analysis (iCross)” project by the Federal Ministry of Education and Research (project number 02NUK053D), the Helmholtz Association (project number SO-093) and the Helmholtz Centre Potsdam - Deutsches GeoForschungsZentrum GFZ.

Funding Open Access funding enabled and organized by Projekt DEAL.

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