Study on the optimal position of the roof low roadway based on the response surface methodology

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For determine the optimum position of the roof low roadway, the optimal solution is derived according to the response surface methodology. The UDEC numerical simulation of the overburden gives the porosity distribution of the strike fractured zone, the upper limit heights of the caving zone and the fractured zone are obtained as 18 m and 65 m, respectively. Based on the porosity distribution, the FLUENT numerical models of the goaf zone, air inlet roadway, air return roadway, working face and roof low roadway were established to simulate the gas concentration in the upper corner and gas drainage volume in roof low roadway during mining. Using the vertical and horizontal distance of the roof low roadway as the influencing factors, the experimental scheme of the position of the roof low roadway was designed according to the response surface method, and the response values were obtained from the FLUENT simulation experiments, predicting that the vertical and horizontal distances of the roof low roadway were 7.7 m and 5.9 m respectively when the interaction between the gas concentration in the upper corner and gas drainage volume in roof low roadway was optimal. Field tests showed that the average gas concentration in the upper corner and the average gas drainage volume in roof low roadway were 0.432% and 40.861 m³/min respectively, both of which were less than 10% of the error from the simulations. The design of the roof low roadway has effectively managed the gas accumulation problem in the upper corner.

In China, gas hazards are still a prominent problem and gas accidents are often accompanied by loss of life1–3. Gas in the upper corner of the working face is a common gas problem and is managed in a number of ways6,7. Li et al.8 concluded that Y-type ventilation can reduce gas concentration in the upper corner. Xie9, Gao10 and Yang11 et al. concluded that directional long drilling group could effectively manage the gas in the upper corner. Lu et al.12 investigates the effectiveness of four gas drainage methods (high drill holes drainage, buried pipe drainage, adjacent roadway drainage and tail roadway drainage)13,14 for gas control in air return roadway. Skotniczny15 and Guo16 et al. studied the transport pattern of gas from the upper corner. Li et al.17 investigated methods for managing gas in the upper corner at different gas outflow levels. Wang et al.18,19 studied the methane distribution at a longwall working face.

Liu et al.20 used FLUENT software to simulate the effect of ground drilling location on gas drainage on the working face. Wang et al.21 used FLUENT software to simulate the air flow effect in the goaf, based on the porosity of the overlying rock obtained by PFC simulation. Brodny et al.22 used FLUENT to numerically simulate the impact of crushed rock types in the goaf on the air flow in the goaf. Zhang et al.23 conducted FLUENT numerical simulation of the dust distribution on the footway in the goaf. Chen et al.24 used FLUENT software to study the law of air movement in the return air roadway. Zhou et al.25 used FLUENT software to study the distribution of various gases in the goaf. Deng et al.26 used FLUENT software to study the influence of gas drainage on the distribution of oxidation zones in the goaf.

Cao27 and Zhang28 et al. investigated the gas drainage from the dug-in coal seam in the floor roadway; however, the floor roadway could not extract gas from the upper corner during mining. Li et al.29 studied the gas drainage from adjacent workings sharing a roof high roadway. Tang et al.30 conclude that gas drainage from roof high roadway has negative effect on the controlling the air leakage into the goaf. Zhang et al.31 investigated the effect of the position of roof high roadway on gas management. The roof high roadway is not effective in

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extracting gas from the upper corner, so it is necessary to design a roof low roadway to address the gas in the upper corner. Gao et al.34 analysed the stress distribution in the surrounding rocks of the roof low roadway. Zheng et al.35 concluded that tensile failure occurs mainly on the upper and lower sides of the roadway, while shear failure symmetrically occurs on the left and right sides.

There is relatively little research related to roof low roadway. The optimal position of the roof low roadway is investigated, which can be used to pre-extraction gas from this seam to cover the coal roadway excavation, and also to extraction gas from the upper corner during mining. The experimental scheme is designed according to the response surface methodology, and the response values are derived from the FLUENT numerical simulation experiments, and the optimal solution is predicted. The design of the roof low roadway has effectively managed the gas accumulation problem in the upper corner.

Table 1. Overburden parameters.

| No.  | Lithology     | Density (kg/m³) | Bulk modulus (Gpa) | Shear modulus (Gpa) | Cohesion (Mpa) | Friction angle (°) | Tensile strength (Mpa) |
|------|---------------|-----------------|--------------------|---------------------|----------------|-------------------|------------------------|
| 19   | Sandy mudstone| 2650            | 2                  | 13.5                | 3.2            | 42                | 1.2                    |
| 18   | Fine sandstone| 1400            | 2                  | 1.3                 | 2.3            | 27                | 2.3                    |
| 17   | No. 9 coal    | 2660            | 3.5                | 2.3                 | 2.1            | 36                | 0.7                    |
| 16   | Sandy mudstone| 2650            | 2                  | 13.5                | 3.2            | 42                | 1.2                    |
| 15   | Coarse sandstone| 2500           | 5.7                | 4.1                 | 5.0            | 38                | 0.8                    |
| 14   | K4 Limestone  | 2650            | 2.5                | 1.8                 | 7.1            | 45                | 1.5                    |
| 13   | Sandy mudstone| 2500            | 3.5                | 2.3                 | 2.1            | 36                | 0.7                    |
| 12   | Coarse sandstone| 2570           | 9.7                | 6.1                 | 8.0            | 40                | 0.8                    |
| 11   | Mid-stone     | 2600            | 5.8                | 4.3                 | 5.0            | 38                | 0.9                    |
| 10   | Sandy mudstone| 2400            | 3.5                | 2.3                 | 2.1            | 36                | 0.7                    |
| 9    | K3 Limestone  | 2650            | 2.5                | 1.8                 | 7.1            | 45                | 1.5                    |
| 8    | Fine sandstone| 2650            | 2                  | 13.5                | 3.2            | 42                | 1.2                    |
| 7    | Sandy mudstone| 2500            | 3.5                | 2.3                 | 2.1            | 36                | 0.7                    |
| 6    | K2 Limestone  | 2650            | 2.5                | 1.8                 | 7.1            | 45                | 1.5                    |
| 5    | Sandy mudstone| 2650            | 2                  | 1.3                 | 2.1            | 36                | 0.7                    |
| 4    | Fine sandstone| 2650            | 2.5                | 1.8                 | 7.1            | 45                | 1.5                    |
| 3    | Sandy mudstone| 2500            | 3.5                | 2.3                 | 2.1            | 36                | 0.7                    |
| 2    | No. 15 coal   | 1400            | 2                  | 1.3                 | 2.3            | 27                | 2.3                    |
| 1    | Sandy mudstone| 2500            | 3.5                | 2.3                 | 2.1            | 36                | 0.7                    |

Porosity and roof low roadway

Porosity of the goaf zone. The theory based on whether the shear stress reaches the shear strength as the failure criterion is the Moore–Coulomb theory. Based on the Mohr–Coulomb principal structure model in the discrete element UDEC software, a numerical calculation model for overburden strike mining at the 15,106 working face was established, with the model gravitational acceleration set to 9.8 m/s² and the coal seam burial depth of 396 m, resulting in a mean distributed load of 9.5 MPa at the top and a pressure measurement coefficient of 0.8 according to the coal seam burial depth. The model size is 500 m × 120 m, and the overburden parameters are shown in Table 1.

The rock layers are divided according to the overburden distribution, and the rock layers and grid are shown in Fig. 1a,b. The model was mining 38 times, the first 37 times for 8 m each and the last time for 4 m, for a total mining of 300 m.

The distribution of overburden fractures at mining depths of 40 m, 112 m and 300 m is shown in red line in Fig. 2a–c. The roof of the coal seam had not collapsed and the overburden had developed "arch" type off-bed fractures at the coal mining depth of 40 m. The direct roof of the coal seam had collapsed and the overburden produced a large number of off-bed fractures and through-bed fractures at the coal mining depth of 112 m. The off-bed fractures and through-bed fractures are mainly concentrated on the open-off cut side and mining side due to re-compaction of the overburden at the coal mining depth of 300 m. It can be inferred that the upper limit of the caving zone height is 18 m and the upper limit of the fractured zone height is 65 m, based on the extent of fracture development in the model.

For the overburden rock in the goaf area, the pore volume of the rock is proportional to the difference between the thickness of the fallen rock and the original rock. The porosity can be expressed as the ratio of the pore volume in the rock body to the total volume of the fallen rock body, as shown in Eq. (1) and Fig. 3:

\[
p = \frac{h_{n+1} - h_n - M_n}{h_{n+1} - h_n}
\]
Figure 1. The model (UDEC 6.0).

(a) Rock layers diagram

(b) Grid division diagram

Figure 2. The distribution of overburden fractures (UDEC 6.0).

(a) The mining depth of 40 m

(b) The mining depth of 112 m

(c) The mining depth of 300 m
where $h_n+1$ is the height of the upper intersection of the fallen rock, m; $h_n$ is the height of the lower intersection of the fallen rock, m; $M_n$ is the thickness of the original rock, m.

Based on the numerical simulation results, the porosity distribution of the strike caving zone is derived according to Eq. (1) and is shown in Fig. 4.

The porosity reaches a maximum of about 0.42 on the open-off cut side and about 0.34 on the mining side, while the porosity in the middle of the goaf is at a relatively small plateau of about 0.1 due to recompaction. Converting two dimensions to three dimensions, the porosity distribution is consistent with the "O" shaped ring theory.

**Range of position in the roof low roadway.** The rectangular section of the air return roadway is 4 m high and 5 m wide, the rectangular section of the roof low roadway is 3 m high and 4 m wide. As the mine is an outburst mine, according to the "Detailed rules for prevention of coal and gas outburst", to ensure safety, the vertical distance between the rock roadway and the coal roadway, i.e., the vertical distance, must be greater than 5 m.

The fracture angle of the overlying rock layer is generally 45°–80°. To ensure that the roof low roadway is within the caving zone, the horizontal distance between the roof low roadway and the air return roadway, i.e., the horizontal distance is 3 m when the vertical distance is 5 m. The schematic diagram of the nearest position between the roof low roadway and the air return roadway is shown in Fig. 5.

The yellow area in Fig. 5 is the fracture angle range, and the roof low roadway should be within the 45° fracture angle range. The closer the distance between the roof low roadway and the air return roadway, the better it is for the construction of downward drilling in the roof low road. Taking this into account, the range of position in the roof low roadway is determined as follows: 5–9 m for the vertical distance and 3–7 m for the horizontal distance.
Fluent numerical simulation
Goaf model and its gas distribution. Assumptions: (1) The variation in temperature and heat transfer during the flow of the fluid is not taken into account, and the mixture of gas and air is treated as an incompressible gas, ignoring the influence of temperature on the change in volume of the gas. (2) The goaf is regarded as a porous medium, and its transverse is divided into natural accumulation zone, stress loading zone and re-compaction zone; In the longitudinal direction, only considering caving zone and fractured zone. The porosity and viscous resistance coefficient of each zone are uniformly distributed and isotropic, but vary from zone to zone.

Combined with the field situation, the geometric model is established and meshed. The model has an inclination of 6°. The goaf, working face, and roadway models are all regular hexahedrons. Submap type grids can create hexahedral grids, which can ensure calculation accuracy and save time and cost. The mesh type is submap46 and 15,446,400 mesh zones are divided, the model is shown in Fig. 6.

The calculation of turbulence intensity is shown in Eq. (2)\(^\text{47}\).

\[
I = 0.16 \left( \frac{\nu d \rho}{\mu} \right)^{-0.125}
\]

where \(I\) is the turbulence intensity; \(\nu\) is the air velocity, m/s; \(d\) is the hydraulic diameter, m; \(\rho\) is the air density, 1.29 kg/m\(^3\); \(\mu\) is the air dynamic viscosity, 1.69×10\(^{-6}\) Pa s.

The standard \(k–\varepsilon\) model is selected as the calculation model\(^\text{48}\). The gas component is mixture of methane and air, and the gravity is set at −9.81 m/s\(^2\). The entrance of air inlet roadway is set to velocity-inlet. The air velocity measured by the air meter on site is 2.9 m/s. The hydraulic diameter is 4.4 m and the outlet of the air return
The turbulence intensity calculated by Eq. (2) is 2.14%. Fan is set at the interface between roof roadway and goaf, the negative pressure is 3 kPa, and the outlet of roof roadway is free outflow. The interface between working face and goaf is set as interior, and the interface of roof roadway in goaf is also interior. The other surfaces are set as wall. Each zone is set as fluid zone, and the porous zone, laminar zone and source terms are activated in the goaf. The porous media zone parameters are set using the UDF program and the source option sets the gas mass source item to $2.5 \times 10^{-7} \text{ kg/m}^3\text{s}$.

The distribution of gas in the goaf zone during normal mining is shown in Fig. 7. The gas concentration rises gradually as the deepening of goaf in the horizontal direction. In the vertical direction, in the upper part of the extraction area, there is very little air leakage and the gas concentration increases with height under the influence of gas uplift and gravity. On the air inlet roadway side, which is more affected by air leakage, the gas concentration is lower. On the air return roadway side, gas accumulation occurs, with maximum gas concentrations of 28%, 42%, 50% and 56% at 1 m, 20 m, 40 m and 60 m from the floor, respectively.

Model of roof low roadway and its drainage effect. The geometric model of roof low roadway is established by "Goaf model and its gas distribution", and drainage roadway is added. Assuming that the conditions and parameters remain unchanged, the model is shown in Fig. 8.

The upper corner gas concentration monitoring is set at the interface between working face and goaf, which is 20 cm away from the wall at the air return roadway. The coordinates of the monitoring points are set as (0, –99.252, 14.253). Gas drainage detection face is set in roof low roadway.

When the vertical distance between the roof low roadway and the roof of coal seam is 7 m, and the horizontal distance between the roof low roadway and the air return roadway is 5 m, the drainage effect is shown in Fig. 9a–c. At the monitoring point, the gas concentration in the upper corner is 0.39% and gas drainage volume in the roof low roadway is 36.4 m$^3$/min. The roof low roadway reduces the gas concentration around it.
Response surface methodology

The factors influencing the effectiveness of gas concentration control in the upper corner include the vertical and horizontal distance of the roof low roadway. The central combination design has a wide range of applicability to the combination of factors and levels, and the regression equation obtained has a good fit with the actual results\( ^{49,50} \). The central combination design is selected based on the two levels and two factors in this paper\( ^{51,52} \).

Based on the central composite design principle, a 2-level experiment was designed with the vertical \((X_1)\) and horizontal \((X_2)\) distance of the roof low roadway as the influencing factors, and the gas concentration in the upper corner \((Y_1)\) and the gas drainage volume in the roof low roadway \((Y_2)\) as the response values, resulting in a response surface experiment with 13 sets of experimental points. The vertical and horizontal distances of the roof low roadway were investigated for optimal gas concentration in the upper corner and gas drainage volume in the roof low roadway. The regression equation is given in Eq. (3)

\[
Y = \beta_0 + \sum_{i=1}^{n} \beta_i X_i + \sum_{i=1}^{n} \beta_{ii} X_i^2 + \sum_{i<j} \beta_{ij} X_i X_j
\]  

Figure 9. The drainage effect (FLUENT 16.0).

| Code number | Factors          | Levels          |
|-------------|------------------|-----------------|
|             | Factors          | Low  | High |
| X_1         | Vertical distance (m) | 5    | 9    |
| X_2         | Horizontal distance (m) | 3    | 7    |

Table 2. Factors and levels used for response surface of the central composite design.
The variance analysis of the gas concentration in the upper corner and the gas drainage volume in the roof low roadway is shown in Tables 4 and 5, respectively.

The P-value reflects the regression effect of the parameter. P < 0.0001 means the regression effect of the factor is extremely significant, 0.0001 ≤ P ≤ 0.05 means the regression effect of the factor is significant, and P ≥ 0.05 means the regression effect of the factor is insignificant. The Model F-value of 48.13 and 37.18 indicate that these two models are significant. There is only a 0.01% chance that an F-value this large could occur due to noise. The signal to noise ratio of the model is 19.289 and 17.654, both of which are greater than 4, indicates an adequate signal.

Response surface method analyses of the gas concentration upper corner (Y₁) and the gas drainage volume in the roof low roadway (Y₂) are shown in Figs. 10a,b and 11a,b.

With respect to the response value Y₁, the interaction between vertical and horizontal distance was not significant, with the effect of vertical distance being significantly more significant than horizontal distance. From the response surface plot of Y₁ and the positive quadratic term of Eq. (4), there is a minimum value of gas concentration in the upper corner. With respect to the response value Y₂, the interaction between vertical and horizontal distance is not significant and the effect of both vertical and horizontal distance is more significant.

$$Y_1 = 1.29422 - 0.24140X_1 - 0.015232X_2 + 0.017578X_1^2 + 0.00351563X_2^2 - 0.0025X_1X_2$$  \( \text{(4)} \)

$$Y_2 = 15.02419 + 3.73373X_1 + 1.171794X_2 - 0.24375X_1^2 - 0.18750X_2^2 + 0.093750X_1X_2$$  \( \text{(5)} \)

The variance analysis of the gas concentration in the upper corner and the gas drainage volume in the roof low roadway is shown in Tables 4 and 5, respectively.

### Table 3. Comparison table of central composite design scheme and simulation response values.

| Number | $X_1$ (m) | $X_2$ (m) | $Y_1$ (%) | $Y_2$ (m³/min) |
|--------|-----------|-----------|-----------|----------------|
| 1      | 9.82843   | 5         | 0.50      | 36.8           |
| 2      | 5         | 3         | 0.46      | 31.8           |
| 3      | 7         | 5         | 0.39      | 36.4           |
| 4      | 7         | 5         | 0.39      | 36.4           |
| 5      | 7         | 5         | 0.39      | 36.4           |
| 6      | 7         | 2.17157   | 0.425     | 34.2           |
| 7      | 5         | 7         | 0.51      | 35.7           |
| 8      | 9         | 3         | 0.46      | 34.4           |
| 9      | 4.17157   | 5         | 0.56      | 32.6           |
| 10     | 9         | 7         | 0.47      | 37.8           |
| 11     | 7         | 5         | 0.39      | 36.4           |
| 12     | 7         | 5         | 0.39      | 36.4           |
| 13     | 7         | 7.82843   | 0.41      | 36.1           |

### Table 4. Table of analysis of variance for quadratic model of gas concentration in upper corner.

| Source   | Degree of freedom | Mean square | F-value | P-value | Significance   |
|----------|-------------------|-------------|---------|---------|----------------|
| Model    | 5                 | 0.00742     | 48.13   | <1 × 10^-4 | Extremely significant |
| $X_1$    | 1                 | 0.00195     | 12.64   | 0.0093  | Significant    |
| $X_2$    | 1                 | 0.00018     | 0.22    | 0.3059  | Insignificant  |
| $X_1^2$  | 1                 | 0.034       | 223.11  | <1 × 10^-4 | Extremely significant |
| $X_2^2$  | 1                 | 0.00138     | 8.92    | 0.0203  | Significant    |
| $X_1X_2$ | 1                 | 0.0004      | 2.59    | 0.1517  | Insignificant  |

### Table 5. Table of analysis of variance for quadratic model of gas drainage volume in the roof low roadway.

| Source   | Degree of freedom | Mean square | F-value | P-value | Significance   |
|----------|-------------------|-------------|---------|---------|----------------|
| Model    | 5                 | 7.57        | 37.18   | <1 × 10^-4 | Extremely significant |
| $X_1$    | 1                 | 19.97       | 98.05   | <1 × 10^-4 | Extremely significant |
| $X_2$    | 1                 | 7.97        | 39.15   | 0.0004  | Significant    |
| $X_1^2$  | 1                 | 6.61        | 32.47   | 0.0007  | Significant    |
| $X_2^2$  | 1                 | 3.91        | 19.21   | 0.0032  | Significant    |
| $X_1X_2$ | 1                 | 0.56        | 2.76    | 0.1405  | Insignificant  |
From the response surface plot of $Y_2$ and the negative quadratic term of Eq. (5), there is a maximum value of the gas drainage volume in the roof low roadway.

The response surface method can be used to predict the vertical and horizontal distances when the interaction between the gas concentration in the upper corner and the gas drainage volume in the roof low roadway is optimal, as shown in Table 6.

The optimal solution under the interaction of response surface prediction is selected as the parameter for numerical simulation experiment. It is obtained that gas concentration in the upper corner is 0.40%, the gas drainage volume in the roof low roadway is 37.4 m$^3$/min, and the error from the predicted value is 0.75%.

| $X_1$ (m) | $X_2$ (m) | Predictive $Y_1$ (%) | Predictive value $Y_2$ (m$^3$/min) |
|-----------|-----------|----------------------|-----------------------------------|
| 7.7       | 5.9       | 0.397                | 37.4                              |

Table 6. Optimal results of response surface prediction.
0.53%, respectively. The predicted value is very close to the simulated value. The optimal position of the roof low roadway is: the vertical distance is 7.7 m, and the horizontal distance is 5.9 m.

Effectiveness of gas control in the upper corner
After construction in accordance with the optimum position of the roof low roadway, the 15,106 working face was ventilated in a "U" pattern. During the normal mining period, the test was conducted for one month, and the average inlet air volume of the air inlet roadway was 3123 m³/min and the average return air volume of the air return roadway was 2160 m³/min. The gas concentration in the upper corner, the gas concentration in roof low roadway, and the daily mining distance were tested. Based on the test results, the gas drainage volume in roof low roadway was obtained.

A scatter plot is made using the daily mining distance as the horizontal coordinate and the gas concentration in the upper corner and the gas drainage volume in roof low roadway as the vertical coordinates, as shown in Fig. 11.

The average daily mining of the working face during normal mining was calculated to be 4.493 m. The average gas concentration in the upper corner was 0.432%, an error of 8.8% compared to the simulation, and the average gas drainage volume in roof low roadway was 40.861 m³/min, an error of 9.8% compared to the simulation, which is a small error between the field test and the simulation. As can be seen in Fig. 11, the gas concentration in the upper corner and the gas drainage volume in roof low roadway show a certain trend of increase with the increase of the daily mining distance, indicating that reducing the mining speed can reduce the gas concentration in the upper corner to a certain extent.

The test results show that the design of roof low roadway, during normal mining, has no gas accumulation and effectively manages the gas accumulation problem in the upper corner.

Conclusions

1. According to the fracture distribution law during the mining, the upper limit heights of the caving zone and the fractured zone are obtained as 18 m and 65 m, respectively. The off-bed fractures and through-bed fractures are mainly concentrated on the open-off cut side and mining side at the coal mining depth of 300 m. On this basis, the maximum porosity of the two is calculated to be 0.42 and 0.34, respectively, which conforms to the ”O” shaped ring theory.

2. FLUENT numerical simulation shows the gas distribution law in the goaf, and the gas concentration on the side of the return air roadway is relatively high. After adding the roof low roadway, when the vertical distance and horizontal distance of the roof low roadway are 7 m and 5 m, respectively, the gas concentration in the upper corner and the gas drainage volume in roof low roadway are 0.39% and 36.4 m³/min, respectively, which reduces gas concentration on the side of the air return roadway.
3. Based on the calculated position range of the roof low roadway, taking the vertical distance and horizontal distance of the roof low roadway as the influencing factors, design the roof low roadway horizon experiment scheme according to the response surface method. FLUENT simulation experiment obtains the response value, predicting that the vertical and horizontal distance of the roof low roadway were 7.7 m and 5.9 m respectively when the interaction between the gas concentration in the upper corner and gas drainage volume in roof low roadway was optimal. Choose the optimal solution as the parameter to do the numerical simulation experiment, and the error between the experimental value and the predicted value is very small.

4. In the field application, the test shows that the average gas concentration in the upper corner and the average gas drainage volume in the roof low roadway are 0.432% and 40.861 m³/min respectively during the normal mining. The error of the experiment is less than 10%. The gas concentration in the upper corner and gas drainage volume in the roof low roadway show a certain increasing trend with the increase of the daily mining distance. The designed roof low roadway effectively controls the problem of gas accumulation in the upper corner.

Data availability
The primary data used to support the findings of this study are available from the corresponding author upon request.

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H.Z., S.F., and F.L. conceived and designed the study. Y.H., S.F., and L.H. were involved in the numerical simulation. H.Z., S.F., Q.L., and Y.Z. were involved in the data analysis. S.F., Y.H., Q.L., and Y.Z. wrote the first draft of the manuscript. All authors have read and approved the final manuscript.

Competing interests
The authors declare no competing interests.

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